Agenda

Huntington Planning Commission

Monday, April 1st, 2024 – 5:30pm

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1. Preliminaries

2. Call to Order

3. Roll Call

4. Approval of the January 2024 Minutes

5. Old Business

6. New Business

   PC 24-03

   Issue: Plan review of a proposal to redevelop 4514 Waverly Road on a property that is approximately 1.03 acres into a commercial retail establishment (Dollar General Market). The property is located at the northwest corner of the intersection of Waverly Road and Burlington Road in the Westmoreland neighborhood and is zoned C-2 Highway Commercial District.

   Owner/Petitioner: DG BTS Huntington, LLC, 2525 Broad Street, Chattanooga, TN 37408

7. Other Business or Announcements
   - Site visit to 111 Kings Highway (PC 24-02)

8. Good and Welfare

9. Adjournment

Minutes
Huntington Planning Commission
January 2, 2024

A meeting of the City of Huntington Planning Commission was held on January 2, 2024 at 5:30 p.m. in the City Hall Council Chambers. Mr. Gallagher called the meeting to order.

Members Present: Brian Gallagher, Mayor Steve Williams, Holly Smith Mount, Sarah Walling, Carl Eastham, Charles Shaw

Members Absent: Stephanie Vlahos Bryant & Ursulette Ward

Staff Present: Bre Shell, Planning Director
Ericka Hernandez, Assistant City Attorney
Steve Curry, Planner II
Cade Williams, Planner II

Ms. Mount made a motion to adopt December 5, 2023 Minutes. *Ms. Walling* seconded motion. *Mr. Gallagher* mentioned an error with Minutes. All were in favor, amendment to Minutes was approved: all were in favor, Minutes were approved.

Mr. Gallagher opened the floor for Chair and Vice Chair nominations. *Ms. Walling* nominated *Mr. Gallagher* as Chair. *Ms. Mount* seconded nomination. All were in favor, *Mr. Gallagher* was reelected as Chair. *Ms. Mount* nominated *Ms. Walling* for Vice Chair. *Mr. Eastham* seconded nomination. All were in favor, *Ms. Walling* was reelected as Vice Chair.

New Business
PC 24-01
A petition to rezone property from C-1 Commercial District to C-2 Highway Commercial District. This property is located on the southwestern block at 8th Street and 8th Avenue, between 7th Street and 8th Street and 8th Avenue and 8 1/2 alley.

Petitioners:
Matt Casto, 6 Brighton Way, Huntington, WV 25705
Mike Woelfel, 604 Ridgewood Rd., Huntington, WV 25701
Huntington Wholesale Furniture Co., 740 8th Ave., Huntington WV 25701
Robert Harrison, 705 8th Ave., Huntington, WV 25701

Ms. Shell read the Staff Report.

Ms. Mount mentioned Motel and Firearm Sales would be permitted by right with this rezoning. Additionally, she stated Adult Use and Pawn Shops would be conditional uses under this rezoning. She expressed concern regarding this petition. *Ms. Walling* agreed with *Ms. Mount's* opinion. She reiterated the concerning question if this rezoning is appropriate for the impacted block.

Matt Casto, President of Casto Land Inc., explained he purchased the property he wants to convert into an Indoor Self-Storage Development (this is the reason a rezoning is needed, Indoor Self-Storage Developments are permitted by right in the C-2 Highway Commercial District) six years ago as a fully occupied apartment building. He stated he plans on converting this building into a sixty-unit, climate controlled development. Mr. Casto proclaimed demographics and economy have changed within the last decade. Additionally, he added the City's comprehensive plan, Plan2025, in his opinion, does not reflect the landscape of the city as it is out-of-date. With recent regional economic development and the effects of COVID-19, new residents will need places to store

items. He wants to adhere to the guidelines the City has set forth, but he can't as the current zoning on this block restricts his desired use. This has led him to request a petition to rezone.

Russell Rice, a Certified Real Estate Appraiser in the State of West Virginia, spoke in support of this petition. He stated Mr. Casto's original plan was to develop an apartment building. However, Mr. Casto had to explore his second idea, a self-storage development, as developing apartments did not have the expected outcome. Mr. Rice had recommended his client to ask for an use variance but his client instead had to petition for a rezoning. Mr. Rice explained the first attempt of Mr. Casto petitioning was to rezone to I-1 Light Industrial/Commercial District. Then, Mr. Rice referenced Mr. Casto's second attempt to petition to rezone to C-2 Highway Commercial District. He acknowledged areas with spots of commercial zoning surrounded by residential zoning. Lastly, he brought up the issues with using Plan2025 and hopes for the creation of a new comprehensive plan.

Ms. Mount explained the petition would rezone the whole block Mr. Casto wants to build on and explained why the City rezoned a parcel to a commercial designation that is surrounded by residential zoning.

Ms. Walling recommended Mr. Rice to familiarize himself with the strategic planning process in correlation to developing a comprehensive plan. She specified the City has been in the process of developing this document for at least a year.

Mayor Steve Williams expressed interest upon the City for new business. However, he is concerned what would happen if the self-storage development fails. He does not want the neighborhood to absorb the pitfall if this business does not succeed.

Mr. Rice acknowledges *Mayor Steve Williams'* concern. He acknowledged this is the reason he wanted to do a zoning use variance. Additionally, he included a dwelling unit in the plans to be able to have an on premise manager 24-7. He confirms with *Mayor Steve Williams* the development will be 3 stories and include an elevator.

Ms. Hernandez and Ms. Shell confirmed zoning use variances are not allowed within the State of West Virginia. Mr. Rice explains the logic behind petitioning to rezone to the C-2 Highway Commercial District and the potential it could be for Adult Uses or Firearm Establishments if Mr. Casto's business fails. Additionally, he adds this rezoning would give Mr. Casto some more options to pursue if his self-storage development does not succeed.

Ms. Walling agreed with *Mayor Steve Williams'* concerns. She questioned Mr. Casto about his plans if the apartment complex he wants to convert to a self-storage development becomes financially unviable. Mr. Casto explained when he bought the building 7 years ago it was occupied and remained at this status until two years had passed. At this point the City had deemed the building unsafe. He explained just repairing windows would be $100,000.00 and believes he would not get ROI (return-on-investment) if he keeps the building as an apartment complex. He reiterated the cost of improvements by stating it would cost him over a million dollars to put in new units: the self-storage development would be a low cost option for him to pursue. Overall, he has explored numerous options and thinks the self-storage development would be the best option to pursue.

Ms. Walling appreciated the idea for investment in the Southside neighborhood, but still was concerned with his business idea. Additionally, *Ms. Walling* questioned why this building has set vacant for around 5 years when the self-storage development was Mr. Casto's backup business idea for this location. Mr. Casto claimed the City would not grant building permits for this property. Ms. Hernandez explained there was a disagreement between Mr. Casto and the Fire Marshal with plan details.

Ms. Walling questioned if Mr. Casto had checked with nearby self-storage developments on their occupancy rates. Mr. Casto confirmed Tri-State storage is at 75%, he stated the industry standard for expansion is 80%.

Mayor Steve Williams questioned the essence of a new plan. Mr. Casto alluded a full set of plans are not available as the ideas he has aren't set in stone.

Mayor Steve Williams alluded the City needs to be flexible for new business, including with making variances. He ended the comment by expressing when making a determination the Commission needs to examine if allowing the rezoning would broaden opportunity.

Ms. Walling questioned the size of the lot. Mr. Casto explained the lot is approximately 60 or 70 feet in length and 100 feet wide. *Ms. Walling* explained if a property is an acre or more in size a proposal must come back to Planning Commission once official plans are submitted. Mr. Casto explained he desires having a provision to allow the Commission to reverse their decision if the property becomes a public nuisance. *Ms. Walling* explained City Council passed an ordinance that makes it harder to declare something a public nuisance. Additionally, *Ms. Walling* questioned if from a legal stance there is anything the Commission is able to do. *Mayor Steve Williams* believed at some point we must make a decision and trust the market as well as the petitioner.

Mr. Shaw questioned if this petition could be laid over to the next meeting. *Mr. Gallagher* stated at the appropriate time that would be possible. Additionally, *Ms. Walling* acknowledged the decision made at this meeting would be as an advisement for City Council regarding this petition and explained the general process for this petition to be either approved or denied.

Ms. Walling and Mr. Gallagher temporarily swapped positions for Mr. Gallagher to voice his viewpoint.

Mr. Gallagher stated he hasn't heard from dialogue during this meeting how this petition would impact the affected area in terms of the criteria needed to be evaluated for the Commission to make a decision. This includes economical, physical, and social factors. Additionally, *Mr. Gallagher* expressed the objective concern the Commission has towards the result of rezoning this portion of the City to the C-2 Highway Commercial District. Mr. Casto explained the impact of COVID-19. *Mr. Gallagher* reiterated the Commission needs to know how his plan impacts the variables described by Ms. Shell and himself for the Commission to evaluate.

Ms. Walling and Mr. Gallagher assumed their regular positions on the Commission.

Mayor Steve Williams made a motion to move this petition to City Council with a favorable recommendation. *Mr. Eastham* seconded motion.

Planning Commission Roll Call: *Mr. Shaw,* Yes; *Ms. Mount,* No; *Mr. Eastham,* Yes; *Mayor Steve Williams,* Yes; *Ms. Walling,* No; *Mr. Gallagher,* No.

The petition to rezone was sent to City Council with an unfavorable recommendation with a vote 3 Yes to 3 No.

Other Business or Announcements
Ms. Shell noted Commission Member *Mr. Holley* has submitted his resignation to vacate his position on the Commission. Additionally, Ms. Shell stated a sheet will be passed around to update contact information for Commission Members and there will be another Steering Committee meeting for the Comprehensive Plan update and wished everyone a Happy New Year.

Ms. Walling and *Mr. Eastham* motioned to adjourn the meeting. *Ms. Mount* and *Ms. Walling* seconded motion. All were in favor, the meeting was adjourned.

Meeting adjourned at 6:58 P.M.

Date approved: _____

Chairperson: _____ Prepared by: _____
Brian Gallagher, Chair Cade Williams, Planner II

City of Huntington Planning Commission **April 1, 2024**
Staff Report: Public Hearing for Preliminary Site Plan Review.

PC 24-03
Issue: Plan review of a proposal to redevelop 4514 Waverly Road on a property that is approximately 1.03 acres into a commercial retail establishment (Dollar General Market). The property is located at the northwest corner of the intersection of Waverly Road and Burlington Road in the Westmoreland neighborhood and is zoned C-2 Highway Commercial District.

Property Owner: Paul Rutherford, 104 Briarwood Dr., Huntington, WV 25704
Petitioner: DG BTS Huntington, LLC, 2525 Broad Street, Chattanooga, TN 37408

Planning Commission Role
PC 24-03 is the public hearing for a preliminary site plan review. Because the site is over 1 acre, the plan must be approved through a public hearing of the Planning Commission. The Planning Commission's responsibility includes:
1. Receiving recommendations from staff and responding agencies and utilities.
2. Reviewing the design of future developments early in its design.
3. Ensuring that the requirements of the development design standards in the development ordinance are met.
4. Conformity with the Comprehensive Plan.
5. Reviewing waiver requests, when applicable.

Planning Commission Duties
Upon presentation of the preliminary plan at the public hearing, the Planning Commission will review recommendations from staff, other agencies, and experts, if used, and then approve, disapprove, or require that the developer provide more information about the preliminary plans.

Planning Commission provides the final decision and the item will not go before City Council. If approved by the Planning Commission, full plan sets can be submitted for review for building permit applications, and no City Council action or approval.

Public Notification
- Property owners and tenants within a 400 ft. radius of this property were notified of the project and hearing via letter.
- A legal ad was posted in the Herald Dispatch.

Introduction to the Petition
DG BTS Huntington LLC, is proposing to redevelop a property at 4514 Waverly Rd, approximately 1.03 acres into a commercial retail business (Dollar General Market).

Existing Conditions/Background
This property, and properties to the east and west on Waverly Road and to the north on Burlington Road are zoned C-2 Highway Commercial, which permits the proposed use. These properties include a mix of varying small businesses and single-family residences.

Properties to the south across Waverly Road are in the B&O Right-of-Way zone which in the vicinity of the proposed site, include single family residences and a carwash. Properties farther to the north, south, and east of are zoned R-2 single-family residential.

This property is 1.03 acres and was formally the Queen's Auto dealership. The lot currently features two buildings that make up the old dealership and six foot chain-link fencing around the western side of the western building. The main structure is currently used by the owner to store personal vehicles. Currently, the property is owned by Paul Rutherford of Huntington who has a tentative agreement with DG BTS Huntington LLC in that ownership of the parcel will not change unless permission is granted by the city to redevelop the parcel.

This property is well connected via a sidewalk network, nearby commercial uses including another dollar store (Family Dollar), a car wash, storage units and a bar.

Because this site is over 1 acre, plans need to be approved by Planning Commission through a public hearing process before applying for a building permit.

Proposed Conditions

The petitioner is proposing to build a new Dollar General Market location, a national chain of dollar stores that feature a larger selection of grocery and produce items than typical Dollar General stores.

In addition the petitioner has requested a waiver for the maximum front yard setback requirement. The maximum front yard setback in a C-2 Highway Commercial District is 75 feet with the proposed front façade facing Burlington Road having a setback of roughly 145 feet.

In addition to the store, the redevelopment will include a small parking lot in the front, a drive lane on the north side for trucks to load/unload, spaces for dumpsters, and landscaping.

There will be two new access points made; one on Waverly Road and one on Burlington Road. Please seen attached site and elevation plan.

Development Ordinance

The Development Ordinance designates any commercial development of over one acre or more as a major development, triggering special requirements and review.

Factors to consider when reviewing this type of development also includes:

- suitability of the land for development due to natural condition, such as flooding, drainage, and topography
- public installations such as location of schools or transportation facilities
- conditions which may endanger health, life, or property
- conformity with the zoning district requirements
- conformity with the comprehensive plan

The Planning Commission may only consider evidence presented for the record which is relevant to authorized grounds for approval.[1]

If applicant meets all requirements, the Commission is required to approve the plat.[2]

[1] *Kaufman v. Planning & Zoning Comm. Of Fairmont*, Syl Pt. 5, 298 SE2d 148 (W.Va. 1982).

[2] *Id.* at Syl. Pt. 8.

Pictures

View of the site looking in a northern direction from Waverly Road.

View of the site from the intersection of Waverly Road and Burlington Road looking in a westerly direction, showing both of the existing structures on the lot.

Comprehensive Plan/ Plan2025

Staff review finds that the proposed development is in conformity with the Future Land Use map of the comprehensive plan, Plan2025. Plan2025 designates this area as Convenience Commercial, which is designed for higher intensity commercial uses that are primarily accessed by cars.

Department/Agency Comments

- Cabell Huntington Health Department: Not applicable because municipal sewer is present.
- Huntington Stormwater Utility: requires any run-off generated would be required to be captured on-site before going into the municipal system.
- Mountaineer Gas: Initially disapproved, but Petitioner amended the plan to conform to MG requirements.
- Public Works: No comment
- Appalachean Power (AEP) : No comment
- West Virginia American Water (WVAW): No comment
- Huntington Sanitary Board: No comment

Staff Comments

In considering plans for new development, the Planning Commission and staff must consider compatibility with surrounding uses; impact on vehicular, freight, bicycle, and pedestrian traffic; capacity of existing utilities and city services, and zoning.

This site was previously a car dealership and has been mostly vacant and underutilized for many years. The proposed re-development demolishes the existing buildings and concentrates parking in the front yard with trucks utilizing the driveway on the north side.

The C-2 Highway Commercial zoning district is designed for high-intensity uses, and this use is permitted by right. Because the proposed development conforms to the zoning district, Staff does not anticipate that this development design would cause many negative or unintended consequences for the nearby residential. In fact, with the Stormwater Utility's requirement that any run-off generated be required to be captured on-site, the new development may actually reduce the impact of the existing site on the stormwater infrastructure. Ample parking is available on-site for the proposed use, so

parking is unlikely to spill over into the neighborhood.

Five-percent landscaping is required by the ordinance but is not reflected in the site plan presented. Staff recommends that the Commission require landscaping be included, possibly to buffer the parking on Waverly.

Redevelopment of former vacant commercial property into a new retail store generates more opportunities for jobs, tax revenue, and sustainability for this redevelopment and for the Westmoreland Neighborhood. Furthermore, this project adds a more convenient location for residents to obtain affordable produce and everyday items, something that has been lacking in the Westmoreland Neighborhood. Staff does not anticipate that development of this type will strain existing public resources, rather, it will contribute to the tax base and vibrancy of this location.

STAFF RECOMMENDATION: Approve with the condition that 5-percent landscaping be added.

Attachments

- Application
- Aerial Map
- Zoning Map
- Future Land Use Map
- Site Plan
- Elevation Plan and study
- Geotechnical Report
- Window and door plans

Application for New Development Review City of Huntington

Site Address/Tax Map, Parcel Number: 4514 Waverly Rd., Huntington, WV (50-06-0001-0223-000)

Acreage of Site: 1.038 AC

Proposed Use: Commercial Retail

Number and Acreage of Proposed Parcels (Subdivision Only): NA

Property Owner Name & Address: DG BTS Huntington, LLC. 2525 Broad Street, Chattanooga, TN 37408

Petitioner: Francis Stanley

Petitioner Email: fstanley@berryconstruction.net Petitioner Phone: 662-665-1195

Petitioner Address: 2525 Broad Street, Chattanooga, TN 37408

Supporting Items:

NA Site Plan — Site plan and survey & legal description have not changed from original submittal sent to Kim Estep on 12/21/23.

NA Survey & Legal Description (required for Major Subdivision)

___ $100 processing fee

FOR INTERNAL USE ONLY

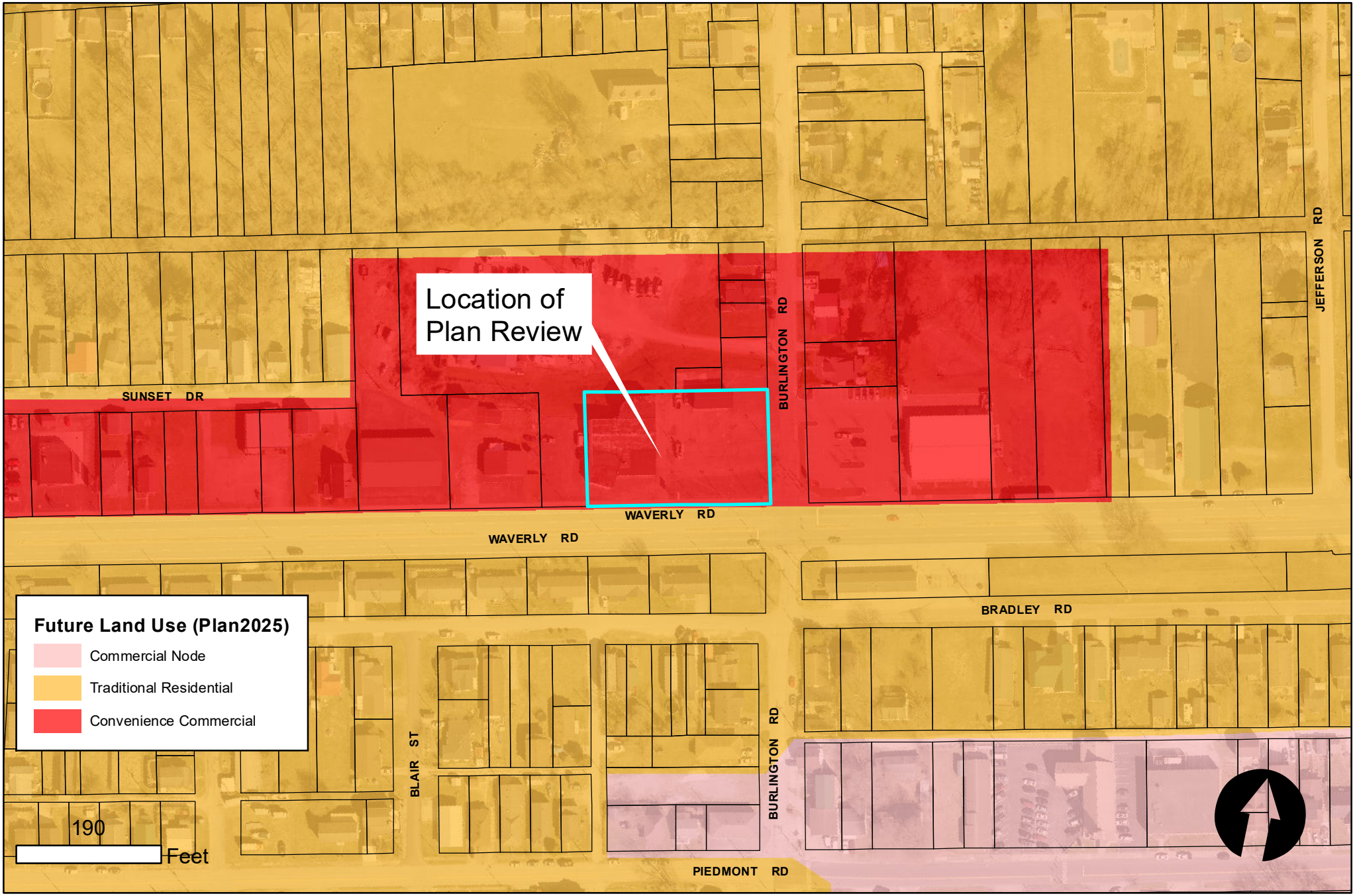
Date Received: _____

Staff Initials: _____

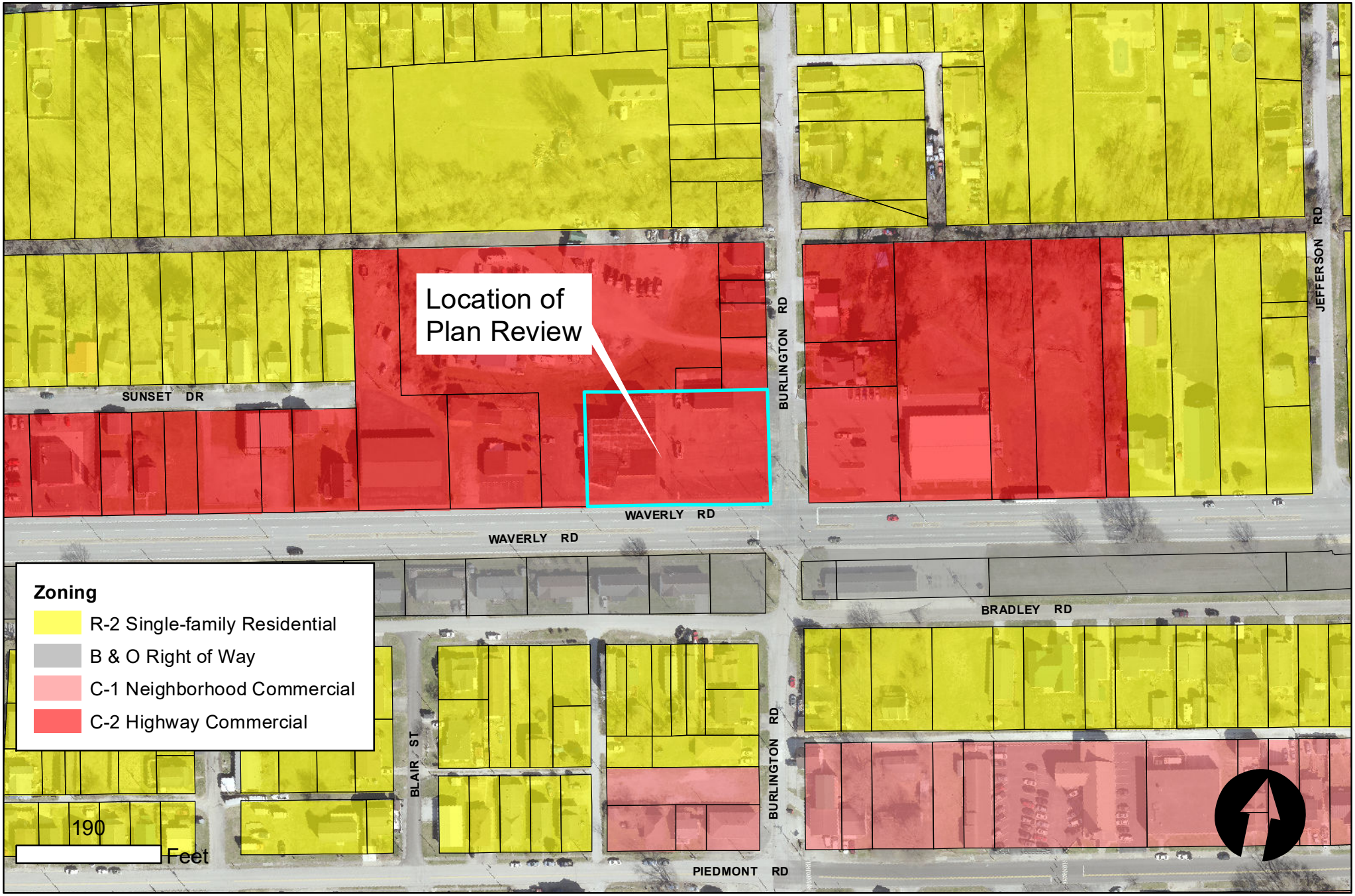
Petition Number:_____

Proposed Use: _____

4514 Waverly Road
Wayne County Tax District 6, Map 1,
Parcels 223
PC 23-03
Plan Review for a development over an acre in the C-2 Highway Commercial District.



4514 Waverly Road
Wayne County Tax District 6, Map 1,
Parcels 223
PC 23-03
Plan Review for a development over an acre in the C-2 Highway Commercial District.



4514 Waverly Road
Wayne County Tax District 6, Map 1,
Parcels 223
PC 23-03
Plan Review for a development over an acre in the C-2 Highway Commercial District.

LEGEND

- ASPHALT / ASPHALT (HD)
- ASPHALT / ASPHALT (LD)
- CONCRETE WALK / PAD
- LAWN/LANDSCAPE AREA
- LIMITS OF DISTURBANCE
- DI — CONCRETE DI
- SITE LIGHTING

GENERAL UTILITY NOTES:

1. THE LOCATIONS OF ALL KNOWN UTILITIES ARE SHOWN ON THE CONTRACT PLANS BASED ON THE BEST AVAILABLE INFORMATION FROM EXISTING PLANS AND FIELD INFORMATION. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ASCERTAIN THE STATUS AND LOCATION OF EACH UTILITY WHEN PERFORMING WORK WHICH MAY AFFECT THESE FACILITIES INCLUDING PROBING, EXCAVATION OR ANY OTHER PRECAUTION REQUIRED TO CONFIRM LOCATION. THE CONTRACTOR WILL BE RESPONSIBLE FOR ANY DAMAGE OR DISRUPTION TO UTILITY LINES WHICH ARE KNOWN ACTIVE AND ARE TO REMAIN IN OPERATION. THE CONTRACTOR SHALL CALL WEST VIRGINIA CALL BEFORE YOU DIG ENTITY PLUS ANY UTILITY COMPANIES NOT COVERED, AND HAVE ALL EXISTING UTILITIES FIELD LOCATED PRIOR TO CONSTRUCTION. IN THE EVENT OF DAMAGE OR DISRUPTION TO UTILITIES WHICH ARE ACTIVE AND ARE TO REMAIN IN SERVICE, THE CONTRACTOR SHALL IMMEDIATELY NOTIFY AN OFFICIAL OF THE AFFECTED UTILITY AND LEND ALL POSSIBLE ASSISTANCE IN RESTORING SERVICE. THE CONTRACTOR SHALL ASSUME ALL COST ASSOCIATED WITH THE REPAIR AND INTERRUPTION OF SUCH SERVICES.
2. ALL WATERLINES AND APPURTENANCES SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE STANDARDS AND SPECIFICATIONS ESTABLISHED BY THE LOCAL UTILITIES DEPARTMENT HAVING JURISDICTION.
3. THE CONTRACTOR SHALL SUPPLY A TEMPORARY SAFE WATER SERVICE TO ANY BUSINESS THAT WILL HAVE ITS WATER SERVICE INTERRUPTED BY THIS CONSTRUCTION.
4. ANY EXISTING HYDRANTS, VALVES, VALVE BOXES, METER PITS, SERVICE LINES, CURB BOXES OR WATER MAIN THAT ARE DAMAGED OR MUST BE ADJUSTED AND/OR MOVED, MUST BE REPAIRED, ADJUSTED, MOVED AND/OR REPLACED AT CONTRACTOR'S EXPENSE.
5. CONTRACTOR IS RESPONSIBLE FOR COORDINATION OF ALL UTILITIES. CONTRACTOR IS RESPONSIBLE FOR ALL TAP AND OTHER ASSOCIATED FEES.
6. ALL NECESSARY UTILITY PERMITS AND FEES BY CONTRACTOR.
7. ALL PVC SANITARY SEWER PIPES SHALL BE TYPE SDR35.

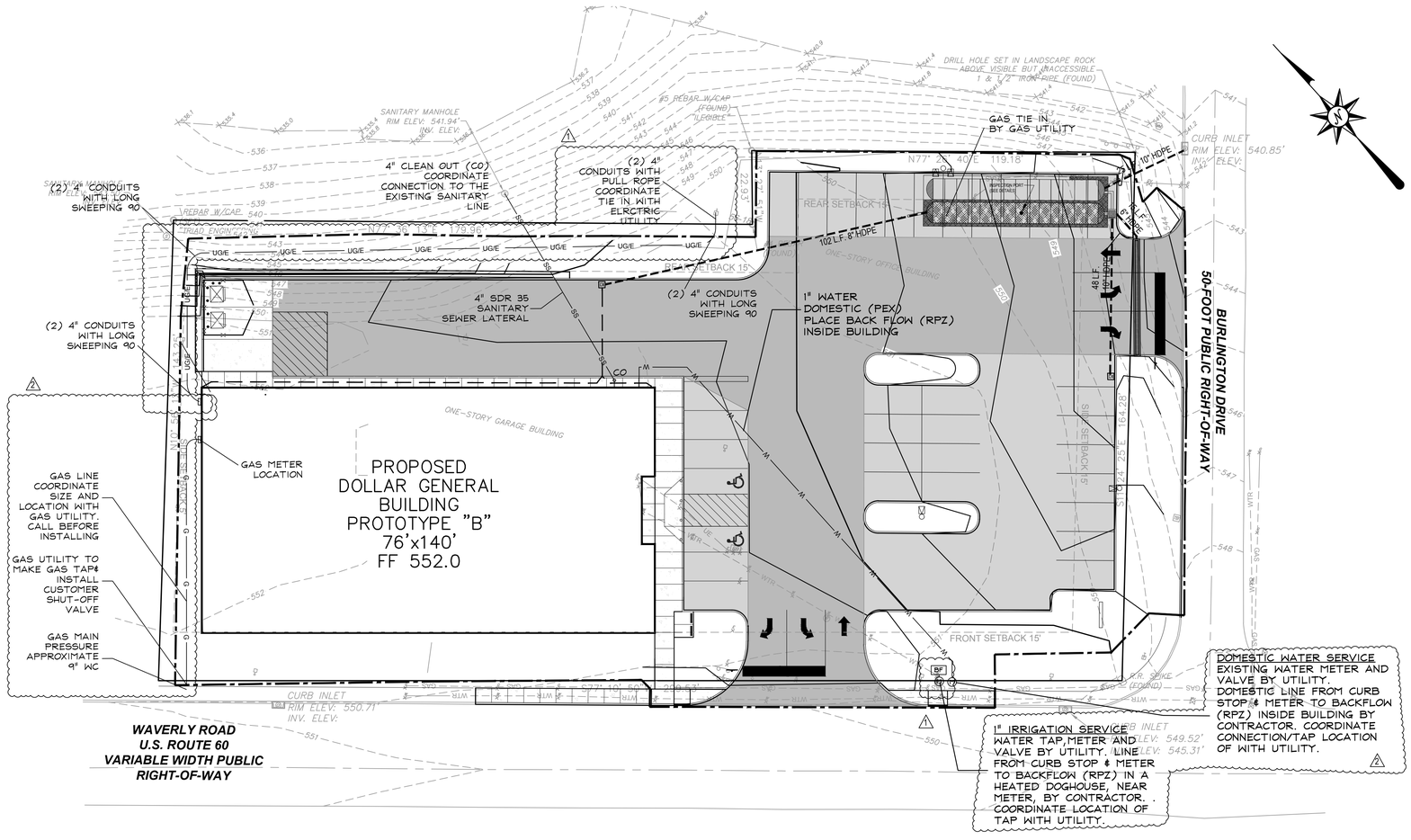
UTILITY CONTACTS:

ELECTRIC COMPANY: AEP
JEREMY BLACKSHIRE
PHONE NUMBER- 304-696-1215,(C) 304-681-219-6105

WATER PROVIDER: WEST VIRGINIA AMERICAN WATER
(WVAWC) HENRY PERKINS
PHONE NUMBER- 304-340-2986

SEWER PROVIDER: HUNTINGTON SANITARY BOARD
PHONE NUMBER- 304-696-4437

GAS COMPANY: MOUNTAINEER GAS
MICHAEL PLYMALE
PHONE NUMBER- 800-834-2070



UTILITY PLAN

20 10 0 20 40
DRAWING SCALE: 1"=20'

CALL BEFORE YOU DIG — MUWV — MISS UTILITY OF WEST VIRGINIA, INC — 1-800-245-4848

TRIAD ENGINEERING, INC.
10541 TEAYS VALLEY ROAD
SCOTT DEPOT, WV 25560
PH: 304.755.0721 FAX: 304.755.1880
OFFICE LOCATIONS
MARYLAND ○ PENNSYLVANIA ○ VIRGINIA ○ WEST VIRGINIA ○ OHIO

REV. #	DATE	DESCRIPTION	BY
1	1-12-24	ADDENDUM #1	
2	3-05-24	ADDENDUM #2 (PER WVAM)	
	3-18-24	UTILITY REVISIONS	

CADD FILE: 23-0595 DESIGN.dwg
DRAWN BY: XXX
CHECKED BY: XXX
SCALE: AS NOTED
DATE: 1/11/24

DG BTS HUNTINGTON, LLC
HUNTINGTON, WAYNE COUNTY WV

UTILITY PLAN

TRIAD ENGINEERING, INC.
www.triadeng.com

SHEET NUMBER:
C500

PROJECT No.: 04-23-0376

CONSTRUCTION DRAWINGS
FOR
DG BTS HUNTINGTON, LLC
HUNTINGTON, WEST VIRGINIA

SHEET INDEX:

SHEET DESCRIPTION	SHEET NUMBER
COVER SHEET	C100
EXISTING CONDITIONS	C200
DEMOLITION PLAN	C300
SITE PLAN	C400
LAYOUT PLAN	C401
UTILITY PLAN	C500
GRADING, DRAINAGE AND PAVING PLAN	C600
LANDSCAPE PLAN	C700
EROSION AND SEDIMENT CONTROL PLAN	C800
DETAILS	C900
DETAILS	C901
DETAILS	C902
DETAILS	C903
CULTEC STORM DETENTION PLANS	1-5

PROJECT INFORMATION:

DESCRIPTION:	COMMERCIAL BUILDING
ACREAGE:	APPROXIMATELY 1.03 ACRES TOTAL
ZONING:	NO
BUILDING SF:	10,640 BLDG / 8,838 SALE SF
BUILDING HEIGHT:	18' FEET
TOTAL # OF PARKING REQUIRED:	30 SPACES
TOTAL # OF PARKING SPACES:	35 SPACES
TOTAL # OF HANDICAP SPACES:	2 HANDICAP SPACES
SF OF IMPERVIOUS SURFACE SF OF PERVIOUS SURFACE	34505 SF (76%) 42388 SF (24%) TOTAL SITE 1.03 AC (45111 SF)(EXIST. 100% PAVED)
INTERIOR GREEN SPACE	21522 TOTAL PAVED (5%) 1076 PROVIDED 1102 SF
PHYSICAL ADDRESS:	4514 WAVERLY ROAD
PARCEL #	50-06-001-0223-0000

PROJECT CONTACTS:

DEVELOPER/OWNER DG BTS HUNTINGTON, LLC
ADDRESS 2525 BROAD STREET
CHATTANOOGA, TN 37408
PHONE NUMBER- 662-665-1195

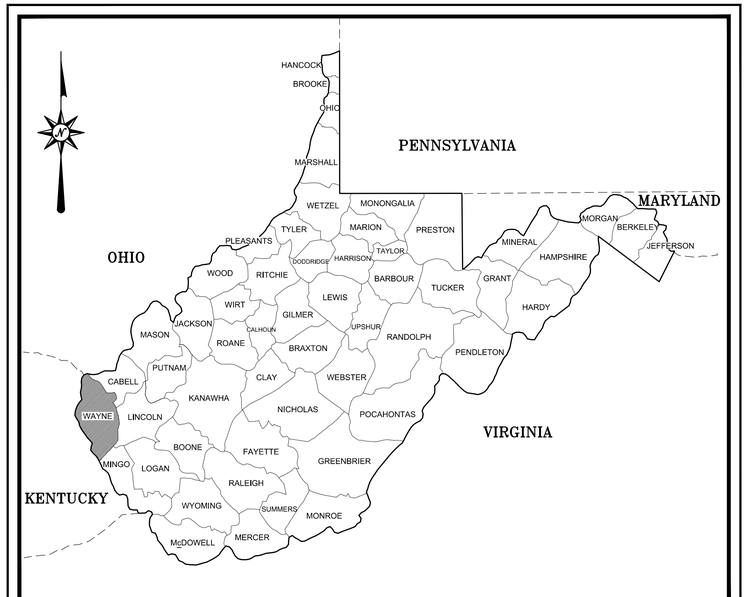
ENGINEER- TRIAD ENGINEERING
ADDRESS- 10541 TEAYS VALLEY ROAD
SCOTT DEPOT, WV 25560
PHONE NUMBER- 304-755-0721

SURVEYOR- TRIAD ENGINEERING
ADDRESS- 10541 TEAYS VALLEY ROAD
SCOTT DEPOT, WV 25560
PHONE NUMBER- 304-755-0721

THIS PROJECT ACCURATELY DEPICTS THE PROPOSED PROJECT

OWNER **DATE**

VICINITY MAP:



LOCATION MAP:

SCALE: NTS

TRIAD ENGINEERING, INC.
10541 TEAYS VALLEY ROAD
SCOTT DEPOT, WV 25560
PH: 304.755.0721 FAX: 304.755.1880
OFFICE LOCATIONS
MARYLAND ◇ PENNSYLVANIA ◇ VIRGINIA ◇ WEST VIRGINIA ◇ OHIO

REV. #	DATE	DESCRIPTION	BY
1			

CADD FILE: 23-0595 DESIGN.dwg
DRAWN BY: JHY
CHECKED BY: LLM
SCALE: NOTED
DATE: 2/13/2023

LE MCCOY
REGISTERED
14711
STATE OF WEST VIRGINIA
PROFESSIONAL ENGINEER

DG BTS HUNTINGTON, LLC
HUNTINGTON, WAYNE COUNTY WV

COVER

TRIAD ENGINEERING, INC.
www.triadeng.com

SHEET NUMBER:
C100
PROJECT No.: 04-23-0376

Plotted by: jyoung
J:\dw_ss_04\2023_5_ 04-23-0595 dg bts huntington civil\civil\cadd\23-0595 design.dwg

ABBREVIATIONS:

POINT OF BEGINNING:	P.O.B.	IRON PIPE OR PIN FOUND (AS NOTED)	
NOW OR FOMERLY:	N/F	RIGHT-OF-WAY MONUMENT (FOUND)	R/M
DEED BOOK:	D.B.	RAILROAD SPIKE (FOUND)	
PAGE:	PG.	MAG NAIL (FOUND)	
TAX MAP:	T.M.	#5 REBAR & TRIAD CAP (SET)	
TAX PARCEL:	PAR.	COMPUTED CORNER (NOT SET)	
AMERICAN DISABILITIES ACT:	A.D.A.	MAG SPIKE & TRIAD BACKING PLATE (SET)	

MAPPING SYMBOLS:

UTILITY POLE		WATER METER	WM
LIGHT POLE		WATER VAULT	W
LIGHT POST		WATER VALVE	
ELECTRICAL METER	M	FIRE HYDRANT	
ELECTRICAL JUNCTION BOX	EJB	NATURAL GAS METER	GM
WEATHER HEAD		DROP INLET	DI
		SANITARY SEWER MANHOLE	S
		BOLLARD	

LEGEND

SURVEYED PROPERTY LINE

UN-SURVEYED PROPERTY LINE

RIGHT-OF-WAY / EASEMENT LINE

CENTERLINE

WATER LINE — WTR

GAS LINE — GAS

UNDERGROUND ELECTRICAL LINE — UE

COORDINATE BASIS / BASIS OF BEARINGS:

THE COORDINATE SYSTEM USED TO PRODUCE THIS SURVEY IS BASED ON WEST VIRGINIA STATE PLANE COORDINATES ESTABLISHED BY TRIAD ENGINEERING USING THE WVDOH VIRTUAL REFERENCE SYSTEM (WVSPCS - SOUTH ZONE, NAD 83 - 2011EPOCH 2010 ADJUSTMENT).

BEARINGS SHOWN HEREON ARE ORIENTED TO GRID NORTH.

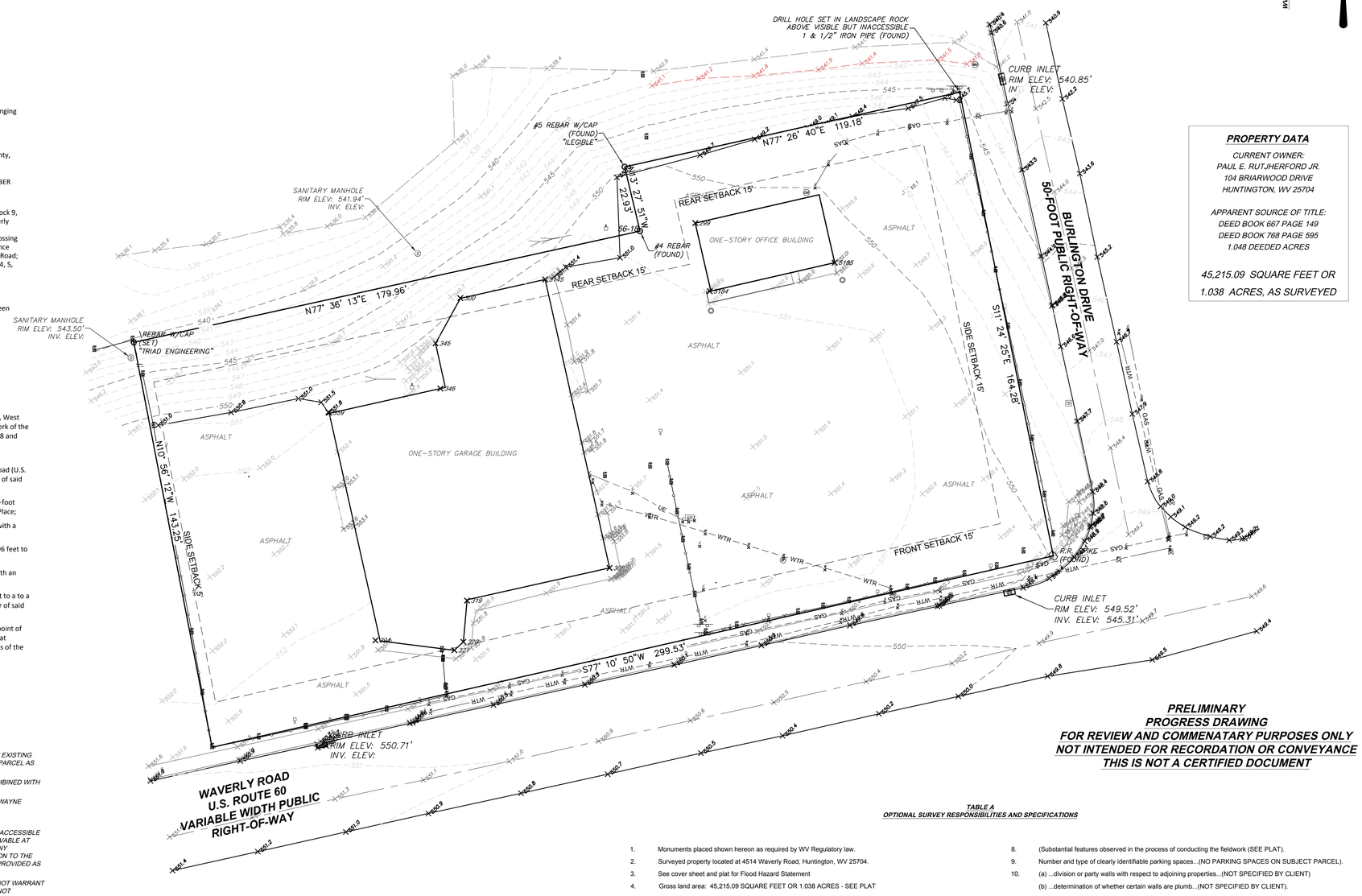
THE DIFFERENCE BETWEEN GRID NORTH AND TRUE NORTH (CONVERGENCE) IS 0° 44' 30" WEST .

THE VERTICAL DATUM FOR THIS SURVEY IS THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD 88).

THE MAGNETIC DECLINATION AT TIME OF SURVEY WAS COMPUTED AS 7° 34' W ± 0° 22' BY THE NATIONAL CENTER OF ENVIRONMENTAL INFORMATION ONLINE DECLINATION CALCULATOR.

GRID NORTH
WV STATE PLANE COORDINATE SYSTEM
NAD 83 - 2011 ADJ.(EPOCH 2010)

20 10 0 20 40
DRAWING SCALE: 1"=20'



PROPERTY DATA

CURRENT OWNER:
PAUL E. RUTJHERFORD JR.
104 BRIARWOOD DRIVE
HUNTINGTON, WV 25704

APPARENT SOURCE OF TITLE:
DEED BOOK 667 PAGE 149
DEED BOOK 768 PAGE 595
1.048 DEEDED ACRES

45,215.09 SQUARE FEET OR
1.038 ACRES, AS SURVEYED

LEGAL DESCRIPTION

SCHEDULE A - COMMITMENT FILE NO. : 281250.0072

(VERBATIM COPY FROM DOCUMENT)

All that certain lot, tract or parcel of land, together with the easements, rights of way and appurtenances thereunto belonging with the buildings and improvements thereon, and being more particularly bounded and described as follows:

TRACT NUMBER ONE (1):

All that certain lot or parcel of real estate situate in the City of Huntington, Huntington Independent District, Wayne County, West Virginia, known and
designated on a Revised Map of Dupont Place, a copy of which map is recorded in the Office of the Clerk of the County Commission of Wayne County, West Virginia on October 11, 1911, in Deed Book 103, at page 395, as part of BLOCK NUMBER NINE (9), of said Dupont Place, and being more particularly described as follows:

Beginning at a point which is the northwest corner of the intersection of Waverly Road with Burlington Road; thence in a westerly direction along the northerly line of Waverly Road, being the southerly line of Lots 9, 10, 11, 12 and 13 of said Block 9, 300 feet to a point; thence with the westerly line of said Lot 13, 150 feet to a point; thence easterly, parallel to the northerly line of Waverly Road and crossing Lots Nos. 13, 12 and 11, 180 feet to a point in the westerly line of Lot 10; thence in a northerly direction with said line, 30 feet to a point; thence easterly, parallel to the northerly line of Waverly Road and crossing Lot 10, 60 feet to a point in the westerly line of Lot 9; thence with said line, in a northerly direction 80 feet to a point; thence easterly, parallel to the northerly line of Waverly Road, crossing Lot 9, 60 feet to a point in the westerly line of Burlington Road; thence with the westerly line of Burlington Road; being the easterly line of Lot 9 (and also the easterly line of Lots 1, 2, 3, 4, 5, 6, 7, 8 and the southerly 5 feet of Lot 9 of the W.A. Lucas Subdivision" 258.46 feet to the point of beginning.

EXCEPTING THEREFROM, the southerly 10.8 feet of Lots 9 (Lot No. 1 of the Lucas Subdivision), 10, 11, 12, and 13, Block 9, Dupont Place, heretofore conveyed to the State Road Commission.

AND FURTHER EXCEPTION that certain 0.130 Acres conveyed in Corrective Deed dated December 17, 2009, by and between Paul E. Rutherford, Jr. and James Hatfield, of record in Deed Book 667, at page 808.

BOUNDARY DESCRIPTION

AS SURVEYED

All that certain lot or parcel of real estate situate in the City of Huntington, in the Westmoreland District of Wayne County, West Virginia, known and designated on a Revised Map of Dupont Place, a copy of which map is recorded in the Office of the Clerk of the County Commission of Wayne County, West Virginia on October 11, 1911, in Deed Book 103, at page 395, as part of Block 8 and Block 9 of said Dupont Place, and being more particularly described as follows:

BEGINNING at railroad spike found in asphalt pavement at the intersection of the northerly right-of-way line of Waverly Road (U.S. Route 60) with the westerly right-of-way line of Burlington Street, said point being the southeast corner of Lot 9 of Block 9 of said DuPont Place;

THENCE, With and as the northerly right-of-way line of Waverly Road, S 77° 10' 50" W for a distance of 299.53 feet to an 8-foot galvanized fence post found in said right-of-way line at the common southerly corner of Lot 13 and Lot 14 of said DuPont Place;

THENCE with and as the common line of Lots 13 and 14, N 10° 56' 12" W for a distance of 143.25 feet to a 5/8-inch rebar with a plastic identification cap marked "Triad Engineering" set in said common line;

THENCE leaving the common line of Lots 13 and 14 and crossing Lots 13, 12, and 11, N 77° 36' 13" E for a distance of 179.96 feet to a bare 1/2-inch rebar found at a point in the common line of Lots 11 and 10;

THENCE with and as the common line of Lots 11 and 10, N 13° 27' 51" W for a distance of 22.93 feet to a 5/8-inch rebar with an illegible plastic identification cap found in said common line;

THENCE leaving the common line of Lots 11 and 10 and crossing Lots 10 and 9, N 77° 26' 40" E for a distance of 119.18 feet to a to a hole drilled in a landscaping rock above a visible but inaccessible 1 and 1/2-inch iron pipe found at the northeastern corner of said Lot 9, being also a point in the westerly right-of-way line of the aforementioned Burlington Drive,

THENCE with and as the westerly right-of-way line of Burlington Drive, S 11° 24' 25" E for a distance of 164.28 feet to the point of beginning, and thus containing 42,215.09 square feet, or 1.038 acres, as surveyed and depicted hereon, and being all of that property described in a deed and shown on a plat of survey recorded in Deed Book 768 at Page 595, of record in the offices of the County Clerk of Wayne County, West Virginia.

SITE SURVEY NOTES:

THE PURPOSE OF THIS PLAT IS TO SHOW THE RESULTS OF A RETRACEMENT SURVEY OF THE BOUNDARY LINES OF AN EXISTING PARCEL OF LAND TO ESTABLISH A BASIS FOR THE SUBDIVISION AND FUTURE CONVEYANCE OF THE PROPOSED NEW PARCEL AS DEPICTED HEREIN.

PROPERTY DATA AS SHOWN HEREON IS COMPILED FROM EVIDENCE COLLECTED FROM AN ACTUAL FIELD SURVEY COMBINED WITH DATA OF PUBLIC RECORD AS REFERENCED HEREON.

ALL PUBLIC RECORD DOCUMENTS REFERENCED HEREON, ARE RECORDED IN THE OFFICE OF THE COUNTY CLERK OF WAYNE COUNTY, WEST VIRGINIA.

THERE WERE NO ENCROACHMENTS OR GAPS DISCOVERED BY THIS SURVEY.

UTILITIES AS SHOWN HEREON WERE LOCATED BY OBSERVED FIELD EVIDENCE OF STRUCTURES OR APPURTENANCES ACCESSIBLE AT THE SURFACE AND/OR BY SUBSURFACE UTILITIES DESIGNATION PAINT RECOVERED AT TIME OF SURVEY (IF OBSERVABLE AT TIME OF SURVEY) AND AS PLACED BY OTHERS. TRIAD WARRANTS ONLY THE ACCURACY OF THE FIELD LOCATION OF ANY DESIGNATION MARKS AS RECOVERED ON-SITE AND MAKES NO CLAIM AS TO THE ACCURACY OF THE MARKS IN RELATION TO THE ACTUAL POSITION OF THE UTILITY DESIGNATED. NO SUBSURFACE OR "DOWN-HOLE" INVESTIGATION REQUESTED OR PROVIDED AS PART OF THIS SURVEY.

TRIAD ENGINEERING WARRANTS ONLY THE COMPLETENESS OF LOCATION OF AVAILABLE UTILITY EVIDENCE AND CANNOT WARRANT THAT NO OTHER UTILITIES EXIST ON THIS SITE. THE UTILITY DATA SHOWN IS REPRESENTATIONAL ONLY AND SHOULD NOT SUBSTITUTE FOR SUBSURFACE UTILITIES INVESTIGATION PERFORMED PRIOR TO THE START OF ANY CONSTRUCTION OR EXCAVATION ON-SITE.

THE SUBJECT PROPERTY CAN BE ACCESSED BY AUTOMOBILE TRAFFIC FROM WINFIELD ROAD (WV ROUTE 817), A PUBLIC ROAD RIGHT-OF-WAY; HOWEVER, A DRIVEWAY ACCESS PERMIT WILL BE REQUIRED BY THE WEST VIRGINIA DEPARTMENT OF TRANSPORTATION FOR ANY FUTURE DEVELOPMENT ON THE SUBJECT PROPERTY.

THE PROPERTY SURVEYED AND DEPICTED HEREIN IS THAT PROPERTY IDENTIFIED IN TITLE COMMITMENT FILE NO. : 281250.0072 PREPARED FOR THE SUBJECT PROPERTY BY FIDELITY NATIONAL TITLE INSURANCE COMPANY, WITH AN EFFECTIVE DATE OF APRIL 5, 2023.

NOTE:

THIS PLAT IS NOT PREPARED AS A STAND-ALONE DOCUMENT AND IS PART OF A SET OF DRAWINGS. SEE SHEET S-1 (COVER SHEET) FOR SCHEDULE B-II COMMENTARY, BOUNDARY DESCRIPTIONS, AND SURVEYOR'S CERTIFICATION.

FLOOD HAZARD STATEMENT

THE SUBJECT PROPERTY DEPICTED HEREON LIES WITHIN A FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) DESIGNATED FLOOD HAZARD AREA IDENTIFIED AS "ZONE X (SHADED - LEVEE PROTECTED)" AS INDICATED BY NATIONAL FLOOD INSURANCE RATE MAP (FIRM) NUMBER 54011C0094E WITH AN EFFECTIVE DATE OF FEBRUARY19, 2014.

FEMA DEFINES "ZONE X (SHADED)" AS AN AREA OF 0.2% ANNUAL FLOODING OR AN AREA OF 1% ANNUAL CHANCE FLOOD WITH AVERAGE DEPTHS OF LESS THAN 1 FOOT. ALTHOUGH THIS PROPERTY DOES NOT LINE WITHIN A SPECIAL FLOOD HAZARD AREA, THIS DETERMINATION DOES NOT MEAN THAT FLOODING CANNOT OCCUR ON THIS SITE.

THIS DETERMINATION IS MADE BY GRAPHICAL PLOTTING OF THE SITE IN REFERENCE TO THE ABOVE NOTED FIRM. A F.E.M.A. ELEVATION CERTIFICATE WOULD BE REQUIRED TO MAKE A MORE DEFINITIVE DETERMINATION FOR THIS SITE; HOWEVER, AN ELEVATION CERTIFICATE WOULD NOT PROVIDE A WAIVER OF REQUIREMENT FOR THE PURCHASE OF FLOOD INSURANCE. ONLY A LOMA OR LOMR-F CAN AMEND THE FIRM AND REMOVE THE FEDERAL MANDATE FOR A LENDING INSTITUTION TO REQUIRE THE PURCHASE OF FLOOD INSURANCE.

PRELIMINARY
PROGRESS DRAWING
FOR REVIEW AND COMMENATARY PURPOSES ONLY
NOT INTENDED FOR RECORDATION OR CONVEYANCE
THIS IS NOT A CERTIFIED DOCUMENT

TABLE A
OPTIONAL SURVEY RESPONSIBILITIES AND SPECIFICATIONS

1. Monuments placed shown hereon as required by WV Regulatory law.
2. Surveyed property located at 4514 Waverly Road, Huntington, WV 25704.
3. See cover sheet and plat for Flood Hazard Statement
4. Gross land area: 45,215.09 SQUARE FEET OR 1.038 ACRES - SEE PLAT
5. Vertical relief shown hereon derived from terrain-based 3-d measurements. Vertical datum tied by GNSS observations (NAVD 88 - GEOID 12B).
6. Subject property is located Wayne County but in an area of the City of Huntington that is regulated by a zoning ordinance and is zoned C-2 Highway Commercial. However, a building permit will be required prior to the start of any construction on-site.
   Setback Requirements
   Front Yard Min/Max: 15 / 75 Feet
   Side Yard: 5 Feet
   Side Yard Adjoining Residential: 15 Feet
   Rear Yard: 15 Feet
   Maximum Building Height: 10 stories / 150 Feet
7. (a) Exterior dimensions of all buildings at ground level. (SEE PLAT)
   (b) Square footage of:
   (1) exterior footprint of all buildings at ground level. (SEE PLAT)
   (2) other areas as specified by the client. (NOT SPECIFIED BY CLIENT)
   (c) Measured height of all buildings above grade. (NOT SPECIFIED BY CLIENT)
8. (Substantial features observed in the process of conducting the fieldwork (SEE PLAT).
9. Number and type of clearly identifiable parking spaces...(NO PARKING SPACES ON SUBJECT PARCEL).
10. (a) ...division or party walls with respect to adjoining properties...(NOT SPECIFIED BY CLIENT)
    (b) ...determination of whether certain walls are plumb...(NOT SPECIFIED BY CLIENT).
11. Location of utilities existing on or serving the surveyed property (SEE PLAT)
12. As specified by the client, Governmental Agency survey-related requirements (NOT SPECIFIED BY CLIENT).
13. Names of adjoining owners according to current tax records....(AS REQUIRED BY WV LAW - SEE PLAT)
14. As specified by the client, distance to the nearest intersecting street.(SEE PLAT SHEET S-3).
15. Rectified orthophotography... (NOT SPECIFIED BY THE CLIENT).
16. Evidence of recent earth moving work, building construction, or building additions observed in the process of conducting the fieldwork. (NONE APPARENT)
17. Proposed changes in street right of way lines, if such information is made available to the surveyor by the controlling jurisdiction. Evidence of recent street or sidewalk construction or repairs observed in the process of conducting the fieldwork. (NONE APPARENT BASED ON BEST AVAILABLE PUBLIC INFORMATION)
18. Plottable off-site (appurtenant) easements (NOT SPECIFIED BY CLIENT)
19. (NOT SPECIFIED BY CLIENT)
20. NO CEMETERIES WERE OBSERVED ON-SITE WHILE CONDUCTING THE SURVEY.

TRIAD ENGINEERING, INC.
10541 TEAYS VALLEY ROAD
SCOTT DEPOT, WV 25560
PH: 304.755.0721 FAX: 304.755.1880
OFFICE LOCATIONS
MARYLAND ○ PENNSYLVANIA ○ VIRGINIA ○ WEST VIRGINIA ○ OHIO

REV. #	DATE	DESCRIPTION	BY

CADD FILE: 23-0595 DESIGN.dwg
DRAWN BY: XXX
CHECKED BY: XXX
DATE: 2/13/2023
SCALE: #########

DG BTS HUNTINGTON, LLC
HUNTINGTON, WAYNE COUNTY WV

EXISTING CONDITIONS

TRIAD ENGINEERING, INC.
www.triadeng.com

SHEET NUMBER:
C200
PROJECT No.: 04-23-0376

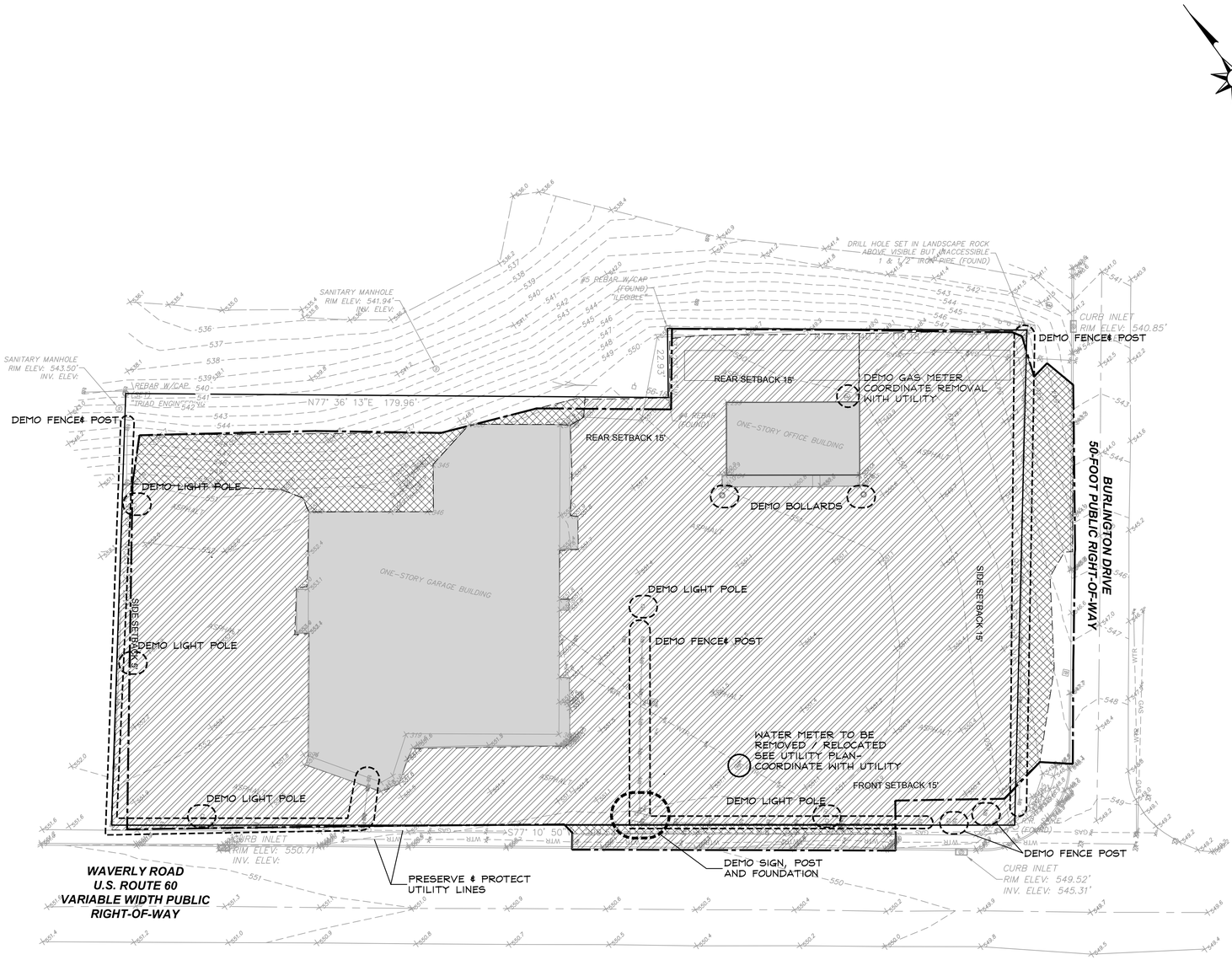
GENERAL DEMOLITION NOTES:

1. COORDINATE DEMOLITION OF ANY EXISTING UTILITIES WITH APPROPRIATE UTILITY COMPANY. REMOVE/ABANDON EXISTING UTILITIES/SERVICES/SITE FEATURES AS INDICATED.
2. CONTRACTOR IS RESPONSIBLE FOR FIELD LOCATING ALL EXISTING UTILITIES WITHIN THE PROJECT AREA. EXISTING APPURTENANCES SUCH AS UTILITY POLES, VALVE BOXES, ETC., ARE TO BE HELD BY THE CONTRACTOR DURING CONSTRUCTION. PROTECT UTILITIES AND APPURTENANCES NOT BEING REMOVED OR RELOCATED.
3. CALL WV UNDERGROUND PROTECTION SERVICE BEFORE DIGGING (8-1-1) OR (1-800-245-4848).
4. WHERE EXISTING ASPHALT CONCRETE OR CONCRETE ABUTS PROPOSED ASPHALT CONCRETE OR CONCRETE, THE CONTRACTOR SHALL SAWCUT EDGES.
5. REMOVE EXISTING ASPHALT IN ALL AREAS RECEIVING NEW ASPHALT. REMOVE/REPLACE/ADD AGGREGATE BASE AS REQUIRED TO PERFORM RE-PAVING OPERATION. IN LAWN AREAS, REMOVE ENTIRE BASE TO SUBGRADE.
6. CONTRACTOR SHALL DISPOSE OF ALL ASPHALT AND OTHER DEMOLISHED MATERIAL IN ACCORDANCE WITH ALL APPLICABLE LOCAL, STATE AND FEDERAL LAWS.
7. REFER TO SITE UTILITY PLAN FOR PROPOSED UTILITY LOCATIONS.
8. PROPERTY CORNERS DISTURBED DURING CONSTRUCTION INCLUDING CLEARING AND GRUBBING SHALL BE REPLACED BY A SURVEYOR LICENSED IN THE STATE OF WV AT THE COST OF THE CONTRACTOR.
9. REMOVE CONCRETE SLABS WITHIN LIMITS. REMOVE EXISTING FOUNDATIONS TO AT LEAST 2 FEET BELOW PROPOSED FEATURES.

LEGEND

- DEMO & CLEARING / GRUBBING LIMITS
- DEMO ASPHALT ASPHALT / CONCRETE
- DEMO BUILDING, FOUNDATIONS & ASSOCIATED CONCRETE WALK
- PROTECT ITEM NOTED
- LIMITS OF DISTURBANCE
- SAW CUT
- DEMO ITEM NOTED

NOTE:
THE DEMOLITION PLAN IS A GRAPHIC REPRESENTATION OF WHAT SHOULD BE REMOVED. THE CONTRACTOR IS TO USE GRADING PLANS, LAYOUT PLANS AND UTILITY PLANS TO ACCURATELY DETERMINE LIMITS AND MATERIALS TO BE REMOVED.



DEMOLITION PLAN

20 10 0 20 40
DRAWING SCALE: 1"=20'

CALL BEFORE YOU DIG
MUWV
MISS UTILITY OF WEST VIRGINIA, INC
1-800-245-4848

TRIAD ENGINEERING, INC.
10541 TEAYS VALLEY ROAD
SCOTT DEPOT, WV 25560
PH: 304.755.0721 FAX: 304.755.1880
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DG BTS HUNTINGTON, LLC
HUNTINGTON, WAYNE COUNTY WV

DEMOLITION PLAN

TRIAD ENGINEERING, INC.
www.triadeng.com

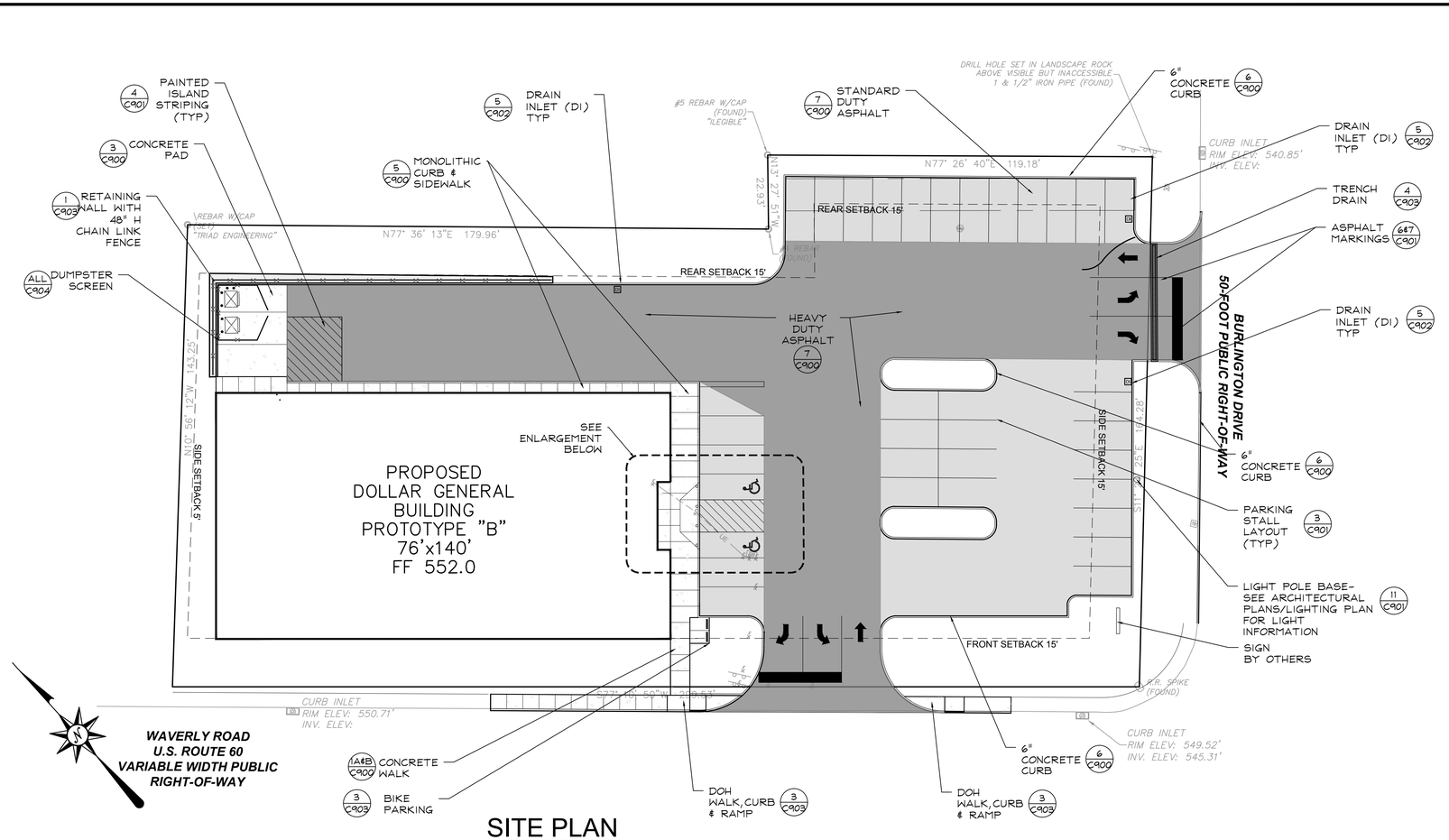
SHEET NUMBER:
C300
PROJECT No.: 04-23-0376

LEGEND

- ASPHALT
- ASPHALT (HD)
- ASPHALT
- ASPHALT (LD)
- CONCRETE WALK / PAD
- LAWN/LANDSCAPE AREA
- CONCRETE PAD
- DI CONCRETE DI

GENERAL NOTES:

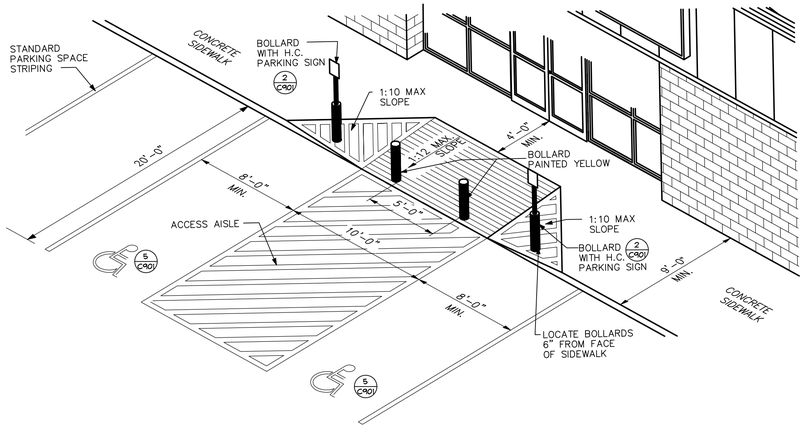
1. GENERAL CONTRACTOR (G.C.) IS RESPONSIBLE FOR ALL FEES AND PAPERWORK REQUIRED FOR PERMITS AND APPROVALS, INCLUDING EROSION AND SEDIMENT, BUILDING,UTILITY, SIGNAGE, DRIVEWAYS, ETC. PRIOR TO CONSTRUCTION VERIFY WITH ENGINEER ANY PERMITS THAT ALREADY HAVE BEEN OBTAINED.
2. UTILITY, TOPOGRAPHICAL, AND BOUNDARY SURVEY INFORMATION PROVIDED BY TRIAD ENGINEERING, INC. PHONE: 304.755.0721.
3. G.C. TO VISIT THE SITE AND ACCEPT THE SITE CONDITIONS AFTER REVIEWING DESIGN DOCUMENTS AND PRIOR TO BIDDING.
4. G.C. IS RESPONSIBLE FOR CHECKING ALL DIMENSIONS AND VERIFY DIMENSIONS NOTED ON PLANS. ANY DISCREPANCIES SHALL BE REPORTED TO THE OWNERS REPRESENTATIVE IN WRITING PRIOR TO FURTHER CONSTRUCTION.
5. G.C. IS RESPONSIBLE FOR OBTAINING PROPER APPROVALS AND OR PERMITS FOR WORK IN THE R.O.W., PRIVATE PROPERTY OR PRIVATE / PUBLIC EASEMENTS.
6. G.C. IS RESPONSIBLE FOR LOCATING AND AVOIDING ALL UNDERGROUND UTILITIES. UTILITIES SHOWN ON SITE PLAN ARE BASED ON THE BEST INFORMATION AVAILABLE AT THE TIME OF THE SURVEY, AND MAY NOT BE ALL INCLUSIVE. CONTACT A UTILITY LOCATION SERVICE FOR UTILITY IDENTIFICATION PRIOR TO ANY WORK.
7. G.C. IS RESPONSIBLE FOR KEEPING ALL PLANTING AREAS FREE OF CONSTRUCTION DEBRIS, STONE, CONCRETE ETC..
8. G.C. TO SAW CUT AND PATCH / REPAIR EXISTING ASPHALT THAT HAS BEEN DAMAGED FOLLOWING SITE IMPROVEMENTS.
9. G.C. TO COORDINATE WITH PLUMBING AND ELECTRICAL CONTRACTOR FOR UTILITIES INSTALLATION, MATERIALS, AND FOR ALL UTILITY PERMITS AND APPROVALS.
10. G.C. TO OBTAIN ANY APPROVALS BY JURISDICTIONAL AUTHORITIES NECESSARY FOR MODIFICATIONS TO APPROVED PLANS.
11. G.C. AND LANDSCAPE CONTRACTOR TO REFER TO LANDSCAPE PLANS AND DETAILS FOR PLANTING NOTES, PLANT SCHEDULE AND PLANTING DETAILS.
12. G.C. TO ENSURE THAT UTILITIES TO ADJACENT PROPERTIES/USES REMAIN OPERATIONAL DURING CONSTRUCTION.
13. A LICENSED SURVEYOR SHALL STAKE OUT ALL PARING, CURBING, ETC.., AND SHALL BE REVIEWED BY THE OWNERS REPRESENTATIVE PRIOR TO ANY CONSTRUCTION.
14. ALL LOT STRIPING, DIRECTIONAL ARROWS, AND STOP BARS TO BE WHITE REFLECTING PAINT AND SHALL CONFORM TO WVDOH MATERIAL SPECIFICATIONS AND THE MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES. PARKING LOT STRIPES TO BE 4" WIDE.
15. CONSTRUCTION EQUIPMENT AND/OR MATERIALS SHALL NOT BE STORED/PARKED WITHIN THE R.O.W. OR WITH IN THE DRIP-LINE OF ANY TREES.
16. ALL SITE UTILITIES SHALL BE RAISED OR LOWERED TO ACCOMMODATE NEW FINISH GRADES AS NEEDED.
17. ALL EXPANSION JOINTS ADJOINING BUILDINGS AND OTHER NON-BITUMINOUS ASPHALTS SHALL CONTAIN ½ INCH EXPANSION MATERIAL.
18. CREATE A TIGHT JOINT WHERE OLD ASPHALT ABUTS NEW ASPHALT. ALL EXPANSION AND CONSTRUCTION JOINS PERPENDICULAR TO CURB AND BUILDINGS SHALL BE LAID OUT AT 90° TO CURB UNLESS OTHERWISE NOTED.

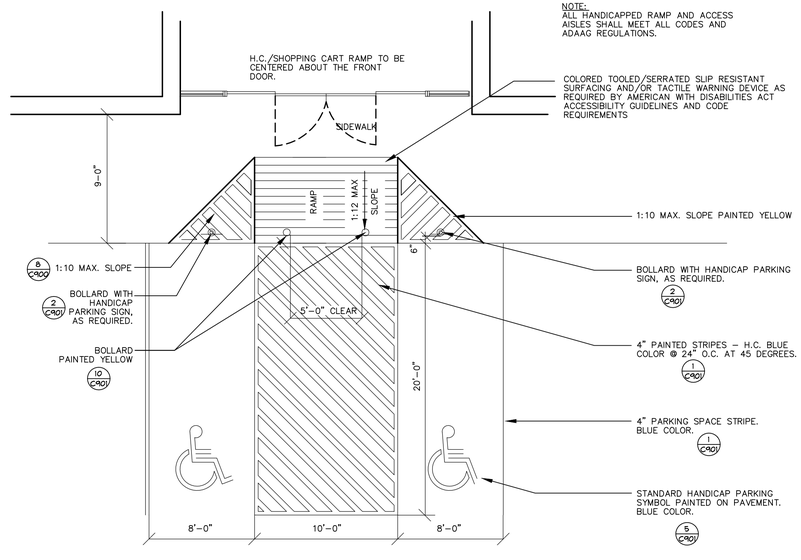


SITE PLAN

20 10 0 20 40

DRAWING SCALE: 1"=20'





ENTRANCE ENLARGEMENT

CALL BEFORE YOU DIG
MUWV
MISS UTILITY OF WEST VIRGINIA, INC
1-800-245-4848

TRIAD ENGINEERING, INC.
10541 TEAYS VALLEY ROAD
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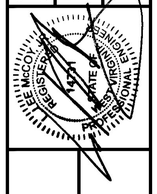


DG BTS HUNTINGTON, LLC
HUNTINGTON, WAYNE COUNTY WV

SITE PLAN

TRIAD ENGINEERING, INC.
www.triadeng.com

SHEET NUMBER:
C400

PROJECT No.: 04-23-0376

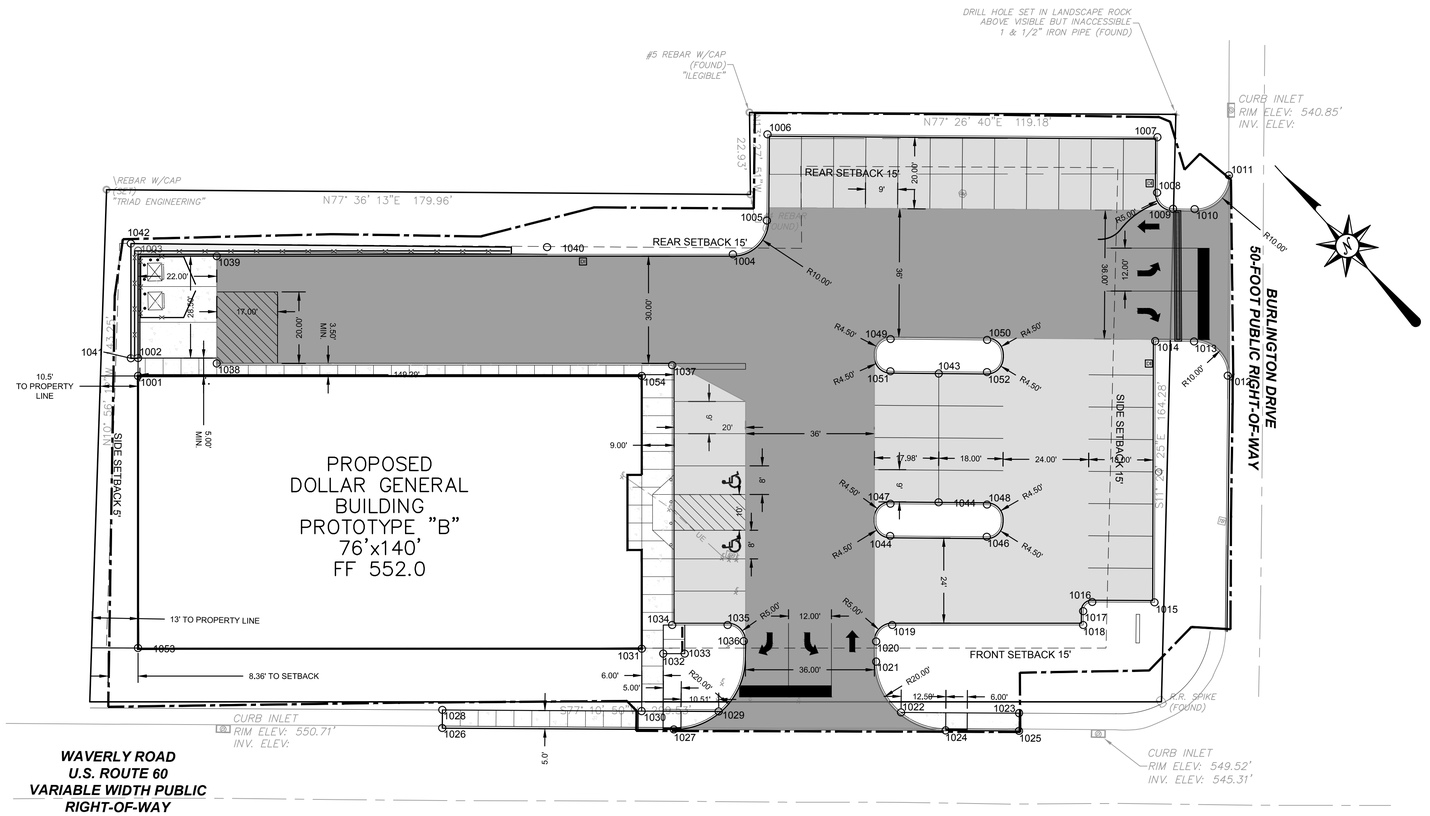
LEGEND

- ASPHALT / ASPHALT (HD)
- ASPHALT / ASPHALT (LD)
- CONCRETE WALK / PAD
- LAWN/LANDSCAPE AREA
- CONCRETE PAD
- DI CONCRETE DI

LAYOUT NOTES:

1. ALL MEASUREMENTS ARE TO BE FIELD VERIFIED.
2. ALL EXPANSION AND CONTRACTION JOINTS PERPENDICULAR TO CURB AND BUILDING SHALL BE LAID OUT AT 90° TO CURB UNLESS OTHERWISE NOTED.
3. DO NOT SCALE FROM THIS DRAWING. ALL WRITTEN DIMENSIONS SHALL GOVERN. ALL ANGLES ARE 90° UNLESS OTHERWISE NOTED.
4. ALL DIMENSIONS SHOWN ON THE PLANS ARE FROM THE FACE OF CURB. ALL LAYOUT POINTS ARE TO THE BACK OF THE CURB OR AS NOTED.
5. DI AND YD POINTS ARE AT CENTER OF DRAIN TYPICAL.

BUILDING: THE BUILDING LOCATION POINTS GIVEN HERE ARE FOR ROUGH GRADING ONLY. FOR ACTUAL BUILDING DIMENSIONS REFER TO ARCHITECT'S PLANS.



LAYOUT PLAN

20 10 0 20 40

DRAWING SCALE: 1"=20'

Point Table			
Point #	Northing	Easting	Description
1001	512930.1226	1532699.4137	1001
1002	512934.9980	1532698.3043	1002
1003	512964.2502	1532691.6479	1003
1004	513000.1665	1532853.9929	1004
1005	513011.4686	1532861.1637	1005
1006	513035.0239	1532855.9878	1006
1007	513058.4167	1532962.4480	1007
1008	513043.2778	1532965.7745	1008
1009	513039.8486	1532971.1360	1009
1010	513041.1362	1532977.0276	1010
1011	513052.4454	1532984.2803	1011
1012	512997.7799	1532996.4028	1012
1013	513005.0302	1532985.0831	1013
1014	513002.6945	1532974.4531	1014
1015	512931.4299	1532990.4631	1015
1016	512927.6989	1532973.4831	1016
1017	512924.6270	1532971.6004	1017
1018	512920.8502	1532972.4598	1018
1019	512909.0036	1532920.3988	1019
1020	512903.5183	1532917.0320	1020
1021	512898.0160	1532918.2909	1021
1022	512885.7154	1532928.2347	1022
1023	512892.8552	1532960.4617	1023
1024	512883.5866	1532941.5094	1024
1025	512888.0226	1532961.5324	1025
1026	512853.0052	1532804.1745	1026
1027	512866.9877	1532867.3666	1027
1028	512857.9745	1532803.0760	1028
1029	512874.6892	1532878.4660	1029
1030	512870.0023	1532857.4692	1030

Point Table			
Point #	Northing	Easting	Description
1031	512887.0122	1532853.6137	1031
1032	512887.0028	1532859.7693	1032
1033	512888.3340	1532865.6197	1033
1034	512895.3580	1532860.4319	1034
1035	512898.7972	1532875.5455	1035
1036	512895.4078	1532880.9317	1036
1037	512966.1725	1532844.3179	1037
1038	512938.4167	1532720.0887	1038
1039	512967.6689	1532713.4323	1039
1040	512990.5918	1532802.9018	1040
1041	512934.4850	1532696.3699	1041
1042	512965.7737	1532689.2501	1042
1043	512980.4472	1532917.4706	1043
1044	512945.3247	1532925.4352	1044
1045	512933.1699	1532914.3221	1044
1046	512938.9383	1532940.6991	1046
1047	512941.9586	1532912.3833	1047
1048	512947.7270	1532938.7603	1048
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1050	512992.6084	1532928.6130	1050
1051	512978.0513	1532904.1747	1051
1052	512983.8198	1532930.5518	1052
1053	512855.9575	1532716.2901	1053
1054	512961.3613	1532836.6955	1054

TRIAD ENGINEERING, INC.
10541 TEAYS VALLEY ROAD
SCOTT DEPOT, WV 25560
PH: 304.755.0721 FAX: 304.755.1880
OFFICE LOCATIONS
MARYLAND ○ PENNSYLVANIA ○ VIRGINIA ○ WEST VIRGINIA ○ OHIO

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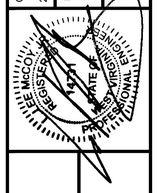
SCALE: #########



DG BTS HUNTINGTON, LLC
HUNTINGTON, WAYNE COUNTY WV

LAYOUT PLAN

TRIAD ENGINEERING, INC.
www.triadeng.com

SHEET NUMBER:
C401

PROJECT No.: 04-23-0376

CALL BEFORE YOU DIG
MUWV
MISS UTILITY OF WEST VIRGINIA, INC
1-800-245-4848

LEGEND

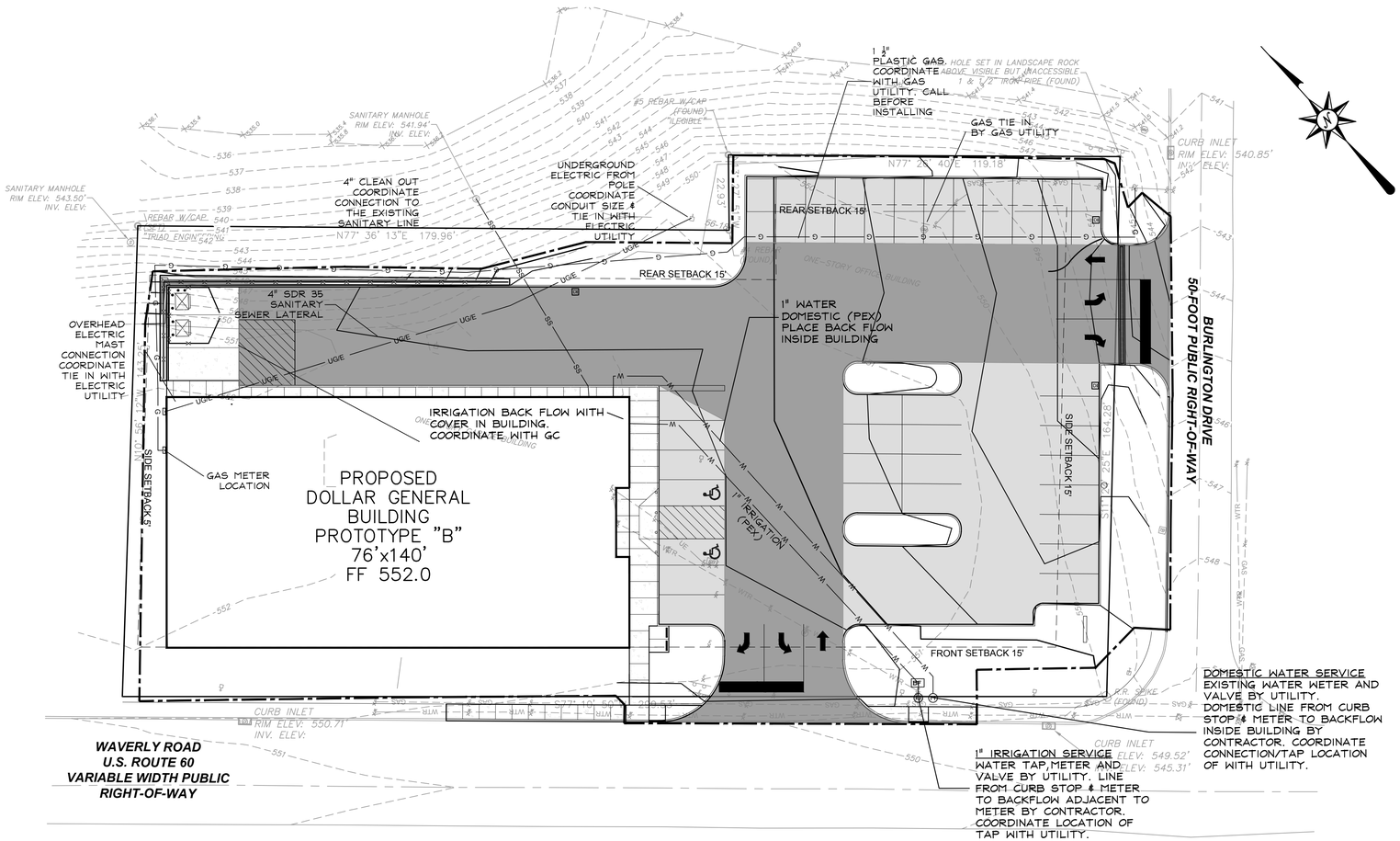
- ASPHALT / ASPHALT (HD)
- ASPHALT / ASPHALT (LD)
- CONCRETE WALK / PAD
- LAWN/LANDSCAPE AREA
- LIMITS OF DISTURBANCE
- DI CONCRETE DI
- SITE LIGHTING

GENERAL UTILITY NOTES:

1. THE LOCATIONS OF ALL KNOWN UTILITIES ARE SHOWN ON THE CONTRACT PLANS BASED ON THE BEST AVAILABLE INFORMATION FROM EXISTING PLANS AND FIELD INFORMATION. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ASCERTAIN THE STATUS AND LOCATION OF EACH UTILITY WHEN PERFORMING WORK WHICH MAY AFFECT THESE FACILITIES INCLUDING PROBING, EXCAVATION OR ANY OTHER PRECAUTION REQUIRED TO CONFIRM LOCATION. THE CONTRACTOR WILL BE RESPONSIBLE FOR ANY DAMAGE OR DISRUPTION TO UTILITY LINES WHICH ARE KNOWN ACTIVE AND ARE TO REMAIN IN OPERATION. THE CONTRACTOR SHALL CALL WEST VIRGINIA CALL BEFORE YOU DIG ENTITY PLUS ANY UTILITY COMPANIES NOT COVERED, AND HAVE ALL EXISTING UTILITIES FIELD LOCATED PRIOR TO CONSTRUCTION. IN THE EVENT OF DAMAGE OR DISRUPTION TO UTILITIES WHICH ARE ACTIVE AND ARE TO REMAIN IN SERVICE, THE CONTRACTOR SHALL IMMEDIATELY NOTIFY AN OFFICIAL OF THE AFFECTED UTILITY AND LEND ALL POSSIBLE ASSISTANCE IN RESTORING SERVICE. THE CONTRACTOR SHALL ASSUME ALL COST ASSOCIATED WITH THE REPAIR AND INTERRUPTION OF SUCH SERVICES.
2. ALL WATERLINES AND APPURTENANCES SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE STANDARDS AND SPECIFICATIONS ESTABLISHED BY THE LOCAL UTILITIES DEPARTMENT HAVING JURISDICTION.
3. THE CONTRACTOR SHALL SUPPLY A TEMPORARY SAFE WATER SERVICE TO ANY BUSINESS THAT WILL HAVE ITS WATER SERVICE INTERRUPTED BY THIS CONSTRUCTION.
4. ANY EXISTING HYDRANTS, VALVES, VALVE BOXES, METER PITS, SERVICE LINES, CURB BOXES OR WATER MAIN THAT ARE DAMAGED OR MUST BE ADJUSTED AND/OR MOVED, MUST BE REPAIRED, ADJUSTED, MOVED AND/OR REPLACED AT CONTRACTOR'S EXPENSE.
5. CONTRACTOR IS RESPONSIBLE FOR COORDINATION OF ALL UTILITIES. CONTRACTOR IS RESPONSIBLE FOR ALL TAP AND OTHER ASSOCIATED FEES.
6. ALL NECESSARY UTILITY PERMITS AND FEES BY CONTRACTOR.
7. ALL PVC SANITARY SEWER PIPES SHALL BE TYPE SDR35.

UTILITY CONTACTS:

ELECTRIC COMPANY:	AEP JEREMY WESTFALL
PHONE NUMBER-	304-541-3306
WATER PROVIDER:	WEST VIRGINIA AMERICAN WATER (WVAWC) HENRY PERKINS
PHONE NUMBER-	304-340-2986
SEWER PROVIDER:	PUTNAM PUBLIC SERVICE DISTRICT TODD PAULEY
PHONE NUMBER-	304-757-6551
GAS COMPANY:	MOUNTAINEER GAS MICHAEL PLYMALE
PHONE NUMBER-	800-834-2070



UTILITY PLAN

20 10 0 20 40

DRAWING SCALE: 1"=20'

CALL BEFORE YOU DIG MUWV MISS UTILITY OF WEST VIRGINIA, INC 1-800-245-4848

TRIAD ENGINEERING, INC.
10541 TEAYS VALLEY ROAD
SCOTT DEPOT, WV 25560
PH: 304.755.0721 FAX: 304.755.1880
OFFICE LOCATIONS
MARYLAND ○ PENNSYLVANIA ○ VIRGINIA ○ WEST VIRGINIA ○ OHIO

BY	DESCRIPTION	DATE	REV. #

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REGISTERED PROFESSIONAL ENGINEER STATE OF WEST VIRGINIA JAMES A. McCOY 17471

DG BTS HUNTINGTON, LLC
HUNTINGTON, WAYNE COUNTY WV

UTILITY PLAN

TRIAD ENGINEERING, INC.
www.triadeng.com

SHEET NUMBER:
C500
PROJECT No.: 04-23-0376

LEGEND

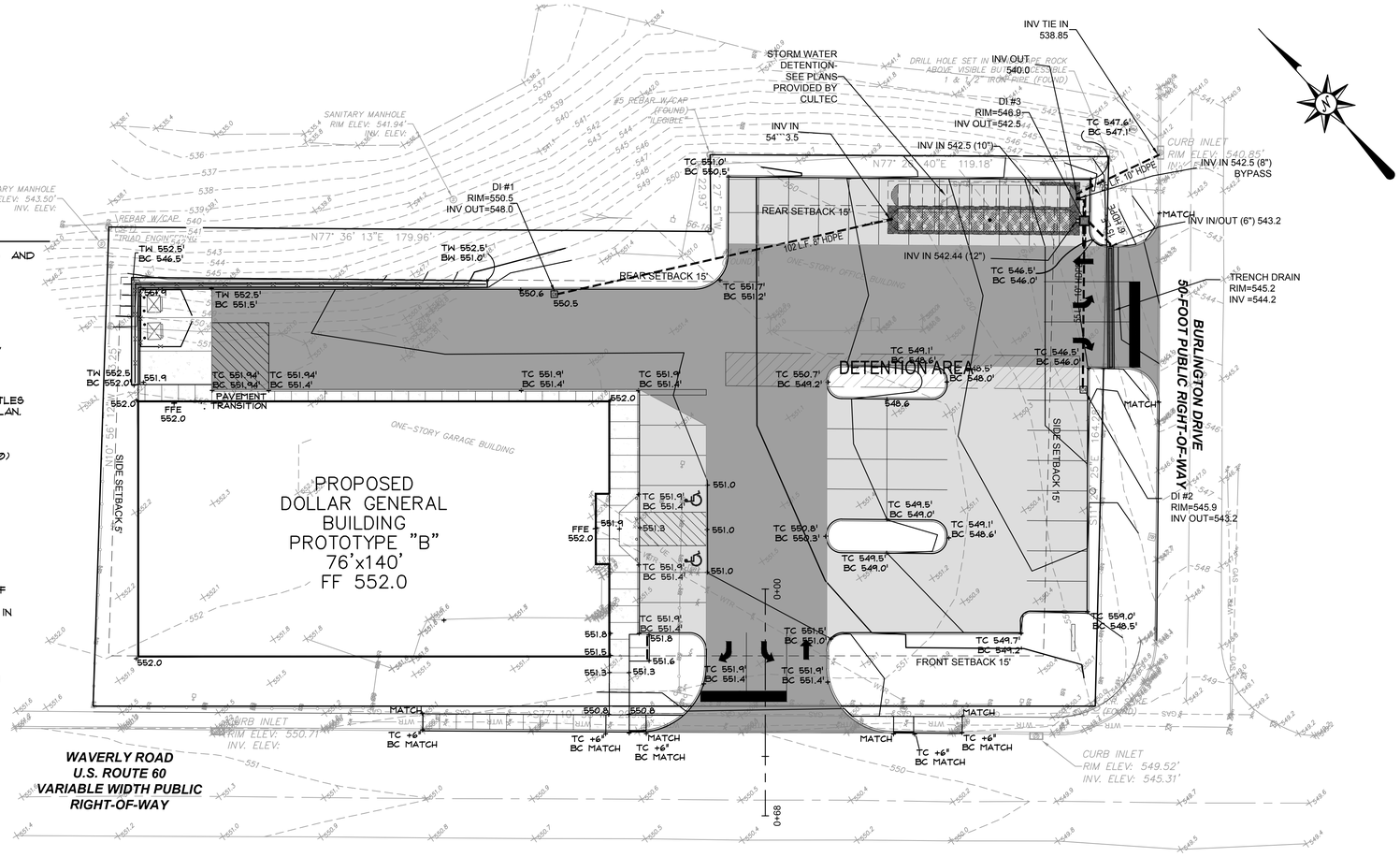
- ASPHALT / ASPHALT (HD)
- ASPHALT / ASPHALT (LD)
- CONCRETE WALK / PAD
- LAWN/LANDSCAPE AREA
- DI — CONCRETE DI
- SITE LIGHTING

GRADING & DRAINAGE NOTES:

1. ALL DISTURBED AND/OR DAMAGED STORM SEWER PIPES AND APPURTENANCES, ASPHALTS, BERMS, LAWN AREAS AND DITCHES SHALL BE RE-SEEDED, REPAIRED AND/OR REPLACED.
2. AT THE LOCATION WHERE PROPOSED WORK ABUTS EXISTING, THE ELEVATIONS OF THE PROPOSED WORK SHALL MATCH ELEVATIONS OF THE EXISTING WORK.
3. THE CONTRACTOR SHALL VERIFY RIM AND INVERT ELEVATIONS AT ALL PROPOSED DROP INLET LOCATIONS.
4. THE CONTRACTOR SHALL MATCH RIM ELEVATIONS OF ALL EXISTING UNDISTURBED WATER METERS, SIGNAL BOXES, ETC. THE CONTRACTOR SHALL REPORT ANY DISCREPANCIES.
5. THE CONTRACTOR SHALL UTILIZE AND COMPLY WITH RECOMMENDATIONS AND STANDARDS FOR EROSION AND SEDIMENT CONTROL AS SET FORTH IN THE WEST VIRGINIA EROSION AND SEDIMENT CONTROL BEST MANAGEMENT PRACTICE MANUAL. THE EROSION AND SEDIMENT CONTROL PLAN SHALL, AT A MINIMUM CONSIST OF 8" STRAW WATTLES ERECTED AT THE CONSTRUCTION LIMITS AND DROP INLET PROTECTION. SEE EROSION AND SEDIMENT CONTROL PLAN.
6. ALL WALKS TO HAVE A MAX. 2% CROSS SLOPE.
7. EARTHWORK SHALL BE COMPACTED THE PERCENTAGES OF MAXIMUM DRY DENSITY (ACCORDING TO AASHTO 1990) AS SHOWN BELOW:

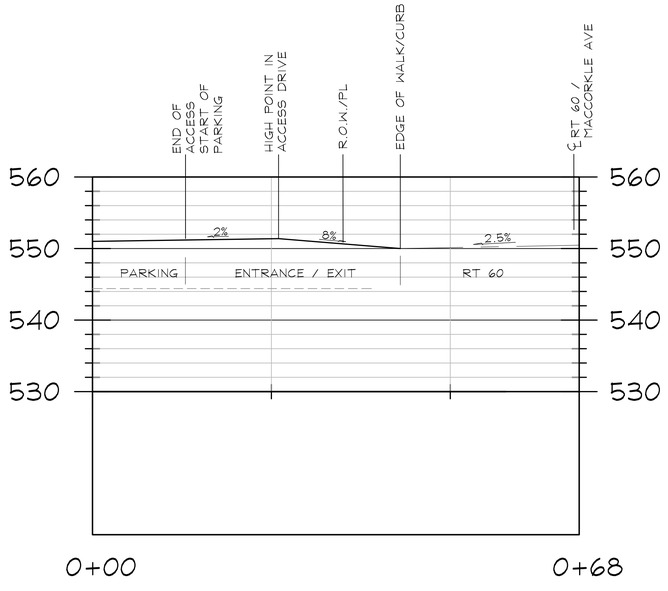
ROADWAYS	98%
BUILDING PADS	100%
PARKING LOTS FOR PASSENGER VEHICLES	95%
PARKING LOTS FOR HEAVY TRUCKS	98%
UTILITY TRENCHES	98%

8. MATERIAL SPECIFICATIONS
REFERENCES IN THE SPECIFICATION AND DRAWINGS TO THE EQUIPMENT, MATERIALS, ARTICLES, OR PATENTED PROCESSES BY TRADE NAME, MAKE, OR CATALOG NUMBER SHALL BE REGARDED AS ESTABLISHING A STANDARD OF QUALITY AND SHALL NOT BE CONSTRUED AS LIMITING COMPETITION. THE ENGINEER MAY, AT HIS OPTION, USE ANY EQUIPMENT, MATERIAL, ARTICLE, OR PROCESS THAT, IN THE OPINION, OF THE ENGINEER, IS EQUAL TO THAT NAMED IN THE SPECIFICATIONS AND DRAWINGS, UNLESS OTHERWISE SPECIFICALLY PROVIDED IN THIS CONTRACT.
9. CONTRACTOR SHALL ADJUST THE TOP ELEVATION OF ALL INLETS, MANHOLES, VALVE BOXES, OR OTHER STRUCTURES AFFECTED BY HIS PROJECT. AS NECESSARY.
10. GRADING SHALL BE PREFORMED TO DRAIN AWAY FROM BUILDINGS AND PREVENT THE POOLING OR COLLECTION OF STORM WATER.



GRADING, DRAINAGE & PAVING PLAN

20 10 0 20 40
DRAWING SCALE: 1"=20'



ENTRANCE PROFILE

10 5 0 10 20
V & H DRAWING SCALE: 1"=10'

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10541 TEAYS VALLEY ROAD
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OFFICE LOCATIONS
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LEE McCOY REGISTERED PROFESSIONAL ENGINEER STATE OF WEST VIRGINIA

DG BTS HUNTINGTON, LLC
HUNTINGTON, WAYNE COUNTY WV

GRADING, DRAINAGE & PAVING PLAN

TRIAD ENGINEERING, INC.
www.triadeng.com

SHEET NUMBER:
C600
PROJECT No.: 04-23-0376

GENERAL LANDSCAPE NOTES:

A. GRADING NOTES:

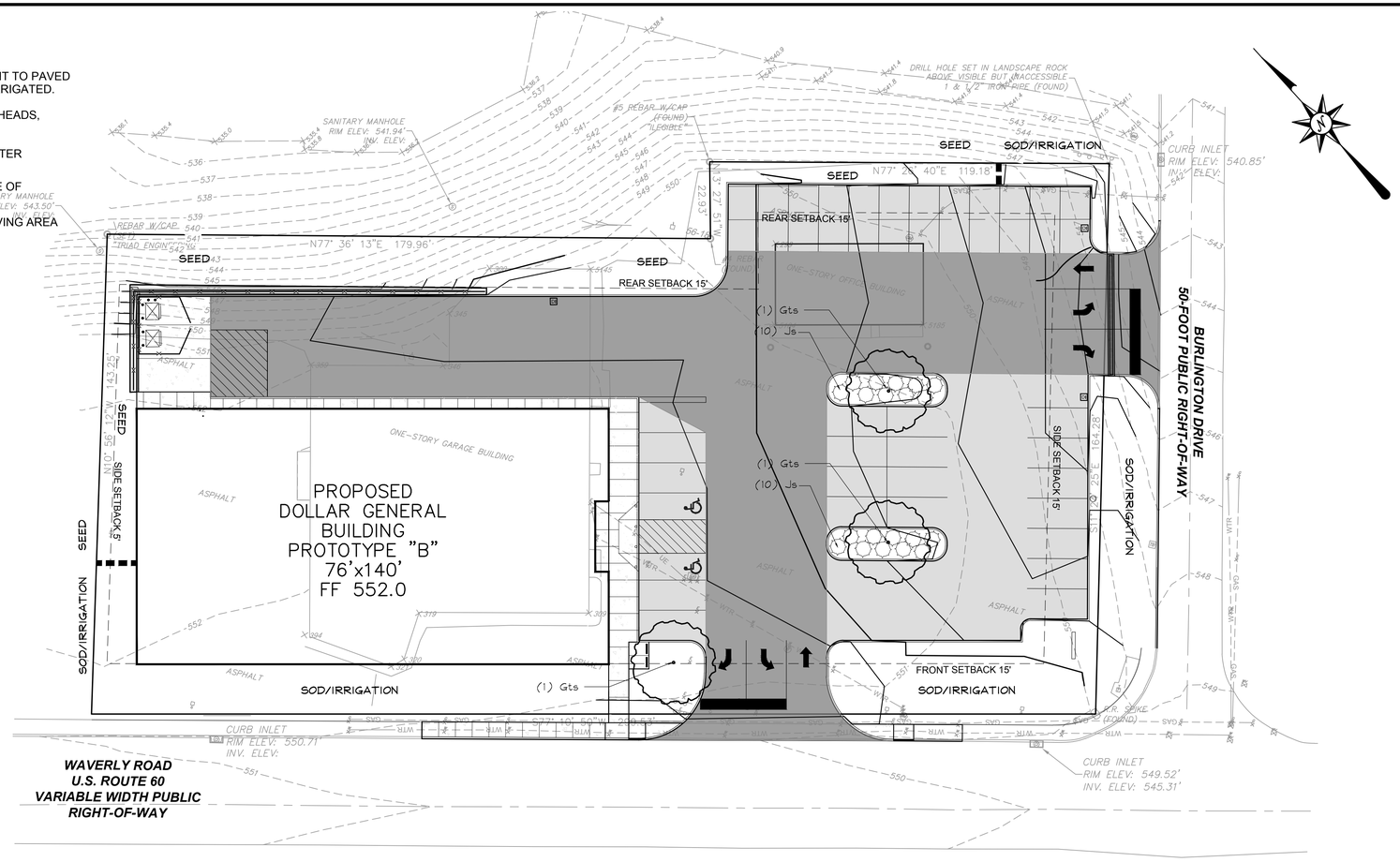
1. CONTRACTOR TO GRADE ALL AREAS SHADED IN THE PLAN, INCLUDING ROW.
2. TOP SOIL SHALL BE STRIPPED FROM ALL CUT AND FILL AREAS, STOCKPILED AND REDISTRIBUTED OVER GRADED AREAS. PROVIDE EROSION AND SEDIMENTATION CONTROLS AROUND STOCKPILES DURING CONSTRUCTION.
3. TILL SOIL TO A DEPTH OF 4" MINIMUM.
4. REMOVE ALL ROCKS LARGER THAN 1" MEASURED IN LARGEST DIRECTION.
5. GRADE ALL AREAS TO MAINTAIN POSITIVE SLOPE AWAY FROM BUILDING.
6. ALL GRADED AREAS TO RECEIVE SEED OR SOD, TOP SOIL, STRAW AND WATER UNTIL A HEALTHY STAND OF GRASS IS OBTAINED.

B. LAWN SEEDING AND SODDING NOTES:

1. ALL LAWNS FROM FACE OF THE BUILDING AND ON THE SIDE WHERE THERE IS PARKING OR A STREET ARE REQUIRED TO BE FULLY SODDED. ALL OTHER LANDSCAPE AREAS TO RECEIVE SEED.
2. AREAS TO RECEIVE SEED OR SOD SHALL BE CLEAN OF DEBRIS AND FREE OF WEEDS.
3. SEED MIX TO BE DROUGHT TOLERANCE FESCUE OR REGIONAL SPECIFIC BLEND. $\frac{1}{4}$ TO $\frac{1}{3}$ OF THE SEED MIXTURE TO BE ANNUAL RYE TO AIDE IN LIMITING EROSION OF PERENNIAL SEED DURING GERMINATION.
4. STRAW SHALL BE THRESHED STRAW OF HAY, OATS, WHEAT, BARLEY, OR RYE. SPREAD AT A RATE OF 2 1/2 TONS PER ACRE. STRAW, NETTING, AND OTHER ANTI-EROSION MATERIALS TO BE REMOVED AFTER ESTABLISHED LAWN.
5. MAINTENANCE SHALL BEGIN IMMEDIATELY AFTER SEEDING. WATER REGULARLY TO KEEP LAWN AREAS MOIST TO MAXIMIZE GERMINATION AND MAINTAIN OPTIMUM GROWTH AND SURVIVAL TO ACHIEVE AN ACCEPTABLE STAND OF ESTABLISHED LAWN PRIOR TO THE STORE POSSESSION DATE, FREE OF ERODED OR BARE AREAS.

D. IRRIGATION NOTES:

1. ALL LANDSCAPE AREAS AND LAWNS ADJACENT TO PAVED AREAS AND/OR STREETS ARE TO BE FULLY IRRIGATED.
2. IRRIGATION SYSTEM TO INCLUDE ALL SPRAY HEADS, VALVES AND CONTROLLERS.
3. A SEPARATE METER AND BACKFLOW PREVENTER WILL BE REQUIRED.
4. LOCATE HEAD A MINIMUM OF 2'-0" FROM EDGE OF ASPHALT/CURB.
5. LOCATE RAINBIRD CONTROL PANEL IN RECEIVING AREA NEXT TO ELECTRICAL PANELS.



LANDSCAPE PLAN

20 10 0 20 40

DRAWING SCALE: 1"=20'

QTY. Total	KEY	BOTANICAL NAME	COMMON NAME	SIZE	COMMENT
3	Gts	Gledirsia tricantohos inermis 'Skycole Skyline"	Skyline Honeylocust	2.0" CAL.,B&B	
20	Js	Juniperous sabina 'Buffalo'	Buffalo Juniper	#3 CONT.	SPACING(AS SHOWN ON PLAN)

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1-800-245-4848

TRIAD ENGINEERING, INC.
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SCOTT DEPOT, WV 25560
PH: 304.755.0721 FAX: 304.755.1880
OFFICE LOCATIONS
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REGISTERED PROFESSIONAL ENGINEER
JEFFREY MCCOY
14471
STATE OF WEST VIRGINIA

DG BTS HUNTINGTON, LLC
HUNTINGTON, WAYNE COUNTY WV

LANDSCAPE PLAN

TRIAD ENGINEERING, INC.
www.triadeng.com

SHEET NUMBER:
C700

PROJECT No.: 04-23-0376

EROSION AND SEDIMENT CONTROL NOTES:

UNLESS OTHERWISE INDICATED, ALL VEGETATIVE AND STRUCTURAL EROSION AND SEDIMENT CONTROL PRACTICES WILL BE CONSTRUCTED AND MAINTAINED ACCORDING TO THE STANDARDS AND SPECIFICATIONS OF THE WEST VIRGINIA EROSION AND SEDIMENT CONTROL BMP MANUAL. THE PLAN APPROVING AUTHORITY MUST BE NOTIFIED ONE WEEK PRIOR TO THE PRE-CONSTRUCTION CONFERENCE, ONE WEEK PRIOR TO THE COMMENCEMENT OF LAND DISTURBING ACTIVITY, AND ONE WEEK PRIOR TO THE FINAL INSPECTION. ALL EROSION AND SEDIMENT CONTROL MEASURES ARE TO BE PLACED PRIOR TO OR AS THE FIRST STEP IN CLEARING. A COPY OF THE APPROVED EROSION AND SEDIMENT CONTROL PLAN SHALL BE MAINTAINED AT THE SITE AT ALL TIMES. PRIOR TO COMMENCING LAND DISTURBING ACTIVITIES IN AREAS OTHER THAN INDICATED ON THESE PLANS (INCLUDING, BUT NOT LIMITED TO, OFF-SITE BORROW OR WASTE AREAS), THE CONTRACTOR SHALL SUBMIT A SUPPLEMENTARY EROSION CONTROL PLAN TO THE OWNER FOR REVIEW AND APPROVAL BY THE PLAN APPROVING AUTHORITY. THE CONTRACTOR IS RESPONSIBLE FOR INSTALLATION OF ANY ADDITIONAL EROSION CONTROL MEASURES NECESSARY TO PREVENT EROSION SEDIMENTATION AS DETERMINED BY THE PLAN APPROVING AUTHORITY.

SITE GRADING IS TO DRAIN TO THE SEDIMENT TRAPPING DEVICES AT ALL TIMES DURING LAND DISTURBING ACTIVITIES AND DURING SITE DEVELOPMENT UNTIL FINAL STABILIZATION IS ACHIEVED. ALL EROSION AND CONTROL STRUCTURES MUST BE INSPECTED AT LEAST ONCE EVERY FOUR CALENDAR DAYS AND AFTER EVERY STORM EVENT OF 0.25 INCHES OR GREATER. ANY NECESSARY REPAIRS OR CLEANUP TO MAINTAIN THE EFFECTIVENESS OF THE EROSION CONTROL DEVICES SHALL BE MADE IMMEDIATELY. INITIAL EFFORTS SHOULD BE TO LIMIT THE AMOUNT OF AREA DISTURBED BY MAINTAINING AS MUCH OF THE ORIGINAL VEGETATIVE COVER AS POSSIBLE. SEDIMENT CONTROL MEASURES SHALL REMAIN ACTIVE UNTIL ALL DISTURBED AREAS HAVE BEEN SATISFACTORY STABILIZED. ALL STATE AND LOCAL REQUIREMENTS SHALL BE MET CONCERNING FENCING AND SIGNS WARNING THE PUBLIC OF THE HAZARDS OF SOFT, SATURATED SEDIMENT AND FLOOD WATERS. ALL DISTURBED AREAS SHALL BE STABILIZED WITHIN 4 DAYS OF REACHING FINAL GRADE, AND IF A DISTURBED PART OF THE PROJECT WILL NOT BE WORKED FOR 14 DAYS OR MORE, THAT AREA SHALL BE STABILIZED WITHIN 4 DAYS AFTER CONSTRUCTION ACTIVITIES HAVE TEMPORARILY CEASED.

THE CONTRACTOR SHALL PROVIDE A DETAILED PLAN AND SCHEDULE FOR ALL ELEMENTS OF THE EROSION CONTROL PLAN. THE PLAN SHOULD BE POSTED AT THE JOB SITE AND STRICTLY FOLLOWED. THE MINIMUM STANDARD OF PERFORMANCE WILL BE A PLAN THAT REQUIRES THAT AN INSPECTION OF ALL PLAN ELEMENTS BE CONDUCTED AT LEAST ONCE EVERY FOUR DAYS, UPON REPORT OF AN OBSERVED FAILURE, OR WITHIN 24 HOURS AFTER ANY STORM EVENT GREATER THAN 0.25 INCHES OF RAIN PER 24 HOUR PERIOD. SPECIAL ATTENTION SHOULD BE GIVEN TO AREAS OF EQUIPMENT FUELING AND CLEANING. MITIGATION MEASURES SUCH AS WATER DIVERSION AND CONTAINMENT, ETC. SHOULD BE EMPLOYED TO MINIMIZE THE POTENTIAL FOR CONTAMINANTS TO REACH SURFACE WATERS.

A. VEGETATIVE CONTROL

TEMPORARY VEGETATIVE CONTROL MEASURES SHALL BE IN ACCORDANCE WITH THE WEST VIRGINIA EROSION AND SEDIMENT CONTROL BMP MANUAL. PERMANENT VEGETATIVE CONTROL SHALL BE PROVIDED AS STATED IN PROJECT SPECIFICATIONS, SECTION 329200, TURF AND GRASSES.

B. STRUCTURAL CONTROL

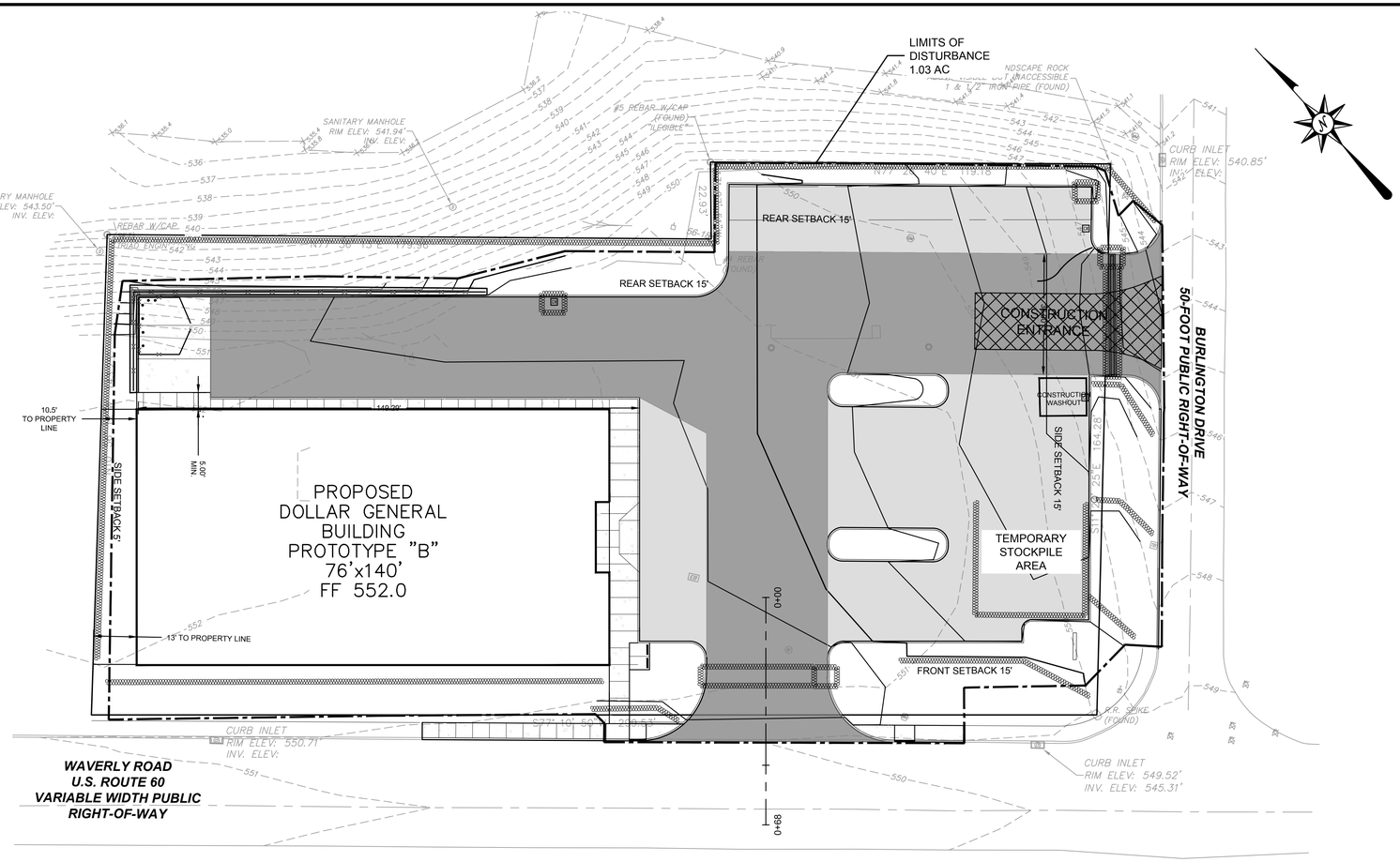
TEMPORARY STRUCTURAL CONTROL IS PROVIDED BY INSTALLING COMPOST FILTER SOCK AS CUT/FILL PROGRESSES, THE CONTRACTOR SHALL MAINTAIN POSITIVE SLOPE TO SEDIMENT TRAPPING DEVICES. ALL SEDIMENT LADEN WATER SHALL PASS THROUGH AN APPROPRIATE SEDIMENT TRAPPING DEVICE.

TEMPORARY STONE CONSTRUCTION ENTRANCE- THE AREA OF THE ENTRANCE MUST BE EXCAVATED TO A MINIMUM OF 3 INCHES AND MUST BE CLEARED OF ALL VEGETATION, ROOTS, AND OTHER OBJECTIONABLE MATERIAL. THE FILTER FABRIC UNDERLINER WILL THEN BE PLACED THE FULL WIDTH AND LENGTH OF THE ENTRANCE. FOLLOWING THE INSTALLATION OF THE FILTER CLOTH, THE STONE SHALL BE PLACED TO THE SPECIFIED DIMENSIONS. THE FILTER CLOTH UTILIZED SHALL BE WOVEN OR NONWOVEN FABRIC CONSISTING OF CONTINUOUS CHAIN POLYMERIC FILAMENTS OR YARNS OF POLYESTER. THE FABRIC SHALL BE INERT TO COMMONLY ENCOUNTERED CHEMICALS AND HYDROCARBONS, BE MILDEW AND ROT RESISTANT, AND CONFORM TO THE PHYSICAL PROPERTIES NOTED IN THE CONSTRUCTION SPECIFICATIONS.

COMPOST FILTER SOCK ARE TO BE PERPENDICULAR TO THE FLOW DIRECTION AND PARALLEL TO THE SLOPE CONTOUR. NARROW TRENCHES SHOULD BE DUG ACROSS THE SLOPE TO A DEPTH OF 3 TO 5 INCHES. INSTALL THE WATTLES SNUGLY INTO THE TRENCHES AND ABUT TIGHTLY END-TO-END. DO NOT OVERLAP THE ENDS. INSTALL STAKES AT EACH END OF THE WATTLE, AND AT 4-FOOT CENTERS ALONG THE ENTIRE LENGTH OF THE WATTLE. AT A MINIMUM, WOODEN STAKES SHOULD BE APPROXIMATELY ¾X¾X24 INCHES. WILLOW CUTTINGS OR 3/8-INCH REBAR CAN BE USED FOR STAKES. STAKES SHOULD BE DRIVEN THROUGH THE MIDDLE OF THE WATTLE, LEAVING 2 TO 3 INCHES OF THE STAKE PROTRUDING ABOVE THE WATTLE.

SEQUENCE OF CONSTRUCTION

- INSTALL TEMPORARY SEDIMENT CONTROL PERIMETER COMPOST FILTER SOCK.
- INSTALL TEMPORARY CONSTRUCTION ENTRANCE.
- GRADE PROPOSED STOCKPILE AREA AND CONCRETE WASHOUT PAD AREA TO LESS THAN 5% GRADE.
- ESTABLISH STOCKPILE AREA WITH EROSION CONTROL.
- CLEAR AND GRUB SITE. SEGREGATE THE TOP SOIL WHERE APPLICABLE.
- ROUGH GRADE SITE EXCEPT FOR STOCK PILE AND CONCRETE WASHOUT AREAS.
- EXCAVATE FOR UTILITIES, STORM WATER SYSTEM AND BUILDING FOOTERS.
- INSTALL PERMANENT STORM WATER SYSTEM WITH INLET PROTECTION.
- INSTALL CONCRETE WASHOUT PAD.
- ASPHALTING AND SIDEWALK CONSTRUCTION.
- BACKFILL NECESSARY LOCATIONS WITH SUBSOIL MATERIAL REMOVED DURING EXCAVATION.
- FINE GRADE SITE (EXCEPT FOR STOCK PILE AND CONCRETE WASHOUT AREAS), REPLACING SEGREGATED TOPSOIL.
- REMOVE STOCK PILE AND CONCRETE WASHOUT AREAS. ROUGH AND FINE GRADE THESE AREAS.
- INSTALL ASPHALT.
- LANDSCAPE, SEED AND MULCH.
- REMOVE EROSION CONTROL MEASURES AFTER STABILIZATION HAS BEEN ESTABLISHED.



EROSION CONTROL PLAN

20 10 0 20 40

DRAWING SCALE: 1"=20'

LEGEND

- ASPHALT / ASPHALT (HD)
- ASPHALT / ASPHALT (LD)
- CONCRETE WALK / PAD
- LAWN/LANDSCAPE AREA
- 18" COMPOST FILTER SOCK
- COMPOST FILTER SOCK INLET PROTECTION
- LIMITS OF DISTURBANCE
- CONSTRUCTION ENTRANCE
- DI CONCRETE DI
- SITE LIGHTING

TRIAD ENGINEERING, INC.
10541 TEAYS VALLEY ROAD
SCOTT DEPOT, WV 25560
PH: 304.755.0721 FAX: 304.755.1880
OFFICE LOCATIONS
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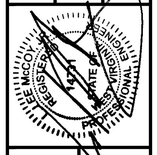


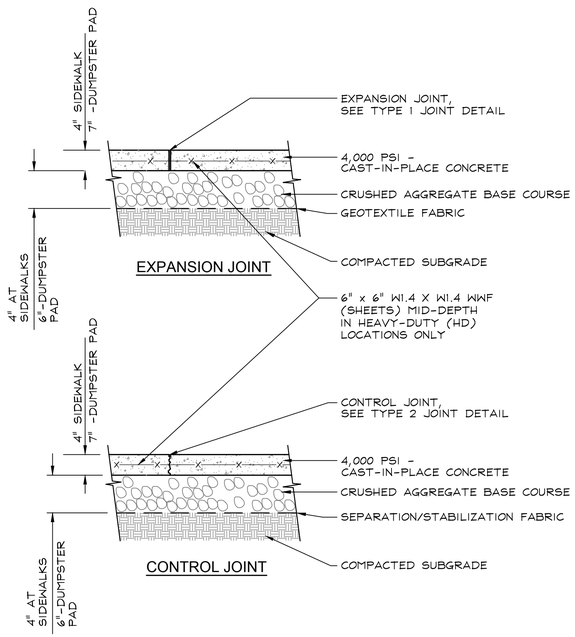
DG BTS HUNTINGTON, LLC
HUNTINGTON, WAYNE COUNTY WV

EROSION & SEDIMENT CONTROL PLAN

TRIAD ENGINEERING, INC.
www.triadeng.com

SHEET NUMBER:
C800

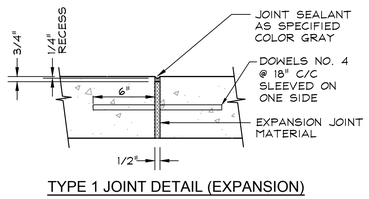
PROJECT No.: 04-23-0376



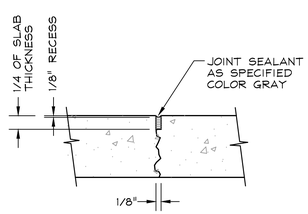
1A C900 N.T.S.

CONCRETE DETAILS

NOTES:
1. 1/2" EXPANSION JOINT AT ALL CURBS, WALKS, WALLS AND OTHER FIXED OBJECTS.
2. CONTROL JOINT AT 15' O.C. MAX. OR AS SHOWN ON DRAWINGS.
3. CROSS SLOPE AT 1/4" PER FOOT MIN. TO DRAIN.



TYPE 1 JOINT DETAIL (EXPANSION)

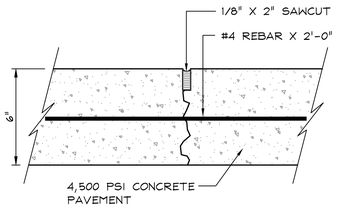


TYPE 2 JOINT DETAIL (CONTROL)

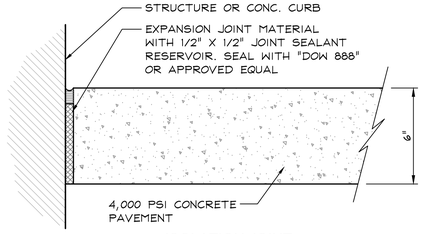
1B C900 N.T.S.

CONCRETE WALK JOINT DETAILS

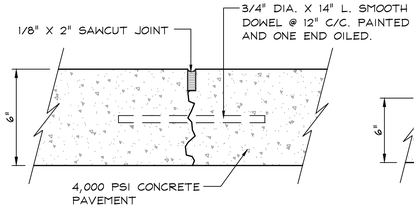
NOTES:
1. NO JOINT DIMENSION SHALL EXCEED 15 FEET IN EITHER DIRECTION.
2. ALL CONTROL JOINTS THAT TRANSVERSE WITH THE FLOW OF CONCENTRATED TRAFFIC SHALL BE DOWELED JOINTS. SEE DETAILS BELOW.
3. ALL CONTROL JOINTS LONGITUDINAL WITH THE FLOW OF CONCENTRATED TRAFFIC SHALL BE TIE BAR JOINT SEE DETAILS BELOW.
4. CONSTRUCTION JOINTS SHALL BE INSTALLED AT ALL POUR STOPS AND BULKHEAD AREAS.
5. THE JOINT LAYOUT SHOWN IS A SUGGESTED LAYOUT BASED ON A 15 FT. X 15 FT. MAXIMUM GRID DIMENSION.



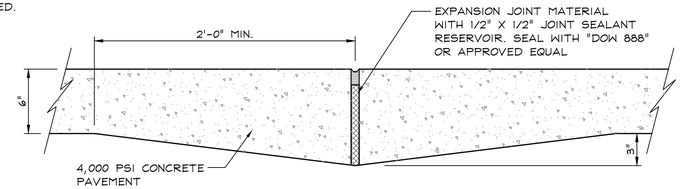
TIED LONGITUDINAL CONTROL JOINT



ISOLATION JOINT



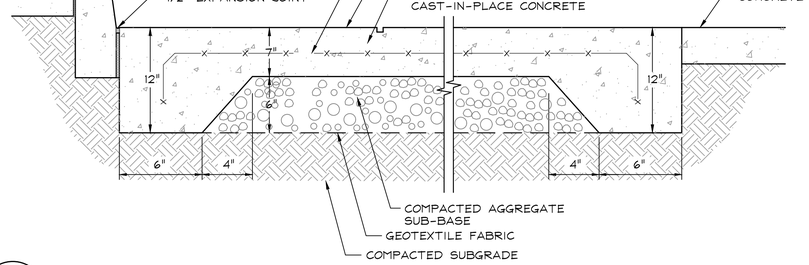
DOWELED TRANSVERSE CONTROL JOINT



THICKENED EDGE EXPANSION JOINT

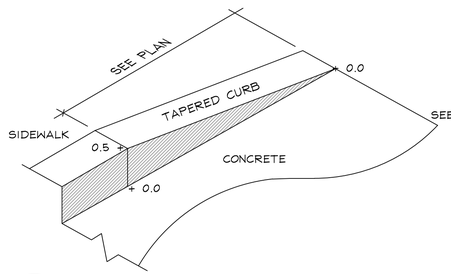
2 C900 N.T.S.

CONCRETE PAVEMENT JOINT DETAILS



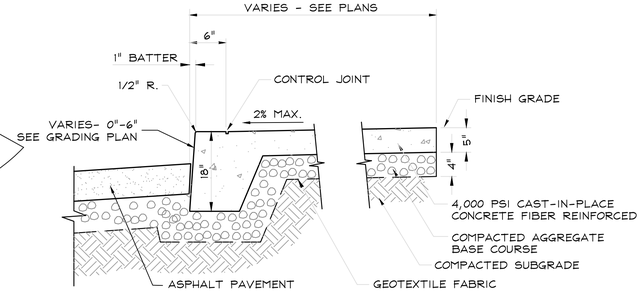
3 C900 N.T.S.

CONCRETE DUMPSTER PAD



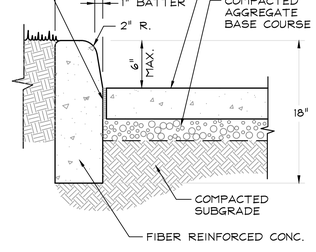
4 C900 N.T.S.

TAPERED CURB



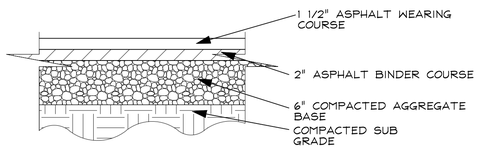
5 C900 N.T.S.

MONOLITHIC CURB & SIDEWALK



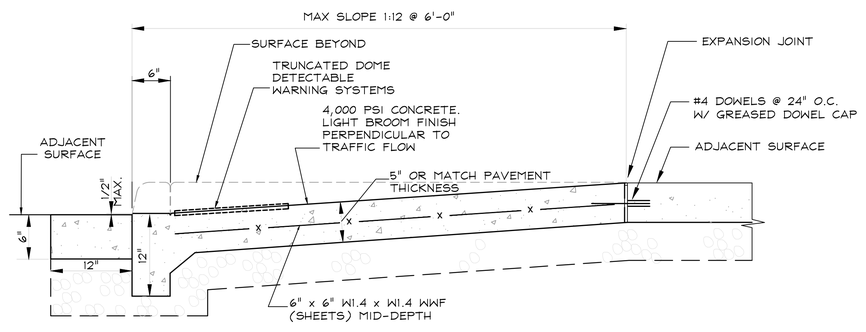
6 C900 N.T.S.

CONCRETE CURB

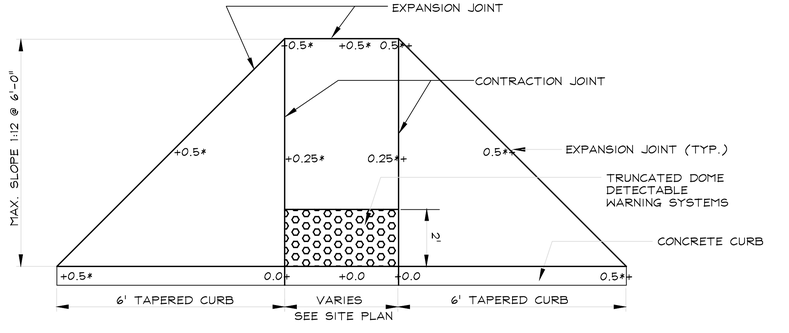


7 C900 N.T.S.

ASPHALT PAVEMENT



SECTION VIEW



PLAN VIEW

8 C900 N.T.S.

ACCESSIBILITY RAMP

TRIAD ENGINEERING, INC.
10541 TEAYS VALLEY ROAD
SCOTT DEPOT, WV 25560
PH: 304.755.0721 FAX: 304.755.1880
OFFICE LOCATIONS
MARYLAND ◇ PENNSYLVANIA ◇ VIRGINIA ◇ WEST VIRGINIA ◇ OHIO

REV. #	DATE	DESCRIPTION	BY

CADD FILE: 23-0595 DETAILS.dwg
DRAWN BY: JTM
CHECKED BY: JHY
DATE: 12/13/2023
SCALE: NOTED

DG BTS HUNTINGTON, LLC
HUNTINGTON, WAYNE COUNTY WV

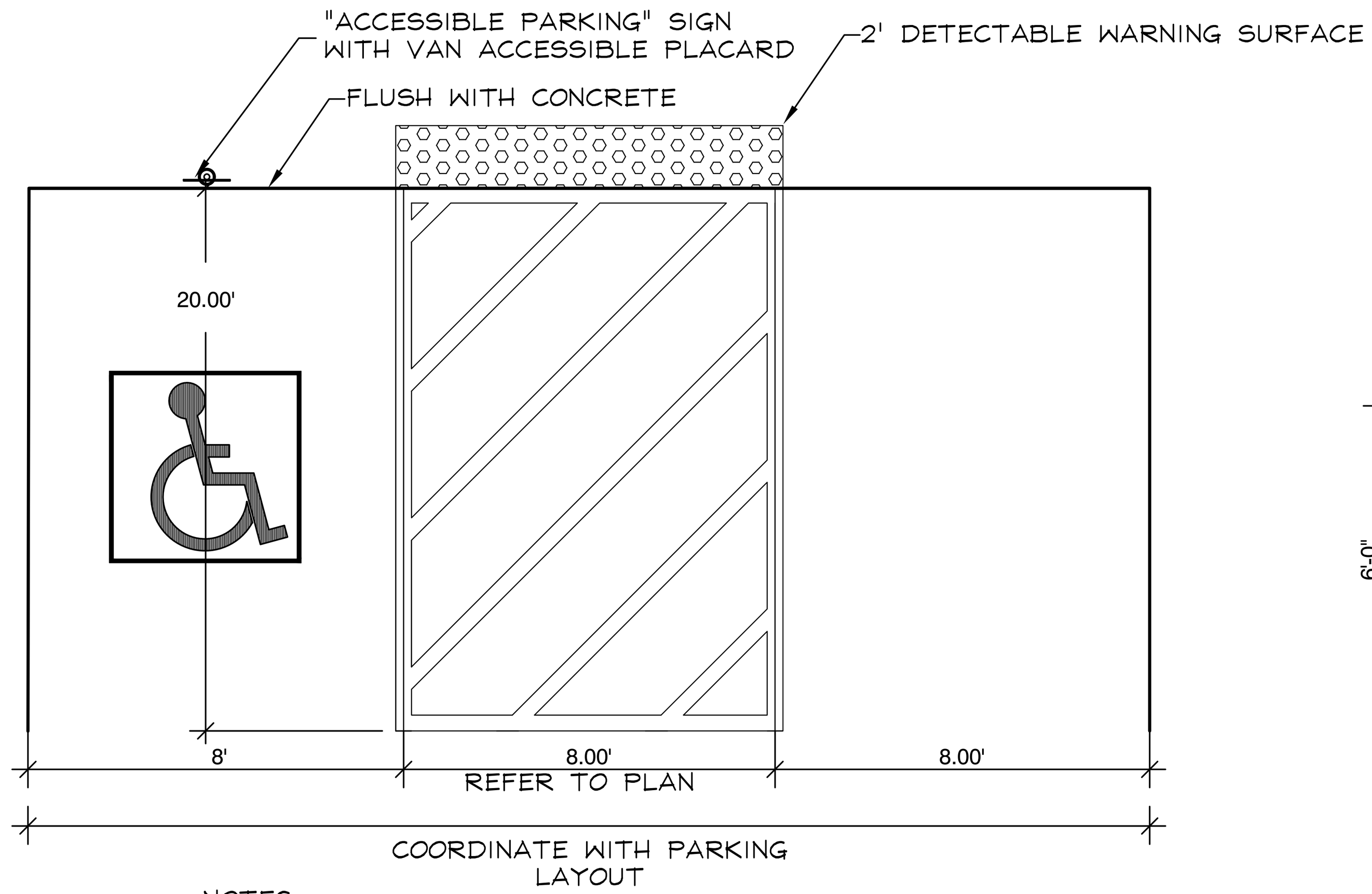
DETAILS

TRIAD ENGINEERING, INC.
www.triadeng.com

SHEET NUMBER: C900

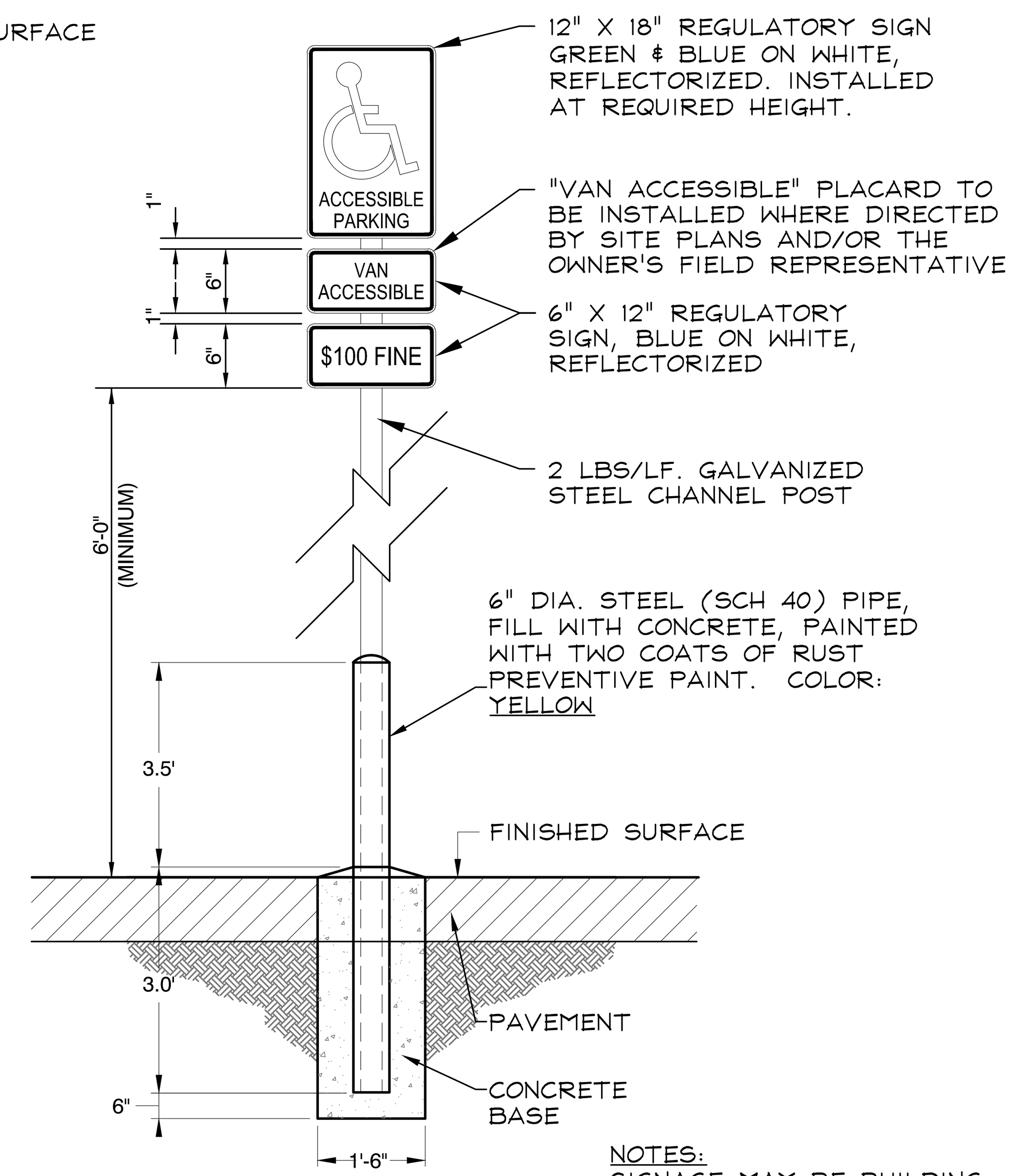
PROJECT No.: 04-23-0376

Plotted by: jyoung
Y:\tsk_3a_04-2023_5_ 04-23-0595 dg bts huntington cnv\civil\cadd\23-0595 details.dwg



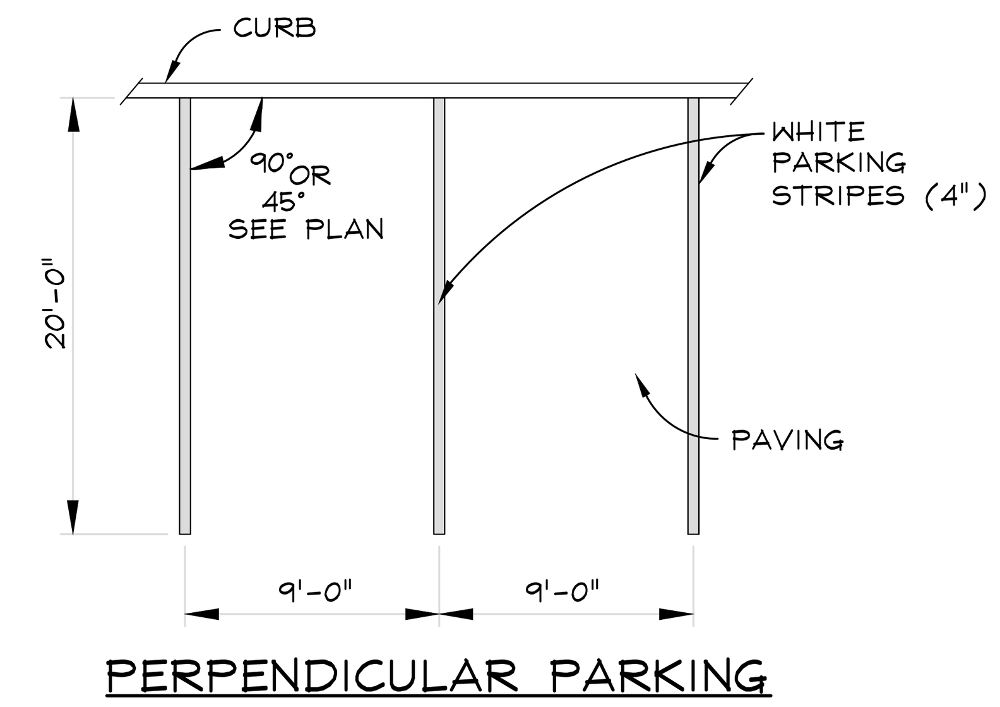
NOTES:
1. PAVEMENT MARKINGS TO CONFORM TO ADA STANDARDS 502.3.3 MARKING AND REQUIREMENTS BY STATE AND LOCAL LAWS AND REGULATIONS.
2. PAVEMENT MARKINGS TO BE PAINTED BLUE WITH HIGHWAY GRADE PAINT CONFORMING TO THE STANDARDS SET FOR THE MUTCD.

1 C901 ACCESSIBILITY PARKING
N.T.S.



NOTES:
SIGNAGE MAY BE BUILDING MOUNTED PER APPROVAL BY OWNER/ARCHITECT

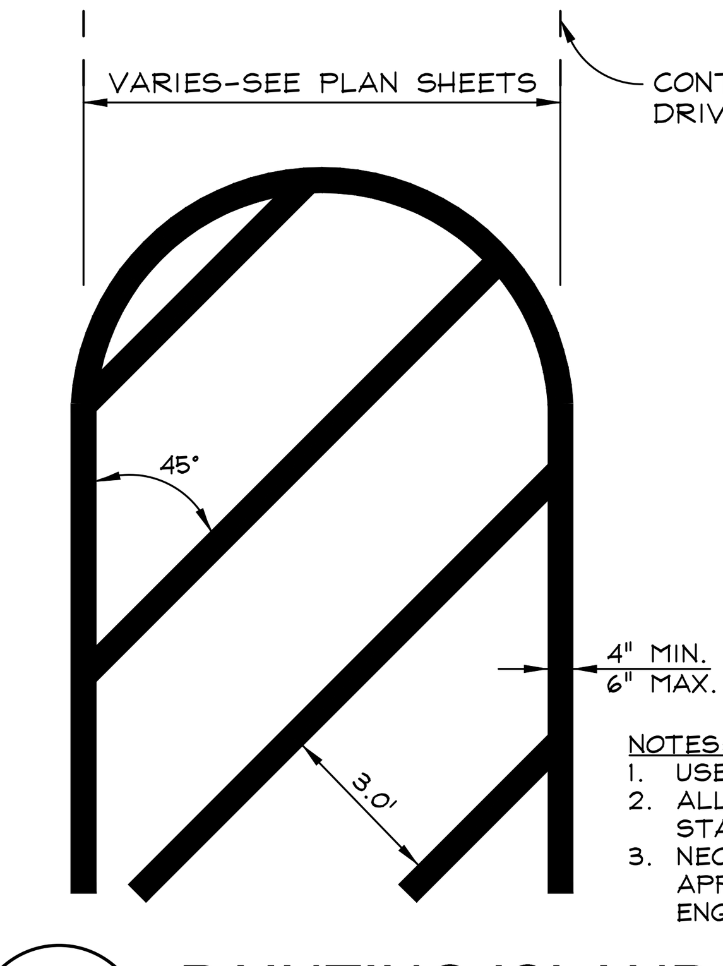
2 C901 BOLLARD MOUNTED ACCESSIBILITY SIGN
N.T.S.



PERPENDICULAR PARKING

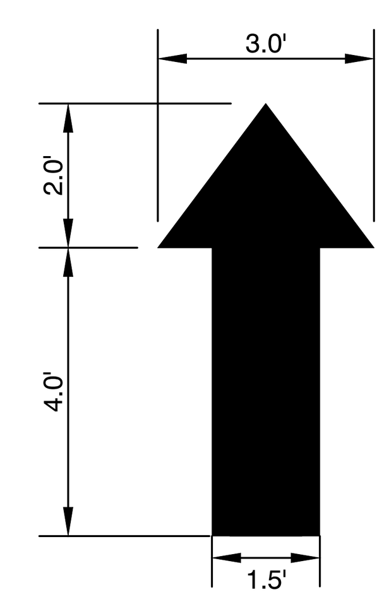
NOTES:
1. PERMANENT PAVEMENT STRIPING SHALL MEET OR EXCEED THE REQUIREMENTS OF THE WEST VIRGINIA DEPARTMENT OF TRANSPORTATION DESIGN MANUAL.
2. PAVEMENT-MARKING PAINT: ALKYD-RESIN TYPE, LEAD AND CHROMATE FREE, READY MIXED, COMPLYING WITH AASHTO M248. COLORS: ACCESSIBLE SPACES- BLUE DRIVE LANES, ARROWS, STALL STRIPING - WHITE

3 C901 PARKING STALL LAYOUT
N.T.S.



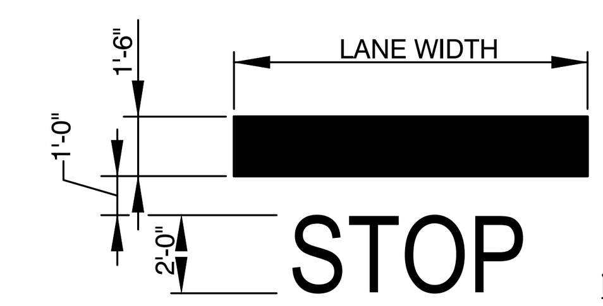
NOTES:
1. USE MUTCD WHITE REFLECTIVE PAINT.
2. ALL STRIPING IS TO CONFORM TO THE STANDARDS SET FOR IN THE MUTCD.
3. NECESSARY ADJUSTMENTS MUST BE APPROVED BY THE OWNER AND ENGINEER PRIOR TO INSTALLATION.

4 C901 PAINTING ISLAND STRIPING
N.T.S.



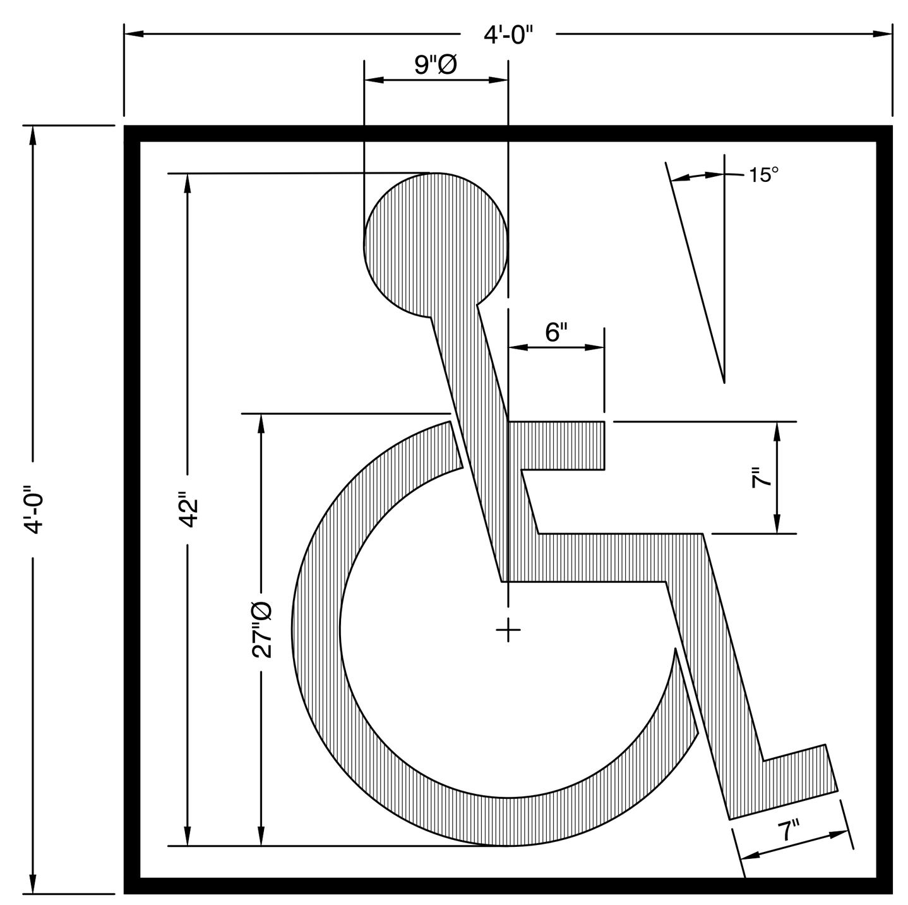
NOTES:
PAVEMENT MARKING TO BE PAINTED WITH 2 COATS OF HIGHWAY GRADE PAINT.
SEE PLAN FOR SPECIFIC LOCATION

6 C901 PAVEMENT MARKING ARROW
N.T.S.



NOTES:
1. PAVEMENT MARKING TO BE THERMOPLASTIC

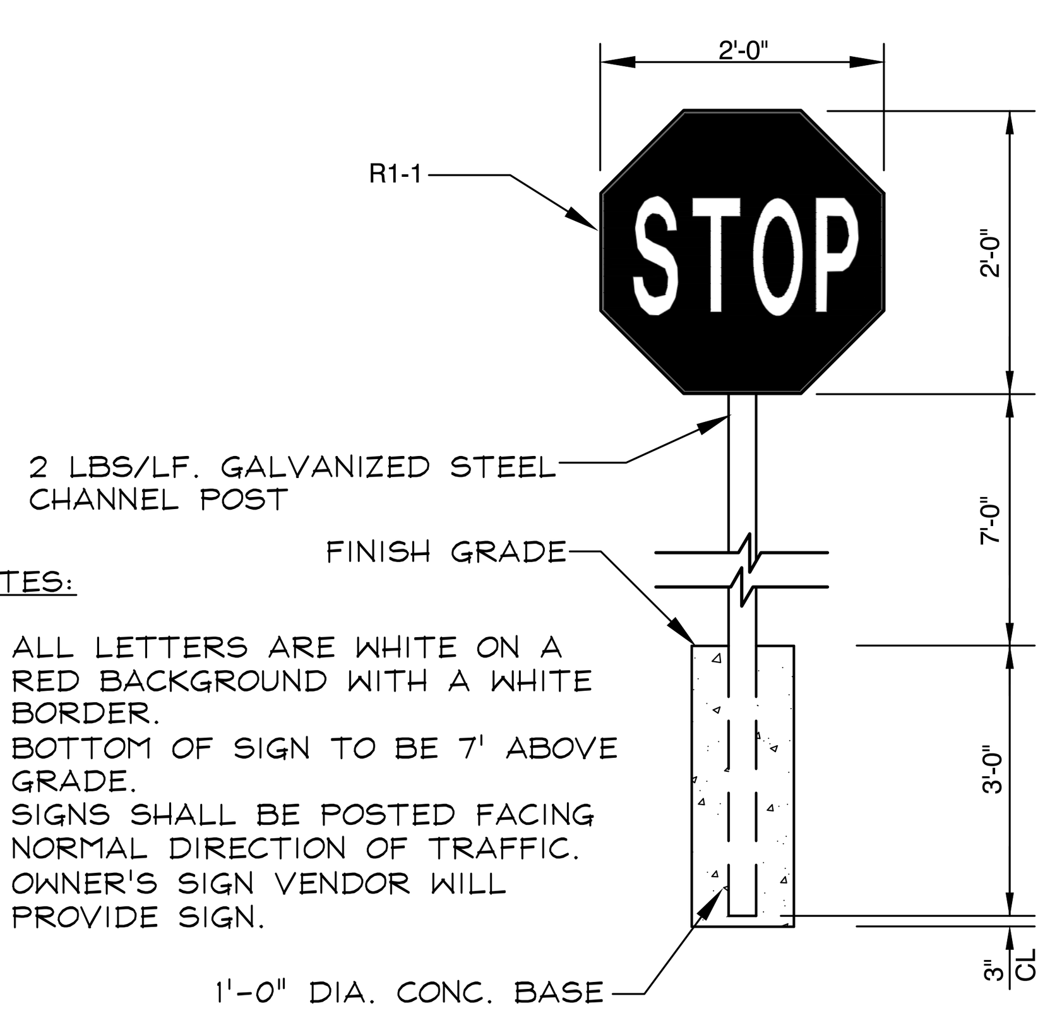
7 C901 STOP BAR
N.T.S.



INTERNATIONAL SYMBOL OF ACCESSIBILITY PER ANSI A117.1-2009 REQUIREMENTS

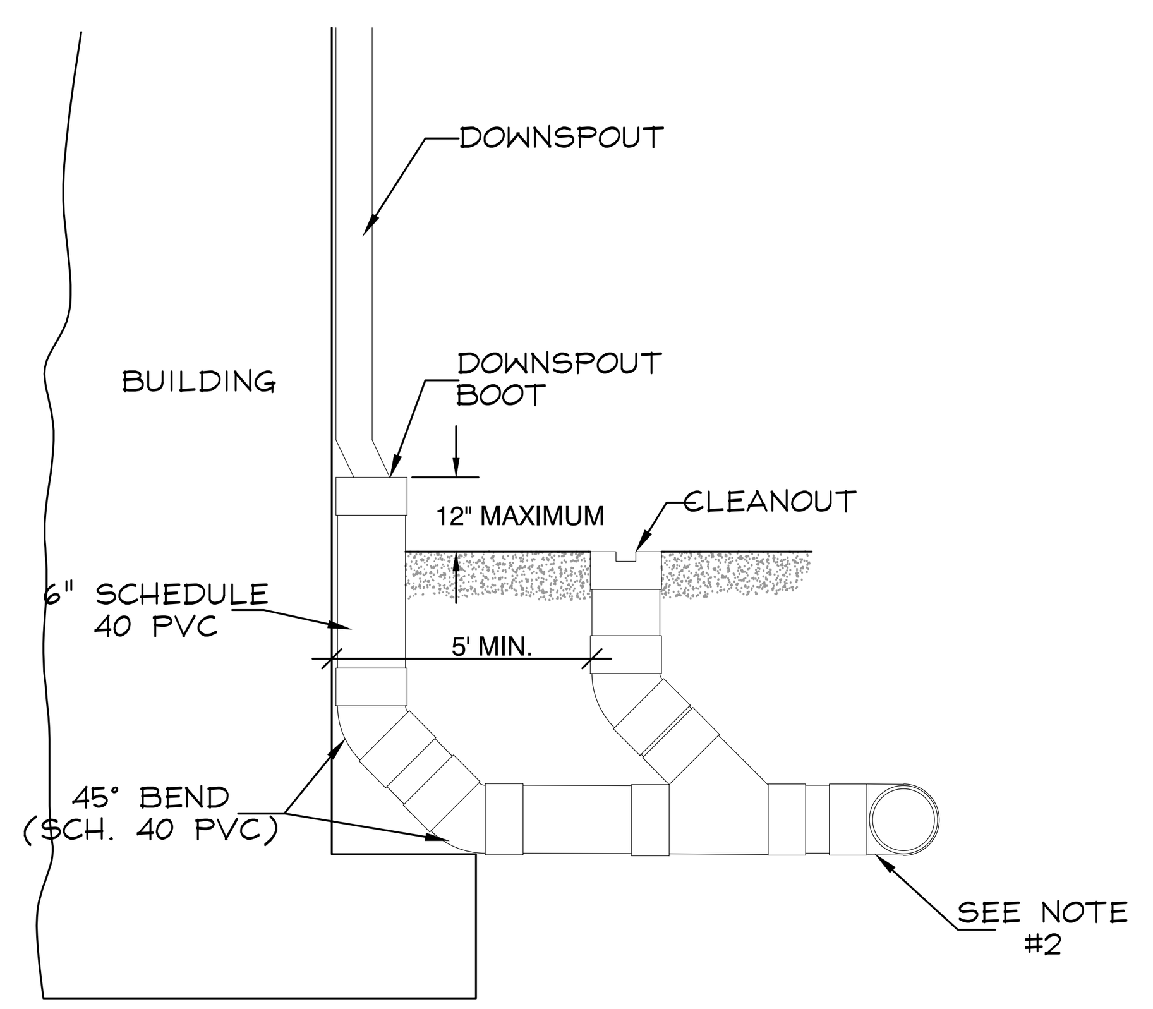
NOTES:
1. PAVEMENT MARKINGS TO CONFORM TO ADA STANDARDS 502.3.3 MARKING AND REQUIREMENTS BY STATE AND LOCAL LAWS AND REGULATIONS.
2. PAVEMENT MARKINGS TO BE PAINTED BLUE WITH HIGHWAY GRADE PAINT.

5 C901 ACCESSIBILITY PARKING SYMBOL
N.T.S.



NOTES:
1. ALL LETTERS ARE WHITE ON A RED BACKGROUND WITH A WHITE BORDER.
2. BOTTOM OF SIGN TO BE 7' ABOVE GRADE.
3. SIGNS SHALL BE POSTED FACING NORMAL DIRECTION OF TRAFFIC.
4. OWNER'S SIGN VENDOR WILL PROVIDE SIGN.

8 C901 STOP SIGN INSTALLATION
N.T.S.



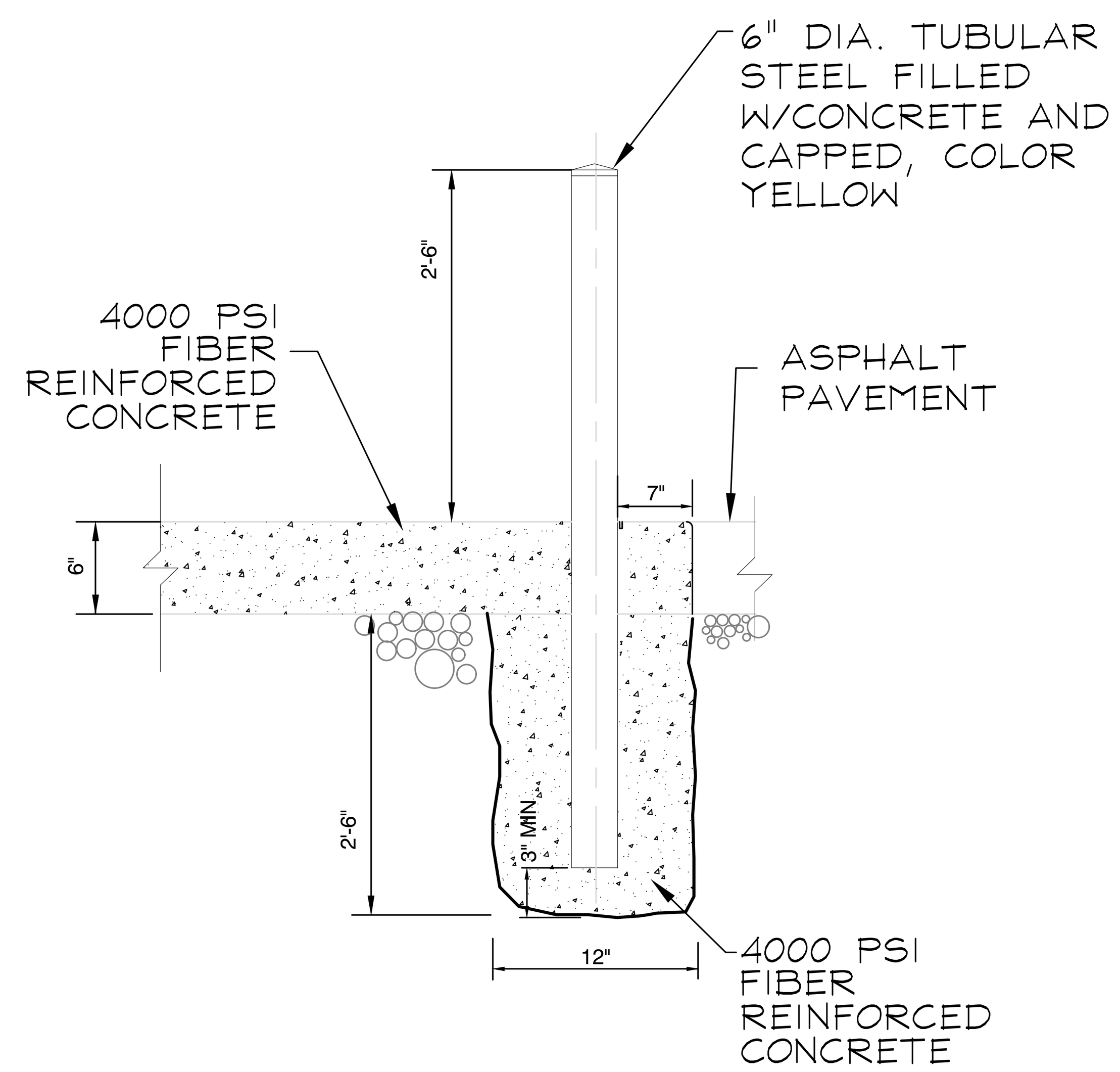
NOTES:
1. FOR ALL DEPTHS OF COVER LESS THAN (2) FEET, PIPE MUST BE SCHEDULE 40 PVC. FOR DEPTH OF COVER GREATER THAN (2) FEET, FLEXIBLE PIPE MAY BE USED. SEE SPECIFICATIONS FOR TYPES OF PIPES.

2. A WATERTIGHT CONNECTION SHALL BE MAINTAINED WITH ANY TRANSITION FROM SCHEDULE 40 PVC PIPE TO ANY OTHER PIPE TYPE.

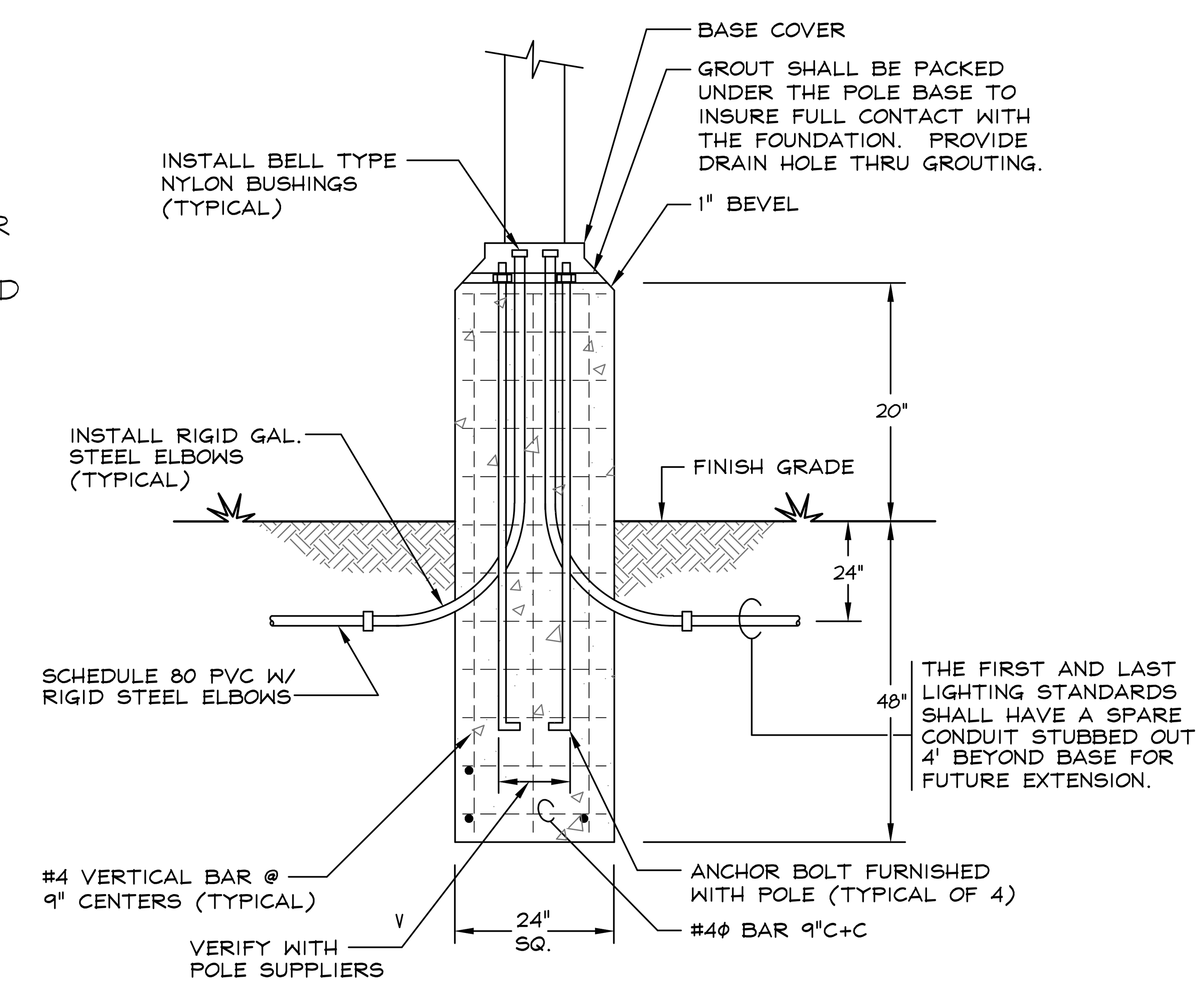
3. THE DOWN SPOUT COLLECTOR DRAIN SHALL BE INSTALLED BEFORE THE DOWN SPOUTS ARE INSTALLED ON THE BUILDING. THE SITE WORK CONTRACTOR SHALL BE RESPONSIBLE FOR ALL WORK TO AND INCLUDING DOWNSPOUT BOOT. THE BUILDING CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONNECTION AT THE POINT OF THE DOWNSPOUT BOOT.

4. ROOF DRAIN LEADERS MINIMUM SLOPE IS 1.00%

9 C901 DOWNSPOUT COLLECTOR
N.T.S.



10 C901 PARKING BOLLARD DETAIL
N.T.S.



11 C901 LIGHT POLE BASE
N.T.S.

TRIAD ENGINEERING, INC.
10541 TEAYS VALLEY ROAD
SCOTT DEPOT, WV 25560
PH: 304.755.0721 FAX: 304.755.1880
OFFICE LOCATIONS
MARYLAND ○ PENNSYLVANIA ○ VIRGINIA ○ WEST VIRGINIA ○ OHIO

REV. #	DATE	DESCRIPTION	BY

CADD FILE:	DRAWN BY:	CHECKED BY:
23-0595 DETAILS.dwg	JTM	JHY

DATE:	SCALE:
12/13/2023	NOTED

DG BTS HUNTINGTON, LLC
HUNTINGTON, WAYNE COUNTY WV

DETAILS

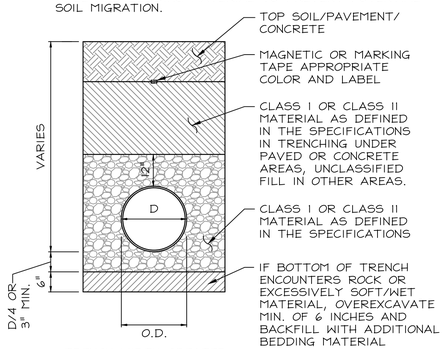
TRIAD ENGINEERING, INC.
www.triadeng.com

SHEET NUMBER:
C901

PROJECT No.: 04-23-0376

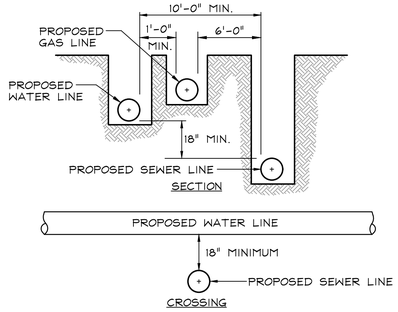
NOTES:

1. REFER TO TRENCHING AND BACKFILLING SPECIFICATION FOR DETAILED INSTALLATION AND COMPACTION INFORMATION.
2. CLASS I OR CLASS II MATERIALS TO BE COMPACTED TO 90% STANDARD PROCTOR. UNCLASSIFIED FILL TO BE COMPACTED TO 95% STANDARD PROCTOR.
3. IF GROUNDWATER SEEPAGE IS ENCOUNTERED IN THE PIPE TRENCH, A 4 OZ. NON-WOVEN GEOTEXTILE SHALL BE UTILIZED BETWEEN THE SEEPAGE IN THE TRENCH WALL AND BEDDING MATERIAL TO PREVENT POTENTIAL SOIL MIGRATION.

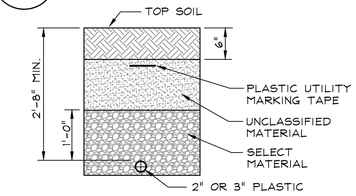


TRENCH WIDTH GREATER OF THE FOLLOWING
(A) O.D. + 18"
(B) 1.25 X O.D. +12"

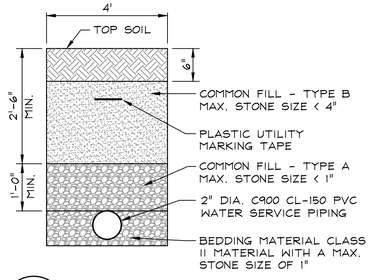
1 C902 STORM TRENCHING
N.T.S.



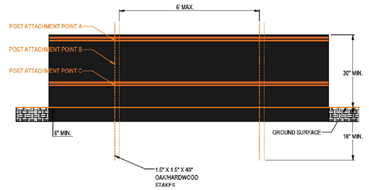
2 C902 UTILITY SEPARATION
N.T.S.

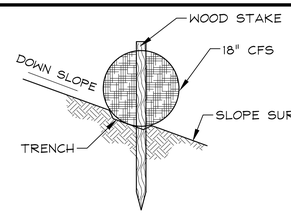


3 C902 GAS LINE TRENCHING
N.T.S.

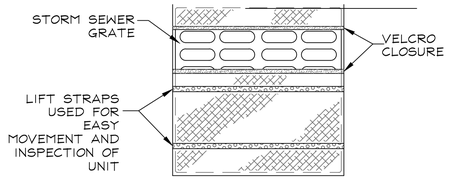


4 C902 WATER LINE TRENCHING
N.T.S.

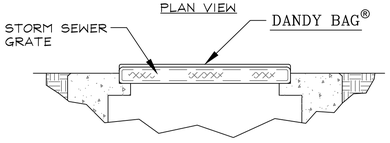




CFS & STAKE DETAIL
NOT TO SCALE



PLAN VIEW



SECTION VIEW
SILT BAG INLET DETAIL
NOT TO SCALE

SMART FENCE 36" INSTALLATION NOTES:

STEP 1: EXCAVATE TRENCH A MAXIMUM OF 2 WIDE AND 4 DEEP. THE TRENCH SHALL BE HAND-CLEANED FOLLOWING EXCAVATION TO REMOVE BULKY DEBRIS SUCH AS ROCKS, STICKS, AND SOIL CLODS FROM THE TRENCH. DRIVE HARDWOOD POSTS, HAVING MINIMUM 1 X 2 CROSS-SECTION DIMENSIONS AND 48 LONG, INTO THE ground. DRIVE POST INTO GROUND A MINIMUM OF 16 depth. POST SPACING MUST BE NO GREATER THAN 6 FT MAXIMUM.

STEP 2: LAYOUT SMARTFENCE℄ 36 ALONG PROPOSED FENCE LINE NEXT TO ANCHOR TRENCH. LOCATE ONE END OF THE SMARTFENCE℄ 36 AND POSITION NEAR THE INITIAL post. POSITION SMARTFENCE℄ 36 VERTICALLY ALONG THE INITIAL POST.

STEP 3: FOR THE INITIAL POST, PLACE THE END OF SMARTFENCE℄ 36 ALONG THE POST HEIGHT AND ROTATE THE POST 360 DEGREES, MAINTAINING TENSION ON THE FENCE SYSTEM. SECURE THE FENCE TO THE POST USING HEAVY-DUTY WIRE STAPLES (HAVING INCH LENGTH AND 1 WIDTH) AT THE TWO (2) ORANGE-COLORED BAND LOCATIONS AND AT A LOCATION HALFWAY BETWEEN THE TWO ORANGE BANDS (MINIMUM 3 ATTACHMENT LOCATIONS). ALTERNATE STAPLES MAY BE USED OF A SIZE AND TYPE AS APPROVED BY THE STATE DOT.

STEP 4: DRIVE THE INITIAL POST WITH THE ATTACHED FENCE INTO THE GROUND TO 16 DEPTH.

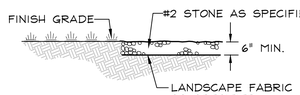
STEP 5: DRIVE THE INTERIOR POSTS OF THE FENCE SYSTEM INTO THE GROUND AT LEAST 16 .

STEP 6: MOVE TO THE NEXT POST WHILE PULLING SMARTFENCE℄ 36 TIGHTLY. POSITION THE SMARTFENCE℄ 36 IN FRONT OF THE ADJACENT POST IN PREPARATION FOR FASTENING THE FENCE TO THE POST. SECURE THE FENCE TO THE POST USING STAPLES AT THE TWO (2) ORANGE-COLORED BAND LOCATIONS AND AT A LOCATION HALFWAY BETWEEN THE TWO ORANGE BANDS (MINIMUM 3 ATTACHMENT LOCATIONS) AS INSTRUCTED IN STEP 3.

STEP 7: AFTER THE INTERIOR POSTS HAVE BEEN FASTENED TO THE SMARTFENCE℄ 36, SECURE THE FENCE TO THE FINAL POST BY PULLING THE FINAL SECTION OF FENCING TAUT, THEN ROTATING THE POST 360 DEGREES, MAINTAINING TENSION ON THE FENCE SYSTEM. SECURE THE FENCE TO THE POST USING STAPLES AT THE TWO (2) ORANGE-COLORED BAND LOCATIONS AND AT A LOCATION HALFWAY BETWEEN THE TWO ORANGE BANDS (MINIMUM 3 ATTACHMENT LOCATIONS) AS INSTRUCTED IN STEP 3. DRIVE THE FINAL POST INTO THE GROUND TO 16 DEPTH.

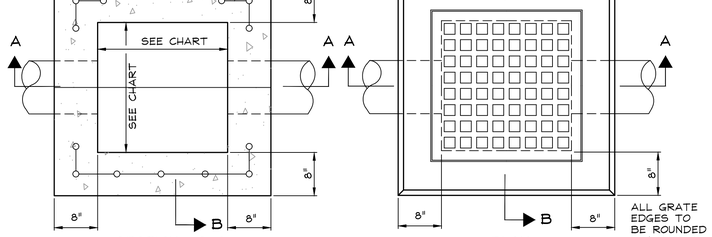
STEP 8: PLACE BOTTOM 6 INCHES OF FABRIC INTO THE trench. BACKFILL TRENCH (OVERFILL) WITH SOIL PLACED AROUND FABRIC. COMPACT SOIL BACKFILL WITH EITHER MANUAL TAMPING (OR OTHER MANUAL MEANS) OR VIA MECHANICAL EQUIPMENT SUCH AS THE FRONT WHEEL OF A TRACTOR, SKID STEER, ROLLER, OR OTHER DEVICE (PER NOTE 5 OF ASTM D 6462 STANDARD PRACTICE FOR SILT FENCE INSTALLATION). DO NOT DAMAGE THE FABRIC DURING COMPACTION (DAMAGED FABRIC SHALL BE REPLACED).

6 C902 SMART FENCE, FILTER SOCK & INLET PROTECTION DETAILS
N.T.S.



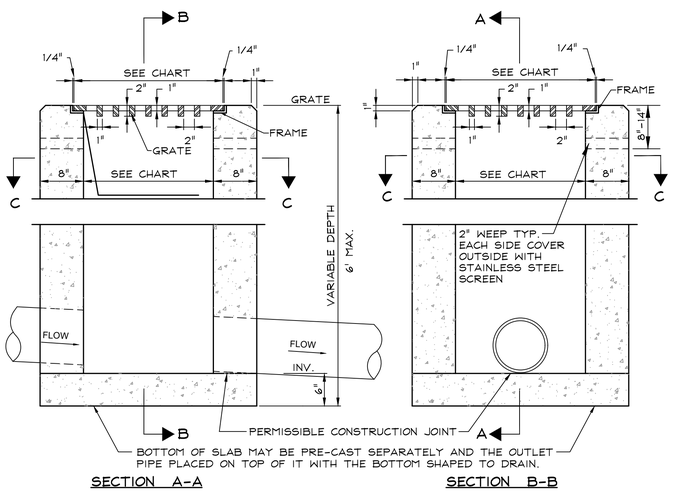
CONSTRUCTION SPECIFICATIONS

1. STONE SIZE - USE No. 2 (2"-3") STONE.
2. LENGTH - AS REQUIRED, BUT NOT LESS THAN 50 FEET.
3. THICKNESS - NOT LESS THAN SIX (6) INCHES.
4. WIDTH - TWENTY FOOT MINIMUM, BUT NOT LESS THAN THE FULL WIDTH AT POINTS WHERE INGRESS OR EGRESS OCCURS.
5. FILTER CLOTH - WILL BE PLACED OVER THE ENTIRE AREA PRIOR TO PLACING OF STONE.
6. SURFACE WATER - ALL SURFACE WATER FLOWING OR DIVERTED TOWARD CONSTRUCTION ENTRANCES SHALL BE PIPED ACROSS THE ENTRANCE. IF PIPING IS IMPRACTICAL, A MOUNTABLE BERM WITH 5:1 SLOPES WILL BE PERMITTED.
7. MAINTENANCE - THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION WHICH WILL PREVENT TRACKING OR FLOWING ONTO RIGHTS-OF-WAY. THIS MAY REQUIRE PERIODIC TOP DRESSING WITH ADDITIONAL STONE AS CONDITIONS DEMAND AND REPAIR AND/OR CLEANOUT OF ANY MEASURES USED TO TRAP SEDIMENT. ALL SEDIMENT SPILLED, DROPPED, WASHED OR TRACKED ONTO PUBLIC RIGHTS-OF-WAY MUST BE REMOVED IMMEDIATELY.
8. WASHING - WHEELS SHALL BE CLEANED TO REMOVE SEDIMENT PRIOR TO ENTRANCE ONTO RIGHTS-OF-WAY. WHEN WASHING IS REQUIRED, IT SHALL BE DONE ON AN AREA STABILIZED WITH STONE AND WHICH DRAINS INTO AN APPROVED SEDIMENT TRAPPING DEVICE.
9. PERIODIC INSPECTION AND NEEDED MAINTENANCE SHALL BE PROVIDED AFTER EACH RAIN.



DIMENSIONS	TYPE 2 GRATE			
Pipe Size	A	F	Bars	WT.
18"	2'-8"	3 3/8"	7	223
21"	2'-8"	3 3/8"	7	223
24"	2'-8"	3 3/8"	7	223

PIPE	FRAME	TYPE 1 GRATE		
Size	WT.	F	Bars	WT.
18"	62	1 3/8"	15	453
21"	62	1 3/8"	15	453
24"	62	1 3/8"	15	453

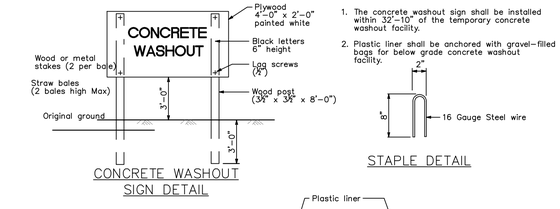


INLET NOTES:

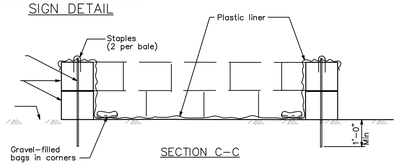
1. LOCATION AND ELEVATION WHEN GIVEN ON THE PLANS IS TOP CENTER OF THE GRATE.
2. TYPE 1 OR 2 GRATING AND FRAME - SQUARE SHALLOW GRATE IN GREENSPACE OR ALONG CURB: EAST JORDAN IRON WORKS MODEL V-5700 WITH PEDESTRIAN SAFE GRATE OR EQUAL. ROUND DEEP FRAME GRATE IN PAVEMENT: EAST JORDAN IRON WORKS MODEL 1040 WITH HEAVY DUTY RADIAL MI GRATE TYPE D.
3. CAST-IN-PLACE WALLS SHALL HAVE NOMINAL THICKNESS OF 8 INCHES. PRECAST WALLS SHALL HAVE A MINIMUM THICKNESS OF 8 INCHES AND BE REINFORCED SUFFICIENTLY TO PERMIT SHIPPING AND HANDLING WITHOUT DAMAGE. (MINIMUM 6" X 6" X 10 GAUGE MESH, UNLESS OTHERWISE SHOWN.)
4. OPENINGS FOR PIPE SHALL BE MADE TO ACCOMMODATE SDR 35 HDPE PIPE.
5. UTILIZE RUBBER BOOT TYPE GASKETS FOR ALL PENETRATIONS. GROUTING OF PIPE CONNECTIONS IS NOT ACCEPTABLE.
6. TRITON DROP INLET CARTRIDGE. MANUFACTURED BY: REVEL ENVIRONMENTAL MANUFACTURING, INC. 888.526.4736 - WWW.REMFILTERS.COM

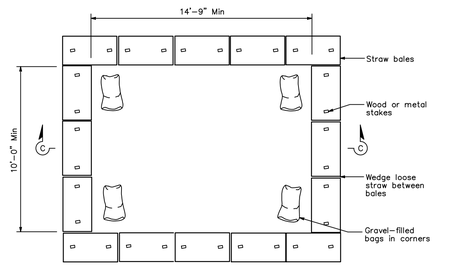
5 C902 TYPICAL PRECAST WVDOH TYPE G INLET
N.T.S.

7 C902 CONSTRUCTION ENTRANCE
N.T.S.



CONCRETE WASHOUT SIGN DETAIL





8 C902 TEMPORARY CONCRETE WASHOUT
N.T.S.

TRIAD ENGINEERING, INC.
10541 TEAYS VALLEY ROAD
SCOTT DEPOT, WV 25560
PH: 304.755.0721 FAX: 304.755.1880
OFFICE LOCATIONS
MARYLAND ○ PENNSYLVANIA ○ VIRGINIA ○ WEST VIRGINIA ○ OHIO

REV. #	DATE	DESCRIPTION	BY

CADD FILE: 23-0596 DETAILS.dwg
DRAWN BY: JTM
CHECKED BY: JHY
DATE: 12/13/2023
SCALE: NOTED

REGISTERED PROFESSIONAL ENGINEER — JAMES H. YOUNG — STATE OF WEST VIRGINIA

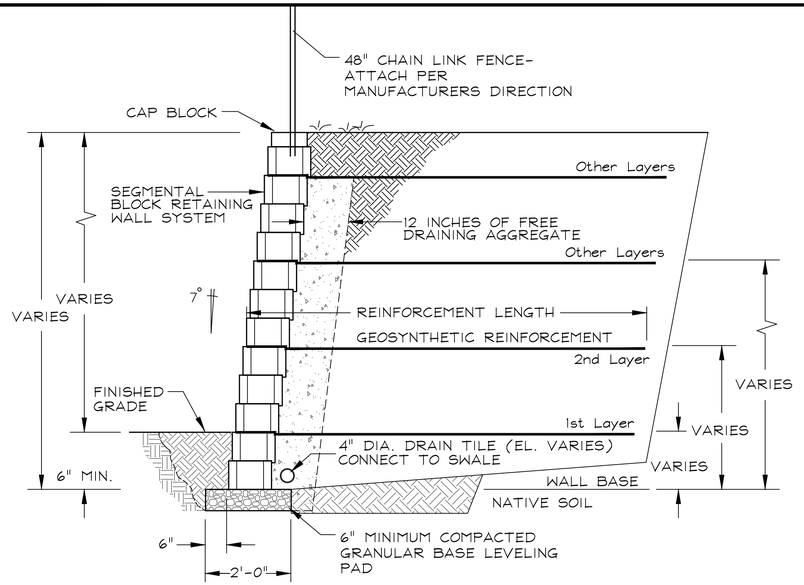
DG BTS HUNTINGTON, LLC
HUNTINGTON, WAYNE COUNTY WV

DETAILS

TRIAD ENGINEERING, INC.
www.triadeng.com

SHEET NUMBER: C902
PROJECT No.: 04-23-0376

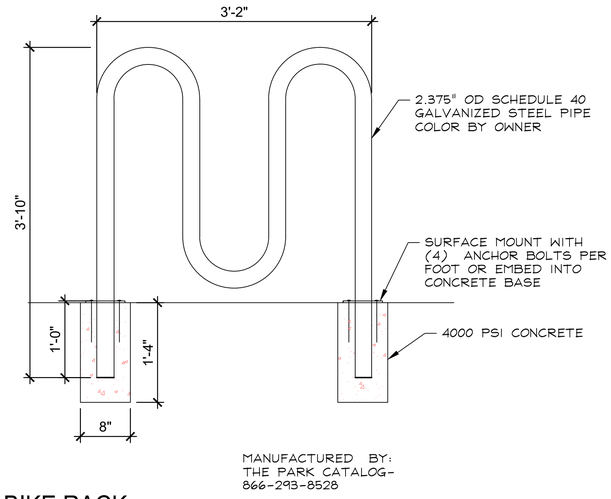
Plotted by: jyoung
y:\sw_se_04\2023_s_ 104-23-0595 dg bts huntington ch\files\cad\23-0596 details.dwg



1. SEGMENTAL BLOCK WALL LAYOUTS, SECTIONS, AND DETAILS ARE SCHEMATIC AND INFORMATIONAL ONLY. THE CONTRACTOR SHALL SELECT AND SUBMIT FOR APPROVAL A RETAINING WALL BLOCK SYSTEM FROM RED ROCK. ENGINEERED DESIGN OF THIS WALL SHALL BE PROVIDED BY MANUFACTURE OF THE WALL AND STAMPED BY A WEST VIRGINIA ENGINEER.

1 C903 SEGMENTED BLOCK RETAINING WALL

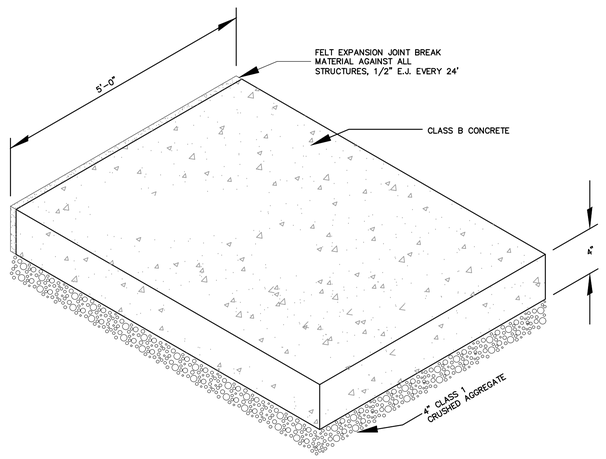
N.T.S.



MANUFACTURED BY:
THE PARK CATALOG-
866-293-8528

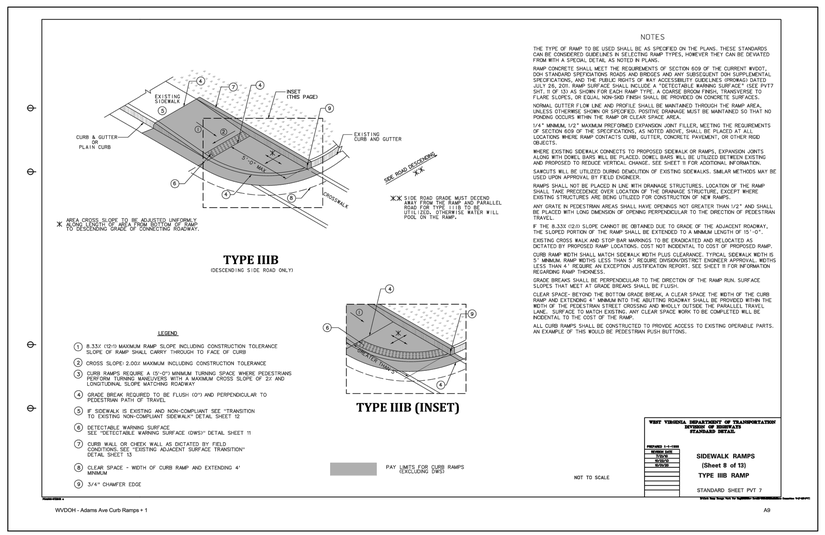
2 C903 BIKE RACK

N.T.S.

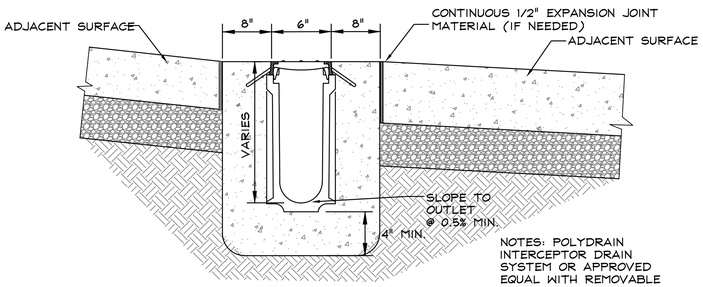


3 C903 DOH CONCRETE WALK CURB AND RAMP DETAILS

N.T.S.



NOTES:
CONCRETE FOR CONSTRUCTING THE INLET SHALL HAVE MIX PROPORTIONS IN ACCORDANCE WITH SECTION 605 OF THE SPECIFICATIONS; HOWEVER, TESTING WILL NOT BE REQUIRED. ALL WORK, INCLUDING PIPE EXTENSIONS IF CALLED FOR ON THE PLANS, IS TO BE INCLUDED IN THE COST OF INLET.



NOTES: POLYDRAIN INTERCEPTOR DRAIN SYSTEM OR APPROVED EQUAL WITH REMOVABLE SLOTTED GRATE. CONTRACTOR TO PROVIDE SHOP DRAWINGS.

4 C903 POLYDRAIN W/SLOTTED GRATE SYSTEM

N.T.S.

TRIAD ENGINEERING, INC.
10541 TEAYS VALLEY ROAD
SCOTT DEPOT, WV 25560
PH: 304.755.0721 FAX: 304.755.1880
OFFICE LOCATIONS
MARYLAND ○ PENNSYLVANIA ○ VIRGINIA ○ WEST VIRGINIA ○ OHIO

REV. #	DATE	DESCRIPTION	BY

CADD FILE: 23-0595 DETAILS.dwg
DRAWN BY: JTM
CHECKED BY: JHY
DATE: 12/13/2023
SCALE: NOTED

DG BTS HUNTINGTON, LLC
HUNTINGTON, WAYNE COUNTY WV

DETAILS

TRIAD ENGINEERING, INC.
www.triadeng.com

SHEET NUMBER:
C903
PROJECT No.: 04-23-0376

GENERAL NOTES

A

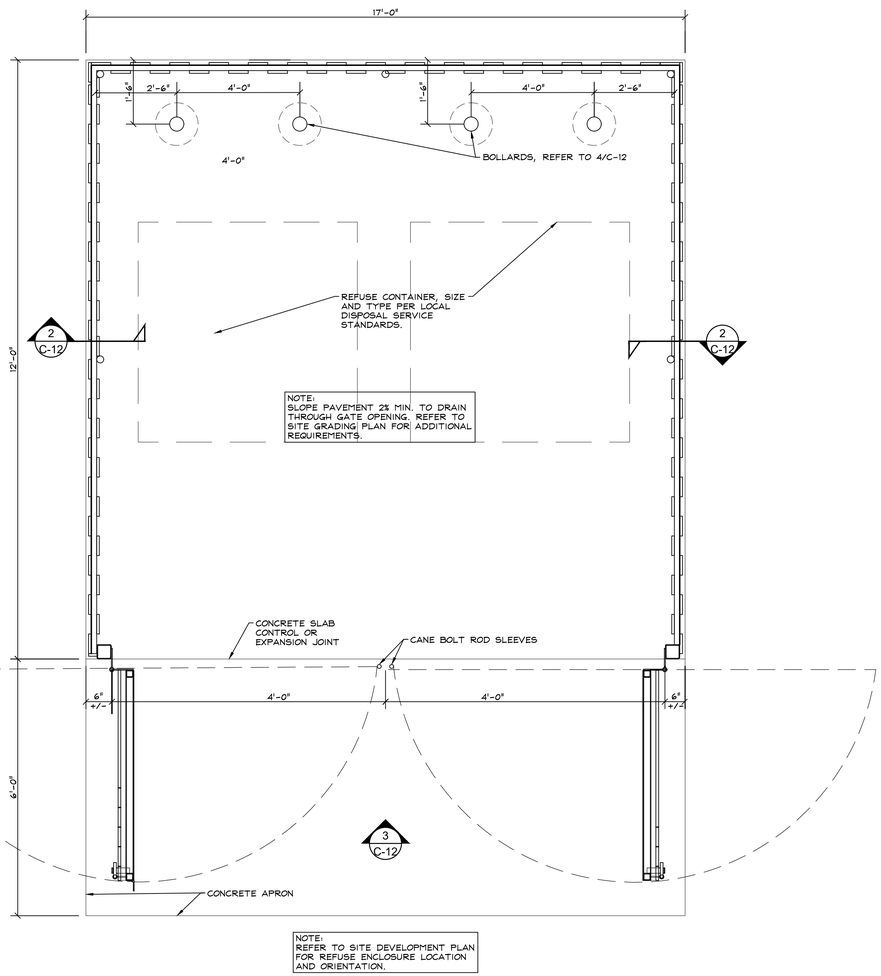
B DIMENSIONS ARE TO ROUGH FACE OF CONCRETE, FRAMING, OR CENTER LINE OF STRUCTURE UNLESS OTHERWISE INDICATED.

C REFER TO CIVIL DRAWINGS FOR ADDITIONAL REQUIREMENTS.

D REFER TO ARCHITECTURAL EXTERIOR FINISH SCHEDULE FOR MATERIAL TYPES, COLORS, AND FINISHES. UNLESS OTHERWISE INDICATED PROVIDE FINISHES AS FOLLOWS:

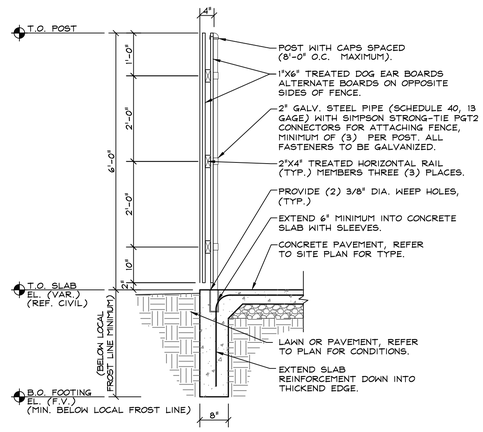
ALL METAL PARTS TO BE PRIMED AND PAINTED (2) COATS SHERWIN WILLIAMS SW6258 "TRICON BLACK".

EXPOSED WOOD TO BE PRIMED AND PAINTED (2) COATS OF COLOR TO MATCH BUILDING.



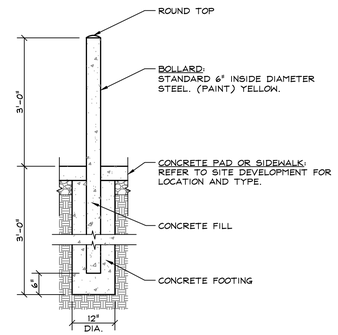
1 C904 WOOD SCREEN FENCE REFUSE ENCLOSURE PLAN

N.T.S.



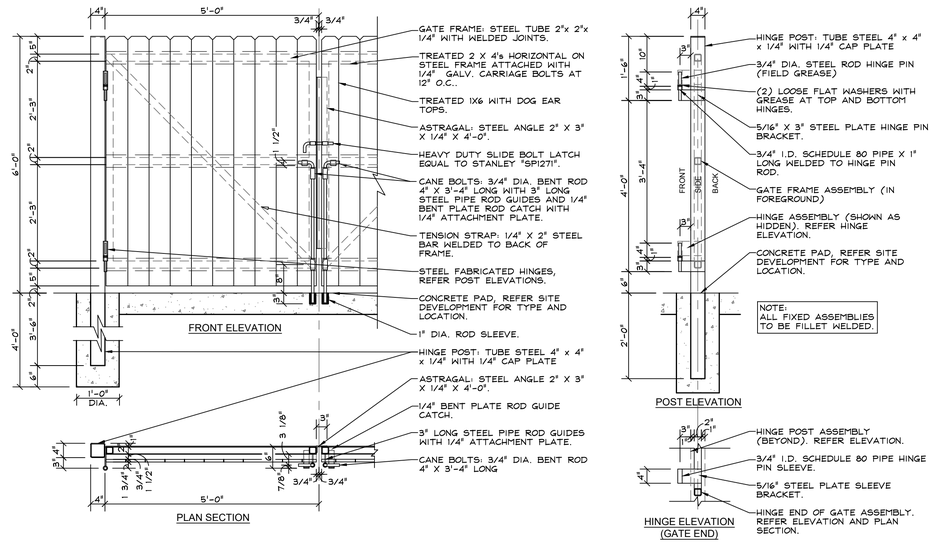
2 C904 SCREEN FENCE SECTION

N.T.S.



4 C904 STEEL BOLLARD SECTION

N.T.S.



3 C904 SCREEN FENCE GATE DETAILS

N.T.S.

TRIAD ENGINEERING, INC.
10541 TEAYS VALLEY ROAD
SCOTT DEPOT, WV 25560
PH: 304.755.0721 FAX: 304.755.1880
OFFICE LOCATIONS
MARYLAND PENNSYLVANIA VIRGINIA WEST VIRGINIA OHIO

REV. #	DATE	DESCRIPTION	BY

CADD FILE: 23-0595 DETAILS.dwg
DRAWN BY: JTM
CHECKED BY: JHY
DATE: 12/13/2023
SCALE: NOTED

DG BTS HUNTINGTON, LLC
HUNTINGTON, WAYNE COUNTY WV

DETAILS

TRIAD ENGINEERING, INC.
www.triadeng.com

SHEET NUMBER:
C904
PROJECT No.: 04-23-0376

DOLLAR GENERAL

US ROUTE 60

HUNTINGTON, WV

DRAWING INDEX

TITLE	SHEET NO.
COVER SHEET	1 OF 5
SYSTEM LAYOUT SHEET	2 OF 5
SYSTEM CALCULATION SHEET	3 OF 5
SYSTEM OVERLAY SHEET	4 OF 5
902HD DETAIL SHEET	5 OF 5

PROJECT INFORMATION						
PROJECT NO:	23-1534					
CULTEC SALES REP:	RYAN THURLING 475-289-7068 RYAN.THURLING@CULTEC.COM					
CULTEC TECHNICAL SALES ENGINEER:	CHRIS BUCCI 484-636-7384 CHRISTOPHER.BUCCI@CULTEC.COM					
CULTEC PROJECT COORDINATOR:	JILL GORNEAU 203-973-7033 JILL.GORNEAU@CULTEC.COM					
ENGINEER OF RECORD	TRIAD ENGINEERING					
REVISIONS:	ITERATION	DATE	BY	COMMENTS	EOR SHEET REFERENCE	DATE
	00	12/8/2023	JIG	SUBMITTAL DRAWINGS	C600 GRADING AND DRAINAGE PLAN	12/1/2023

CULTEC

Subsurface Stormwater Management Systems

P.O. Box 280
878 Federal Road
Brookfield, CT 06804
www.cultec.com

PH: 1(203) 775-4416
PH: 1(800) 4-CULTEC
CT-tech@cultec.com

NOTE: THESE SHOP DRAWINGS MAY CONTAIN COMPONENTS INCLUDING BUT NOT LIMITED TO MANHOLES, CATCH BASINS, STORM PIPES AND FITTINGS, MANIFOLDS, CASTINGS AND OTHER NECESSARY APPURTENANCES THAT MAY NOT BE SUPPLIED BY CULTEC, INC. IT IS THE RESPONSIBILITY OF THE CONTRACTOR AND/OR SUPPLIER TO CONFIRM WITH CULTEC THE MATERIALS PROVIDED.

BEFORE YOU BEGIN - REQUIRED MATERIALS AND EQUIPMENT

1. PROPER GEOTECHNICAL SOIL EVALUATION BY A QUALIFIED ENGINEER OR SOIL SCIENTIST TO DETERMINE SUITABILITY OF STRUCTURAL INSTALLATION
2. OSHA COMPLIANCE
3. CULTEC WARNING TAPE, OR EQUIVALENT
4. ASSURANCES FROM LOCAL UTILITIES THAT NO UNDERGROUND GAS, ELECTRICAL OR OTHER POTENTIALLY DANGEROUS PIPELINES OR CONDUITS ARE ALREADY BURIED AT THE SITE
5. ACCEPTABLE 1- 2 INCH (25 - 51 mm) WASHED, CRUSHED STONE AS DETAILED IN CULTEC'S INSTALLATION INSTRUCTIONS. CLEANLINESS OF STONE TO BE VERIFIED BY ENGINEER.
6. ACCEPTABLE FILL MATERIAL AS SHOWN IN CULTEC'S INSTALLATION INSTRUCTIONS.
7. ALL CULTEC CHAMBERS AND ACCESSORIES AS SPECIFIED IN THE ENGINEER'S PLANS INCLUDING CULTEC NO. 410 NON-WOVEN GEOTEXTILE, CULTEC STORMFILTER AND CULTEC NO. 4800 WOVEN GEOTEXTILE, WHERE APPLICABLE.
8. RECIPROCATING SAW OR ROUTER
9. STONE BUCKET
10. STONE CONVEYOR AND/OR TRACKED EXCAVATOR
11. TRANSIT OR LASER LEVEL MEASURING DEVICE
12. COMPACTION EQUIPMENT WITH MAXIMUM GROSS VEHICLE WEIGHT OF 12,000 LBS (5,440 KGS). VIBRATORY ROLLERS MAY ONLY BE USED ON THE STONE BASE PRIOR TO THE INSTALLATION OF CHAMBERS.
13. CHECK CULTEC CHAMBERS FOR DAMAGE PRIOR TO INSTALLATION. DO NOT USE DAMAGED CULTEC CHAMBERS, CONTACT YOUR SUPPLIER IMMEDIATELY TO REPORT DAMAGE OR PACKING-LIST DISCREPANCIES.

REQUIREMENTS FOR CULTEC CHAMBER SYSTEM INSTALLATIONS

1. INSTALLING CONTRACTORS ARE EXPECTED TO COMPREHEND AND USE THE MOST CURRENT INSTALLATION INSTRUCTIONS PRIOR TO BEGINNING A SYSTEM INSTALLATION. IF THERE IS ANY QUESTION AS TO WHETHER YOU POSSESS THE MOST CURRENT INSTRUCTIONS, CONTACT CULTEC AT (203) 775-4416 OR VISIT WWW.CULTEC.COM.
2. CONTACT CULTEC AT LEAST THIRTY DAYS PRIOR TO SYSTEM INSTALLATION TO ARRANGE FOR A PRE-CONSTRUCTION MEETING.
3. ALL CULTEC SYSTEM DESIGNS MUST BE CERTIFIED BY A REGISTERED PROFESSIONAL ENGINEER.
4. USE CULTEC INSTALLATION INSTRUCTIONS AS A GUIDELINE ONLY FOR MINIMUM/MAXIMUM REQUIREMENTS. ACTUAL DESIGN MAY VARY. REFER TO APPROVED CONSTRUCTION DRAWINGS FOR JOB-SPECIFIC DETAILS. BE SURE TO FOLLOW THE ENGINEER'S DRAWINGS AS YOUR PRIMARY GUIDE.
5. THE FOUNDATION STONE SHALL BE LEVEL AND COMPACTED PRIOR TO CHAMBER INSTALLATION.
6. OVERLAPPING RIB CONNECTIONS OF CHAMBERS SHALL BE FULLY SHOULDERED PRIOR TO STONE PLACEMENT.
7. CENTER-TO-CENTER SPACING SHALL BE CHECKED AND MAINTAINED THROUGHOUT INSTALLATION PROCESS.
8. ANY DISCREPANCIES WITH THE SYSTEM SUB-GRADE SOIL'S BEARING CAPACITY MUST BE REPORTED TO THE DESIGN ENGINEER.
9. NON-WOVEN GEOTEXTILE MUST BE USED AS SPECIFIED IN THE ENGINEER'S DRAWINGS.
10. CULTEC REQUIRES THE CONTRACTOR TO REFER TO CULTEC'S INSTALLATION INSTRUCTIONS CONCERNING VEHICULAR TRAFFIC. RESPONSIBILITY FOR PREVENTING VEHICLES THAT EXCEED CULTEC'S REQUIREMENTS FROM TRAVELING ACROSS OR PARKING OVER THE CHAMBER SYSTEM LIES SOLELY WITH THE CONTRACTOR THROUGHOUT THE ENTIRE SITE CONSTRUCTION PROCESS. THE PLACEMENT OF WARNING TAPE, TEMPORARY FENCING, AND/OR APPROPRIATELY LOCATED SIGNS IS HIGHLY RECOMMENDED. IMPRINTED WARNING TAPE IS AVAILABLE FROM CULTEC. FOR ACCEPTABLE VEHICLE LOAD INFORMATION, REFER TO CULTEC INSTALLATION INSTRUCTIONS.
11. TRAFFIC OF INSTALLATION EQUIPMENT OR OTHER VEHICULAR TRAFFIC OVER TOP OF THE CULTEC STORMWATER SYSTEM IS STRICTLY RESTRICTED AND PROHIBITED UNTIL SATISFACTORY COVER AND COMPACTION IS ACHIEVED ACCORDING TO CULTEC'S MANUFACTURER INSTALLATION INSTRUCTIONS.
12. EROSION AND SEDIMENT-CONTROL MEASURES MUST MEET LOCAL CODES AND THE DESIGN ENGINEER'S SPECIFICATIONS THROUGHOUT THE ENTIRE SITE CONSTRUCTION PROCESS.
13. CULTEC SYSTEMS MUST BE DESIGNED AND INSTALLED IN ACCORDANCE WITH CULTEC'S MINIMUM REQUIREMENTS. FAILURE TO DO SO WILL VOID THE LIMITED WARRANTY.
14. CONTACT CULTEC, INC. AT 203-775-4416 WITH ANY QUESTIONS OR FURTHER CLARIFICATION OF REQUIREMENTS.
15. PLACEMENT OF EMBEDMENT STONE MUST BE IN ACCORDANCE WITH CULTEC'S INSTALLATION INSTRUCTIONS. STONE COLUMN HEIGHT DEFERENTIAL MUST NEVER EXCEED 12" (305 mm) BETWEEN CHAMBER ROWS, ADJACENT CHAMBERS OR STONE PERIMETER. STONE MUST BE PLACED OVER THE CROWN OF THE CHAMBERS TO ANCHOR THE CHAMBERS IN PLACE AND MAINTAIN ROW SPACING.
16. EMBEDMENT STONE MUST ONLY BE PLACED BY EXCAVATOR OR TELESCOPING CONVEYOR BOOM. PLACEMENT OF EMBEDMENT STONE WITH BULLDOZER IS NOT AN ACCEPTABLE METHOD OF INSTALLATION AND MAY CAUSE DAMAGE TO THE CHAMBERS. ANY CHAMBERS DAMAGED USING AN UNACCEPTABLE METHOD OF BACKFILL ARE NOT COVERED UNDER THE CULTEC LIMITED WARRANTY.

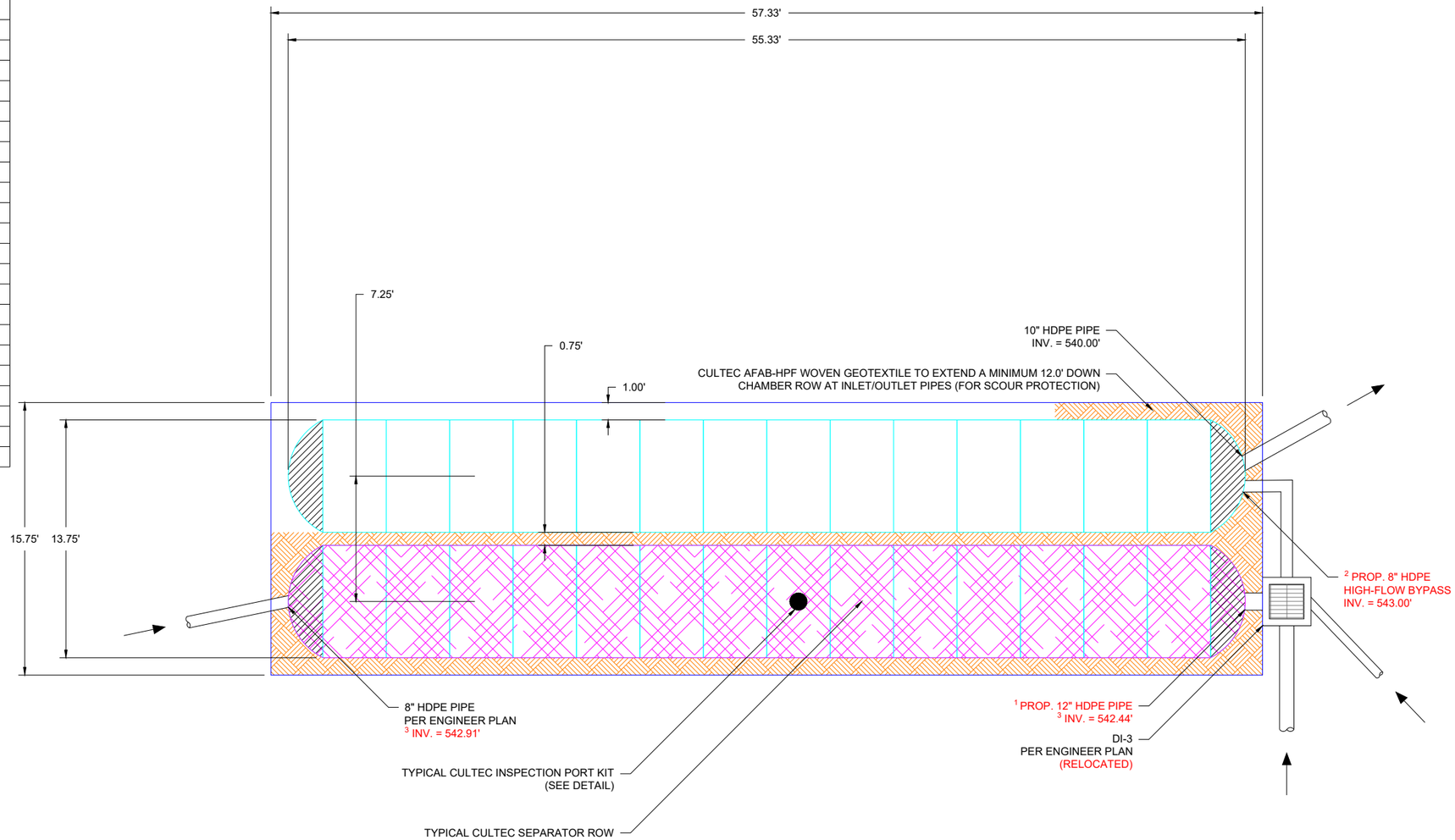
THIS DRAWING HAS BEEN PREPARED TO SUPPORT THE PROJECT ENGINEER OF RECORD FOR THE PROPOSED SYSTEM. THIS DRAWING HAS BEEN PREPARED BASED ON INFORMATION PROVIDED TO CULTEC UNDER THE DIRECTION OF THE PROJECT ENGINEER OF RECORD OR OTHER PROJECT REPRESENTATIVE. IT IS ULTIMATE RESPONSIBILITY OF THE PROJECT ENGINEER OF RECORD TO ENSURE THAT THE CULTEC SYSTEM'S DESIGN IS IN FULL COMPLIANCE WITH ALL APPLICABLE LAWS, REGULATIONS AND MANUFACTURER REQUIREMENTS.

PROPOSED STORMWATER MANAGEMENT SYSTEM ELEVATIONS
(TO BE APPROVED BY ENGINEER OF RECORD)
*ENGINEER OF RECORD TO CONFIRM MINIMUM AND MAXIMUM BURIAL REQUIREMENTS ARE MET)

MAXIMUM ALLOWABLE GRADE (TOP OF PAVEMENT OR UNPAVED)	552.30
MINIMUM ALLOWABLE GRADE (UNPAVED TRAFFIC)	546.50
MINIMUM ALLOWABLE GRADE (BASE OF FLEXIBLE PAVEMENT)	546.00
MINIMUM ALLOWABLE GRADE (TOP OF RIGID PAVEMENT)	546.00
TOP OF STONE ELEVATION	545.00
TOP OF CHAMBER ELEVATION	544.00
8" HIGH-FLOW BYPASS PIPE INVERT	543.00
OUTLET PIPE INVERT	540.00
INLET PIPE INVERT FROM DI-1	542.91
INLET PIPE INVERT FROM DI-3	542.44
BOTTOM OF CHAMBER ELEVATION	540.00
BOTTOM OF STONE ELEVATION	539.25

CULTEC STORMWATER MANAGEMENT SYSTEM SUMMARY

TOTAL STORAGE REQUIRED (CF)	3,000
TOTAL STORAGE PROVIDED (CF)	3,186
% STONE POROSITY	40
SYSTEM AREA (SF)	903.00
DEPTH OF EMBEDMENT STONE (IN)	12
DEPTH OF BEDDING STONE (IN)	9
STONE PERIMETER (IN)	12
SPACING BETWEEN CHAMBER ROWS (IN)	9



1 SYSTEM LAYOUT DETAIL
NTS

NOTE: ALL EXTERNAL SYSTEM STRUCTURES, INLET/OUTLET PIPES AND PROPOSED ELEVATIONS MUST BE DESIGNED AND APPROVED BY THE ENGINEER OF RECORD. ALL PROPOSED SYSTEM ELEVATIONS PROVIDED MUST BE VERIFIED BY THE ENGINEER OF RECORD ENGINEER AND THE ENGINEER OF RECORD MUST ENSURE CHAMBER BURIAL REQUIREMENTS ARE MET

MATERIALS LIST SUPPLIED BY CULTEC		
CULTEC RECHARGER 902HD CHAMBER	28	PIECES
CULTEC RECHARGER 902HD END CAP	4	PIECES
CULTEC HVLV FC-48 FEED CONNECTORS	0	PIECES
CULTEC NO. 410 NON-WOVEN GEOTEXTILE	424	SQ. YARDS
CULTEC AFAB-HPF WOVEN GEOTEXTILE	87	LINEAL FEET
CULTEC INSPECTION PORT KIT (BASE AND SOLID COVER)	1	PIECES
MATERIALS LIST NOT SUPPLIED BY CULTEC		
1-2 INCH WASHED, CRUSHED STONE	124	CUBIC YARDS
8 OZ. NON-WOVEN GEOTEXTILE	N/A	SQ. YARDS
30 MIL. PVC THERMOPLASTIC LINER	N/A	SQ. YARDS

CULTEC RECHARGER®
902HD LEGEND

- CULTEC RECHARGER 902HD CHAMBER
- CULTEC RECHARGER 902HD END CAP
- CULTEC HVLV FC-48 FEED CONNECTORS
- CULTEC SEPARATOR ROW
- CULTEC WOVEN GEOTEXTILE
- STONE BORDER

PROPOSED SYSTEM ALTERATION TABLE	
1	PROPOSED SEPARATOR ROW ACCESS PIPE
2	PROPOSED SEPARATOR ROW HIGH-FLOW BYPASS PIPE
3	PROPOSED PIPE INVERTS

CULTEC STORMWATER CHAMBER

PROJECT NO:	23-1534.00	DATE:	12/8/2023
DESIGNED BY:	JIG	CHECKED BY:	JMG
SCALE:	N.T.S.	SHEET NO:	2 OF 5

DOLLAR GENERAL
US ROUTE 60
HUNTINGTON, WV
SYSTEM LAYOUT SHEET

CULTEC
Subsurface Stormwater Management Systems
P.O. Box 280
878 Federal Road
Brookfield, CT 06804
www.cultec.com
PH: 1(203) 775-4416
PH: 1(800) 4-CULTEC
CT-tech@cultec.com

CULTEC Recharger 902HD Stormwater System Calculations

Consulting Engineer:
Triad Scott Depot

Project Information:
Dollar General
Huntington, WV

Calculations Performed By:
Johnny Gonzalez
CULTEC
878 Federal Rd.
Brookfield, CT 06804

Date: 12/8/23

Project Number: 23-1534.00

System Information

Rectangular Bed Inputs — **No. of Rows** 2 — **No. of Chambers/Row** 14

Given:

Storage required	3,000	CF		m^3
Number of Inlet/Outlet Pipes (Do Not Include Separator Rows)	2			
Stone **Base**	9	inches	229	mm
Stone **Above**	12	inches	305	mm
Spacing **Between** Rows	9	inches	229	mm
12" PVC Universal Inline Drain Body Only - Kit	1	units		
12" Ductile Iron Square Solid Drain Base Cover	1	units		
Stone Porosity	40	%		
Stone **Border Width**	12	inches	305	mm

☐ Discount stone base from total storage provided (If Applicable)
☐ Discount stone above from total storage provided (If Applicable)

Other Parameters:

Length of Separator Row	55	feet	16.865	m
Type of Lining	None			
☐ Sand Filter Depth (If Applicable)		feet	0.000	m
☐ Sloped Sides (1:1) (If Applicable)				

Maximum Finished Grade Elevation:	552.30
Minimum Finished Grade Elevation (Unpaved):	546.50
Minimum Finished Grade Elevation (Base of Flexible Pavement):	546.00
Minimum Finished Grade Elevation (Top of Rigid Pavement):	546.00
Top of Stone Elevation:	545.00
Top of Chamber Elevation:	544.00
Bottom of Chamber Elevation:	540.00
Bottom of Stone Elevation:	539.25

Assumptions

Model Name		Chamber Height	Design Unit Height	Chamber Width	Chamber Spacing	Design Unit Width	Chamber Volume per Linear Foot	Design Unit Volume	Installed Chamber Length
		inches	feet	inches	inches	feet	cu. ft/ft	cu. ft/ft	feet
		mm	m	mm	mm	m	cu. m/m	cu. m/m	m
Recharger® 902HD Chamber	English	48	5.750	78	9	7.25	17.31	27.061	3.667
	Metric	1219	1.753	1981	229	2.21	1.608	2.514	1.118
Recharger® 902HD End Cap	English	48.5	5.750	78	9	7.25	9.010	22.081	2.000
	Metric	1232	1.753	1981	229	2.21	0.837	2.051	0.610
HVLV™ FC-48 Feed Connectors	English	12	n/a	16	n/a	n/a	0.913	n/a	0.750
	Metric	305	n/a	406	n/a	n/a	0.085	n/a	0.229

Storage Provided within CULTEC Recharger 902HD Stormwater Chamber, End Caps and HVLV FC-48 Feed Connector Internal Manifold System - *not including stone*

Number of Recharger 902HD chambers by design			=	28 pcs	
28	pcs x	3.667	=	102.67 feet	31.29 m
Number of Recharger 902HD end caps			=	4 pcs	
4	pcs x	2.000	=	8.00 feet	2.44 m
Number of HVLV FC-48 Feed Connectors			=	0 pcs	
0	pcs x	0.750	=	0.00 feet	0.00 m
Total footage of Recharger 902HD chambers			=	102.67 feet	31.29 m
Total footage of Recharger 902HD end caps			=	8.00 feet	
Total footage of HVLV FC-48 Feed Connectors			=	0.00 feet	0.00 m
Storage provided within Recharger 902HD chambers			=	1777.16 CF	50.33 m^3
Storage provided within Recharger 902HD end caps				72.08 CF	2.04 m^3
Storage provided within HVLV FC-48 Feed Connectors			=	0.00 CF	0.00 m^3
Total Storage within Recharger 902HD chambers and feed connectors			=	**1849.24 CF**	**52.37 m^3**

Storage Provided within Entire CULTEC Stormwater System - *including stone*

Bed width		15.75 feet	4.80 m
Bed length		57.33 feet	17.48 m
Bed Depth		5.75 feet	1.75 m
Total Area		903.00 sq. ft.	83.89 m^2
Volume of Effective Excavation *(not including additional cover)*		5192.25 CF	147.04 m^3
Perimeter of Bed		146.17 feet	44.55 m
Total Storage within chambers, end caps and feed connectors		1849.24 CF	52.37 m^3
Total Stone Required		3343.01 CF	94.67 m^3
		124 CY	
		173 tons	
Storage provided within stone		1337.20 CF	37.87 m^3
Total Storage within CULTEC Stormwater System	=	**3186 CF**	**90 m^3**

Req. storage attained.

CULTEC MATERIALS LIST

Model	Model #	Quantity	Unit of Measure	Quantity	Unit of Measure
Recharger 902HD Heavy Duty Chamber	902HD	28	pcs		
Recharger 902HD End Cap	902HD EC	4	pcs		
HVLV FC-48 Feed Connectors	FC-48	0	pcs		
CULTEC No. 410 Non-Woven Geotextile	NWG410	424	Sq. Yards	355	m2
CULTEC AFAB-HPF Woven Geotextile 7.5' x 100'	75WGHPF	87	feet	27	m
12" PVC Universal Inline Drain Body Only - Kit	2712AGSB	1	pcs		
12" Ductile Iron Square Solid Drain Base Cover	1299CGC	1			
Total Stone		124	cubic yards	95	cubic meters

SYSTEM STORAGE CALCULATION

CULTEC Recharger 902HD Stormwater Incremental Storage

Date: December 8, 2023

Project Information
Dollar General
Huntington, WV

Project Number
23-1534.00

Base of Stone Elevation- 539.25

CULTEC Recharger 902HD Incremental Storage Volumes

Height of System		End Cap Volume		Chamber Volume		HVLV FC-48 Feed Connector Volume		Stone Volume		Cumulative Storage Volume		Total Cumulative Storage Volume		Stage/Area		Elevation		
in	mm	ft^3	m^3	ft^3	m^3	ft^3	m^3	ft^3	m^3	ft^3	m^3	ft^3	m^3	ft^2	m^2	ft	m	
69.00	1753	0.00	0.00	0.00	0.00	0.00	0.00	30.10	0.85	30.100	0.85	3186	90.23	361.20	33.56	545.00	541.00	Top of Stone Elevation
68.00	1727	0.00	0.00	0.00	0.00	0.00	0.00	30.10	0.85	30.100	0.85	3156	89.38	361.20	33.56	544.92	540.98	
67.00	1702	0.00	0.00	0.00	0.00	0.00	0.00	30.10	0.85	30.100	0.85	3126	88.53	361.20	33.56	544.83	540.95	
66.00	1676	0.00	0.00	0.00	0.00	0.00	0.00	30.10	0.85	30.100	0.85	3096	87.67	361.20	33.56	544.75	540.93	
65.00	1651	0.00	0.00	0.00	0.00	0.00	0.00	30.10	0.85	30.100	0.85	3066	86.82	361.20	33.56	544.67	540.90	
64.00	1626	0.00	0.00	0.00	0.00	0.00	0.00	30.10	0.85	30.100	0.85	3036	85.97	361.20	33.56	544.58	540.88	
63.00	1600	0.00	0.00	0.00	0.00	0.00	0.00	30.10	0.85	30.100	0.85	3006	85.12	361.20	33.56	544.50	540.85	
62.00	1575	0.00	0.00	0.00	0.00	0.00	0.00	30.10	0.85	30.100	0.85	2976	84.26	361.20	33.56	544.42	540.82	
61.00	1549	0.00	0.00	0.00	0.00	0.00	0.00	30.10	0.85	30.100	0.85	2946	83.41	361.20	33.56	544.33	540.80	
60.00	1524	0.00	0.00	0.00	0.00	0.00	0.00	30.10	0.85	30.100	0.85	2916	82.56	361.20	33.56	544.25	540.77	
59.00	1499	0.00	0.00	0.00	0.00	0.00	0.00	30.10	0.85	30.100	0.85	2885	81.71	361.20	33.56	544.17	540.75	
58.00	1473	0.00	0.00	0.00	0.00	0.00	0.00	30.10	0.85	30.100	0.85	2855	80.85	361.20	33.56	544.08	540.72	
57.00	1448	0.08	0.00	2.05	0.06	0.00	0.00	29.25	0.83	31.380	0.89	2825	80.00	376.56	34.98	544.00	540.70	Top of Chamber Elevation
56.00	1422	0.12	0.00	5.13	0.15	0.00	0.00	28.00	0.79	33.252	0.94	2794	79.11	399.02	37.07	543.92	540.67	
55.00	1397	0.16	0.00	7.19	0.20	0.00	0.00	27.16	0.77	34.508	0.98	2761	78.17	414.10	38.47	543.83	540.65	
54.00	1372	0.20	0.01	12.32	0.35	0.00	0.00	25.09	0.71	37.612	1.07	2726	77.19	451.34	41.93	543.75	540.62	
53.00	1346	0.24	0.01	16.43	0.47	0.00	0.00	23.43	0.66	40.100	1.14	2688	76.13	481.20	44.70	543.67	540.60	
52.00	1321	0.32	0.01	20.53	0.58	0.00	0.00	21.76	0.62	42.612	1.21	2648	74.99	511.34	47.50	543.58	540.57	
51.00	1295	0.40	0.01	22.59	0.64	0.00	0.00	20.91	0.59	43.892	1.24	2606	73.79	526.70	48.93	543.50	540.55	
50.00	1270	0.44	0.01	24.64	0.70	0.00	0.00	20.07	0.57	45.148	1.28	2562	72.54	541.78	50.33	543.42	540.52	
49.00	1245	0.52	0.01	27.72	0.78	0.00	0.00	18.80	0.53	47.044	1.33	2517	71.27	564.53	52.44	543.33	540.49	
48.00	1219	0.56	0.02	27.72	0.78	0.00	0.00	18.79	0.53	47.068	1.33	2470	69.93	564.82	52.47	543.25	540.47	
47.00	1194	0.64	0.02	29.77	0.84	0.00	0.00	17.93	0.51	48.348	1.37	2423	68.60	580.18	53.90	543.17	540.44	
46.00	1168	0.68	0.02	30.80	0.87	0.00	0.00	17.51	0.50	48.988	1.39	2374	67.23	587.86	54.61	543.08	540.42	
45.00	1143	0.76	0.02	31.83	0.90	0.00	0.00	17.07	0.48	49.652	1.41	2325	65.84	595.82	55.35	543.00	540.39	
44.00	1118	0.84	0.02	33.88	0.96	0.00	0.00	16.21	0.46	50.932	1.44	2276	64.44	611.18	56.78	542.92	540.37	
43.00	1092	0.92	0.03	34.91	0.99	0.00	0.00	15.77	0.45	51.596	1.46	2225	63.00	619.15	57.52	542.83	540.34	
42.00	1067	1.04	0.03	35.93	1.02	0.00	0.00	15.31	0.43	52.284	1.48	2173	61.54	627.41	58.29	542.75	540.32	
41.00	1041	1.12	0.03	35.93	1.02	0.00	0.00	15.28	0.43	52.332	1.48	2121	60.06	627.98	58.34	542.67	540.29	
40.00	1016	1.20	0.03	36.96	1.05	0.00	0.00	14.84	0.42	52.996	1.50	2068	58.57	635.95	59.08	542.58	540.27	
39.00	991	1.24	0.04	37.99	1.08	0.00	0.00	14.41	0.41	53.636	1.52	2016	57.07	643.63	59.79	542.50	540.24	
38.00	965	1.32	0.04	39.01	1.10	0.00	0.00	13.97	0.40	54.300	1.54	1962	55.55	651.60	60.53	542.42	540.22	
37.00	940	1.40	0.04	40.04	1.13	0.00	0.00	13.52	0.38	54.964	1.56	1908	54.02	659.57	61.27	542.33	540.19	
36.00	914	1.44	0.04	40.04	1.13	0.00	0.00	13.51	0.38	54.988	1.56	1853	52.46	659.86	61.30	542.25	540.16	
35.00	889	1.52	0.04	41.07	1.16	0.00	0.00	13.07	0.37	55.652	1.58	1798	50.90	667.82	62.04	542.17	540.14	
34.00	864	1.56	0.04	41.07	1.16	0.00	0.00	13.05	0.37	55.676	1.58	1742	49.33	668.11	62.07	542.08	540.11	
33.00	838	1.60	0.05	42.09	1.19	0.00	0.00	12.62	0.36	56.316	1.59	1686	47.75	675.79	62.78	542.00	540.09	
32.00	813	1.68	0.05	42.09	1.19	0.00	0.00	12.59	0.36	56.364	1.60	1630	46.16	676.37	62.83	541.92	540.06	
31.00	787	1.72	0.05	42.09	1.19	0.00	0.00	12.57	0.36	56.388	1.60	1574	44.56	676.66	62.86	541.83	540.04	
30.00	762	1.80	0.05	43.12	1.22	0.00	0.00	12.13	0.34	57.052	1.62	1517	42.96	684.62	63.60	541.75	540.01	
29.00	737	1.84	0.05	43.12	1.22	0.00	0.00	12.12	0.34	57.076	1.62	1460	41.35	684.91	63.63	541.67	539.99	
28.00	711	1.92	0.05	43.12	1.22	0.00	0.00	12.08	0.34	57.124	1.62	1403	39.73	685.49	63.68	541.58	539.96	
27.00	686	1.96	0.06	44.15	1.25	0.00	0.00	11.66	0.33	57.764	1.64	1346	38.11	693.17	64.40	541.50	539.94	
26.00	660	2.00	0.06	44.15	1.25	0.00	0.00	11.64	0.33	57.788	1.64	1288	36.48	693.46	64.42	541.42	539.91	
25.00	635	2.04	0.06	45.17	1.28	0.00	0.00	11.21	0.32	58.428	1.65	1230	34.84	701.14	65.14	541.33	539.89	
24.00	610	2.12	0.06	45.17	1.28	0.00	0.00	11.18	0.32	58.476	1.66	1172	33.19	701.71	65.19	541.25	539.86	
23.00	584	2.16	0.06	46.20	1.31	0.00	0.00	10.76	0.30	59.116	1.67	1114	31.53	709.39	65.90	541.17	539.83	
22.00	559	2.20	0.06	46.20	1.31	0.00	0.00	10.74	0.30	59.140	1.67	1054	29.86	709.68	65.93	541.08	539.81	
21.00	533	2.24	0.06	46.20	1.31	0.00	0.00	10.72	0.30	59.164	1.68	995	28.18	709.97	65.96	541.00	539.78	
20.00	508	2.28	0.06	46.20	1.31	0.00	0.00	10.71	0.30	59.188	1.68	936	26.51	710.26	65.98	540.92	539.76	
19.00	483	2.36	0.07	47.23	1.34	0.00	0.00	10.27	0.29	59.852	1.69	877	24.83	718.22	66.72	540.83	539.73	
18.00	457	2.40	0.07	47.23	1.34	0.00	0.00	10.25	0.29	59.876	1.70	817	23.14	718.51	66.75	540.75	539.71	
17.00	432	2.48	0.07	47.23	1.34	0.00	0.00	10.22	0.29	59.924	1.70	757	21.44	719.09	66.80	540.67	539.68	
16.00	406	2.52	0.07	47.23	1.34	0.00	0.00	10.20	0.29	59.948	1.70	697	19.74	719.38	66.83	540.58	539.66	
15.00	381	2.56	0.07	48.25	1.37	0.00	0.00	9.77	0.28	60.588	1.72	637	18.05	727.06	67.54	540.50	539.63	
14.00	356	2.60	0.07	48.25	1.37	0.00	0.00	9.76	0.28	60.612	1.72	577	16.33	727.34	67.57	540.42	539.61	
13.00	330	2.60	0.07	49.28	1.40	0.00	0.00	9.35	0.26	61.228	1.73	516	14.61	734.74	68.26	540.33	539.58	
12.00	305	2.64	0.07	49.28	1.40	0.00	0.00	9.33	0.26	61.252	1.73	455	12.88	735.02	68.28	540.25	539.55	
11.00	279	2.68	0.08	49.28	1.40	0.00	0.00	9.32	0.26	61.276	1.74	394	11.15	735.31	68.31	540.17	539.53	
10.00	254	2.96	0.08	49.28	1.40	0.00	0.00	9.20	0.26	61.444	1.74	332	9.41	737.33	68.50	540.08	539.50	
9.00	229	0.00	0.00	0.00	0.00	0.00	0.00	30.10	0.85	30.100	0.85	271	7.67	361.20	33.56	540.00	539.48	Bottom of Chamber Elevation
8.00	203	0.00	0.00	0.00	0.00	0.00	0.00	30.10	0.85	30.100	0.85	241	6.82	361.20	33.56	539.92	539.45	
7.00	178	0.00	0.00	0.00	0.00	0.00	0.00	30.10	0.85	30.100	0.85	211	5.97	361.20	33.56	539.83	539.43	
6.00	152	0.00	0.00	0.00	0.00	0.00	0.00	30.10	0.85	30.100	0.85	181	5.11	361.20	33.56	539.75	539.40	
5.00	127	0.00	0.00	0.00	0.00	0.00	0.00	30.10	0.85	30.100	0.85	151	4.26	361.20	33.56	539.67	539.38	
4.00	102	0.00	0.00	0.00	0.00	0.00	0.00	30.10	0.85	30.100	0.85	120	3.41	361.20	33.56	539.58	539.35	
3.00	76	0.00	0.00	0.00	0.00	0.00	0.00	30.10	0.85	30.100	0.85	90	2.56	361.20	33.56	539.50	539.33	
2.00	51	0.00	0.00	0.00	0.00	0.00	0.00	30.10	0.85	30.100	0.85	60	1.70	361.20	33.56	539.42	539.30	
1.00	25	0.00	0.00	0.00	0.00	0.00	0.00	30.10	0.85	30.100	0.85	30	0.85	361.20	33.56	539.33	539.28	
0.00	0	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.000	0.00	0	0.00	0.00	0.00	539.25	539.25	Bottom of Stone Elevation

SYSTEM STAGE-STORAGE TABLE

CULTEC STORMWATER CHAMBER

PROJECT NO:	23-1534.00
DATE:	12/8/2023
DESIGNED BY:	JIG
CHECKED BY:	JMG
SCALE:	N.T.S.
SHEET NO:	3 OF 5

DOLLAR GENERAL
US ROUTE 60
HUNTINGTON, WV
SYSTEM CALCULATION SHEET

CULTEC
Subsurface Stormwater Management Systems
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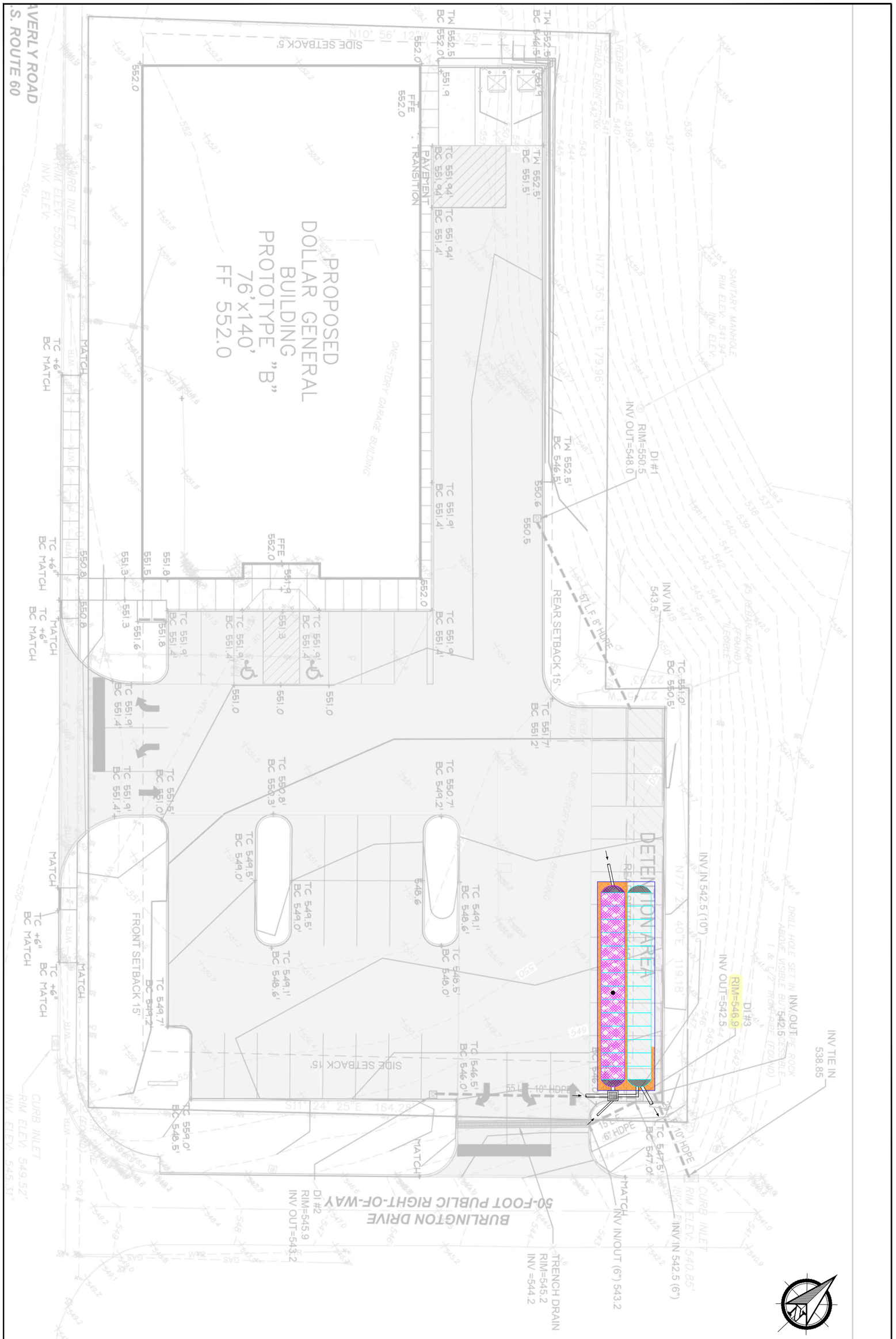

CULTEC
Subsurface Stormwater Management Systems

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www.cultec.com

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PH: 1(800) 4-CULTEC
CT-tech@cultec.com

1.02

DOLLAR GENERAL

US ROUTE 60

HUNTINGTON, WV

SYSTEM OVERLAY SHEET

CULTEC STORMWATER CHAMBER

PROJECT NO:	23-1534.00	**DATE:**	12/8/2023
DESIGNED BY:	JIG	**CHECKED BY:**	JMG
SCALE:	N.T.S.	**SHEET NO:**	4 OF 5

CULTEC RECHARGER® 902HD SPECIFICATIONS

GENERAL

CULTEC RECHARGER® 902HD CHAMBERS ARE DESIGNED FOR UNDERGROUND STORMWATER MANAGEMENT. THE CHAMBERS MAY BE USED FOR RETENTION, RECHARGING, DETENTION OR CONTROLLING THE FLOW OF ON-SITE STORMWATER RUNOFF.

CHAMBER PARAMETERS

1. THE CHAMBERS SHALL BE MANUFACTURED IN THE U.S.A. OR CANADA BY CULTEC OF BROOKFIELD, CT (CULTEC.COM, 203-775-4416).
2. THE CHAMBERS SHALL BE DESIGNED AND TESTED IN ACCORDANCE WITH ASTM F2787 "STANDARD PRACTICE FOR STRUCTURAL DESIGN OF THERMOPLASTIC CORRUGATED WALL STORMWATER COLLECTION CHAMBERS". THE LOAD CONFIGURATION SHALL INCLUDE:
   A. INSTANTANEOUS AASHTO DESIGN TRUCK LIVE LOAD AT MINIMUM COVER
   B. MAXIMUM PERMANENT (50-YEAR) COVER LOAD
   C. 1-WEEK PARKED AASHTO DESIGN TRUCK LOAD
3. THE CHAMBERS SHALL MEET THE REQUIREMENTS OF ASTM F3430-20 "STANDARD SPECIFICATION FOR CELLULAR POLYPROPYLENE (PP) CORRUGATED WALL STORMWATER COLLECTION CHAMBERS".
4. THE INSTALLED CHAMBER SYSTEM SHALL PROVIDE RESISTANCE TO THE LOADS AND LOAD FACTORS AS DEFINED IN THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS SECTION 12.12, WHEN INSTALLED ACCORDING TO CULTEC'S RECOMMENDED INSTALLATION INSTRUCTIONS. THE STRUCTURAL DESIGN OF THE CHAMBERS SHALL INCLUDE THE FOLLOWING:
   A. THE CREEP MODULUS SHALL BE 50-YEAR AS SPECIFIED IN ASTM F3430
   B. THE MINIMUM SAFETY FACTOR FOR LIVE LOADS SHALL BE 1.75
   C. THE MINIMUM SAFETY FACTOR FOR DEAD LOADS SHALL BE 1.95
5. THE CHAMBER SHALL BE STRUCTURAL FOAM INJECTION MOLDED OF BLUE VIRGIN HIGH MOLECULAR WEIGHT IMPACT-MODIFIED POLYPROPYLENE.
6. THE CHAMBER SHALL BE ARCHED IN SHAPE.
7. THE CHAMBER SHALL BE OPEN-BOTTOMED.
8. THE CHAMBER SHALL BE JOINED USING AN INTERLOCKING OVERLAPPING RIB METHOD. CONNECTIONS MUST BE FULLY SHOULDERED OVERLAPPING RIBS, HAVING NO SEPARATE COUPLINGS.
9. THE NOMINAL CHAMBER DIMENSIONS OF THE CULTEC RECHARGER® 902HD SHALL BE 48 INCHES (1219 MM) TALL, 78 INCHES (1981 MM) WIDE AND 4.25 FEET (1.30 M) LONG. THE INSTALLED LENGTH OF A JOINED RECHARGER 902HD SHALL BE 3.67 FEET (1.12 M).
10. MULTIPLE CHAMBERS MAY BE CONNECTED TO FORM DIFFERENT LENGTH ROWS. EACH ROW SHALL BEGIN AND END WITH A SEPARATELY FORMED CULTEC RECHARGER® 902HD END CAP. MAXIMUM INLET OPENING ON THE END CAP IS 30 INCHES (750 MM) HDPE OR 36 INCHES (900 MM) PVC.
11. THE CHAMBER SHALL HAVE TWO SIDE PORTALS TO ACCEPT CULTEC HVLV™ FC-48 FEED CONNECTORS TO CREATE AN INTERNAL MANIFOLD. MAXIMUM ALLOWABLE PIPE SIZE IN THE SIDE PORTAL IS 10 INCHES (250 MM) HDPE AND 12 INCHES (300 MM) PVC.
12. THE NOMINAL CHAMBER DIMENSIONS OF THE CULTEC HVLV™ FC-48 FEED CONNECTOR SHALL BE 12 INCHES (305 MM) TALL, 16 INCHES (406 MM) WIDE AND 49 INCHES (1245 MM) LONG.
13. THE NOMINAL STORAGE VOLUME OF THE RECHARGER 902HD CHAMBER SHALL BE 17.31 FT^3/ FT (1.61 M^3 / M) - WITHOUT STONE. THE NOMINAL STORAGE VOLUME OF A JOINED RECHARGER 902HD SHALL BE 63.47 FT^3 / UNIT (1.80 M^3 / UNIT) - WITHOUT STONE.
14. THE NOMINAL STORAGE VOLUME OF THE HVLV™ FC-48 FEED CONNECTOR SHALL BE 0.913 FT^3 / FT (0.085 M^3 / M) - WITHOUT STONE.
15. THE RECHARGER 902HD CHAMBER SHALL HAVE 5 CORRUGATIONS.
16. THE CHAMBER SHALL BE CAPABLE OF ACCEPTING A 6 INCH (150 MM) INSPECTION PORT OPENING AT THE TOP CENTER OF EACH CHAMBER, CENTERED ON THE CORRUGATION CREST.
17. THE CHAMBER SHALL BE MANUFACTURED IN A FACILITY EMPLOYING CULTEC'S QUALITY CONTROL AND ASSURANCE PROCEDURES.
18. MAXIMUM ALLOWABLE COVER OVER THE TOP OF THE CHAMBER SHALL BE 8.3 FEET (2.53 M).

END CAP PARAMETERS

1. THE CULTEC RECHARGER® 902HD END CAP (REFERRED TO AS 'END CAP') SHALL BE MANUFACTURED IN THE U.S.A. BY CULTEC OF BROOKFIELD, CT (CULTEC.COM, 203-775-4416).
2. THE END CAP SHALL BE STRUCTURAL FOAM INJECTION MOLDED OF BLUE VIRGIN HIGH MOLECULAR WEIGHT IMPACT-MODIFIED POLYPROPYLENE.
3. THE END CAP SHALL BE ARCHED IN SHAPE.
4. THE END CAP SHALL BE JOINED AT THE BEGINNING AND END OF EACH ROW OF CHAMBERS USING AN INTERLOCKING OVERLAPPING RIB METHOD. CONNECTIONS MUST BE FULLY SHOULDERED OVERLAPPING RIBS, HAVING NO SEPARATE COUPLINGS.
5. THE END CAP SHALL HAVE 5 CORRUGATIONS.
6. THE NOMINAL DIMENSIONS OF THE END CAP SHALL BE 48.5 INCHES (1231 mm) TALL, 78 INCHES (1982 mm) WIDE AND 28.0 INCHES (711 mm) LONG. WHEN JOINED WITH A RECHARGER 902HD CHAMBER, THE INSTALLED LENGTH OF THE END CAP SHALL BE 24.0 INCHES (610 mm).
7. THE NOMINAL STORAGE VOLUME OF THE END CAP SHALL BE 9.01 FT3 / FT (0.83 m3 / m) - WITHOUT STONE. THE NOMINAL STORAGE VOLUME OF AN INTERLOCKED END CAP SHALL BE 18.02 FT3 / UNIT (0.51 m3 / UNIT) - WITHOUT STONE.
8. MAXIMUM INLET OPENING ON THE END CAP IS 30 INCHES (750 mm) HDPE OR 36 INCHES (900 mm) PVC.
9. THE END CAP SHALL PROVIDE RESISTANCE TO THE LOADS AND LOAD FACTORS AS DEFINED IN THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS SECTION 12.12.

CULTEC HVLV FC-48 FEED CONNECTOR PRODUCT SPECIFICATIONS

GENERAL

CULTEC HVLV FC-48 FEED CONNECTORS ARE DESIGNED TO CREATE AN INTERNAL MANIFOLD FOR CULTEC RECHARGER MODEL 902HD STORMWATER CHAMBERS.

FEED CONNECTOR PARAMETERS

1. THE FEED CONNECTOR SHALL BE MANUFACTURED BY CULTEC OF BROOKFIELD, CT. (203-775-4416 OR 1-800-428-5832)
2. THE FEED CONNECTOR SHALL BE VACUUM THERMOFORMED OF BLACK HIGH MOLECULAR WEIGHT HIGH DENSITY POLYETHYLENE (HMWHDPE).
3. THE FEED CONNECTOR SHALL BE ARCHED IN SHAPE.
4. THE FEED CONNECTOR SHALL BE OPEN-BOTTOMED.
5. THE NOMINAL DIMENSIONS OF THE CULTEC HVLV FC-48 FEED CONNECTOR SHALL BE 12 INCHES (305 mm) TALL, 16 INCHES (406 mm) WIDE AND 49 INCHES (1245 mm) LONG.
6. THE NOMINAL STORAGE VOLUME OF THE HVLV FC-48 FEED CONNECTOR SHALL BE 0.913 FT^3 / FT (0.085 m^3 / m) - WITHOUT STONE.
7. THE HVLV FC-48 FEED CONNECTOR SHALL HAVE 4 CORRUGATIONS.
8. THE HVLV FC-48 FEED CONNECTOR MUST BE FORMED AS A WHOLE UNIT HAVING TWO OPEN END WALLS AND HAVING NO SEPARATE END PLATES OR SEPARATE END WALLS. THE UNIT SHALL FIT INTO THE SIDE PORTALS OF THE CULTEC RECHARGER STORMWATER CHAMBER AND ACT AS CROSS FEED CONNECTIONS CREATING AN INTERNAL MANIFOLD.
9. THE FEED CONNECTOR SHALL BE DESIGNED TO WITHSTAND AASHTO HS-25 DEFINED LOADS WHEN INSTALLED ACCORDING TO CULTEC'S RECOMMENDED INSTALLATION INSTRUCTIONS.
10. THE FEED CONNECTOR SHALL BE MANUFACTURED IN AN ISO 9001:2008 CERTIFIED FACILITY.

CULTEC NO. 410™ NON-WOVEN GEOTEXTILE

CULTEC NO. 410™ NON-WOVEN GEOTEXTILE MAY BE USED WITH CULTEC CONTACTOR® AND RECHARGER® STORMWATER INSTALLATIONS TO PROVIDE A BARRIER THAT PREVENTS SOIL INTRUSION INTO THE STONE.

GEOTEXTILE PARAMETERS

1. THE GEOTEXTILE SHALL BE PROVIDED BY CULTEC OF BROOKFIELD, CT. (203-775-4416 OR 1-800-428-5832)
2. THE GEOTEXTILE SHALL BE BLACK IN APPEARANCE.
3. THE GEOTEXTILE SHALL HAVE A TYPICAL WEIGHT OF 4.5 OZ/SY (142 G/M).
4. THE GEOTEXTILE SHALL HAVE A TENSILE STRENGTH VALUE OF 120 LBS (533 N) PER ASTM D4632 TESTING METHOD.
5. THE GEOTEXTILE SHALL HAVE AN ELONGATION @ BREAK VALUE OF 50% PER ASTM D4632 TESTING METHOD.
6. THE GEOTEXTILE SHALL HAVE A MULLEN BURST VALUE OF 225 PSI (1551 KPA) PER ASTM D3786 TESTING METHOD.
7. THE GEOTEXTILE SHALL HAVE A PUNCTURE STRENGTH VALUE OF 65 LBS (289 N) PER ASTM D4833 TESTING METHOD.
8. THE GEOTEXTILE SHALL HAVE A CBR PUNCTURE VALUE OF 340 LBS (1513 N) PER ASTM D6241 TESTING METHOD.
9. THE GEOTEXTILE SHALL HAVE A TRAPEZOID TEAR VALUE OF 50 LBS (222 N) PER ASTM D4533 TESTING METHOD.
10. THE GEOTEXTILE SHALL HAVE A AOS VALUE OF 70 U.S. SIEVE (0.212 MM) PER ASTM D4751 TESTING METHOD.
11. THE GEOTEXTILE SHALL HAVE A PERMITTIVITY VALUE OF 1.7 SEC-1 PER ASTM D4491 TESTING METHOD.
12. THE GEOTEXTILE SHALL HAVE A WATER FLOW RATE VALUE OF 135 GAL/MIN/SF (5500 L/MIN/SM) PER ASTM D4491 TESTING METHOD.
13. THE GEOTEXTILE SHALL HAVE A UV STABILITY @ 500 HOURS VALUE OF 70% PER ASTM D4355 TESTING METHOD.

CULTEC AFAB-HPF™ WOVEN GEOTEXTILE

CULTEC AFAB-HPF WOVEN GEOTEXTILE IS DESIGNED AS A UNDERLAYMENT TO PREVENT SCOURING CAUSED BY WATER MOVEMENT WITHIN THE CULTEC CHAMBERS AND FEED CONNECTORS UTILIZING THE CULTEC MANIFOLD FEATURE. IT MAY ALSO BE USED AS A COMPONENT OF THE CULTEC SEPARATOR ROW TO ACT AS A BARRIER TO PREVENT SOIL/CONTAMINANT INTRUSION INTO THE STONE WHILE ALLOWING FOR MAINTENANCE.

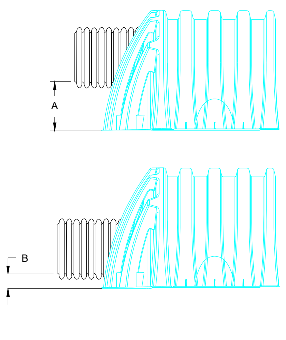
GEOTEXTILE PARAMETERS

1. THE GEOTEXTILE SHALL BE PROVIDED BY CULTEC OF BROOKFIELD, CT. (203-775-4416 OR 1-800-428-5832)
2. THE GEOTEXTILE SHALL BE BLACK IN APPEARANCE.
3. THE GEOTEXTILE SHALL HAVE A TENSILE STRENGTH OF 320 X 320 LBS (1,420 X 1,420 N) PER ASTM D4632 TESTING METHOD.
4. THE GEOTEXTILE SHALL HAVE A ELONGATION @ BREAK RESISTANCE OF 15 X 15% PER ASTM D4632 TESTING METHOD.
5. THE GEOTEXTILE SHALL HAVE A WIDE WIDTH TENSILE RESISTANCE OF 3,563 X 3,563 LBS/FT (52 X 52 KN/M) PER ASTM D4595 TESTING METHOD.
6. THE GEOTEXTILE SHALL HAVE A CBR PUNCTURE RESISTANCE OF 1,500 LBS (6,670 N) PER ASTM D6241 TESTING METHOD.
7. THE GEOTEXTILE SHALL HAVE A TRAPEZOIDAL TEAR RESISTANCE OF 120 X 120 LBS (540 X 540 N) PER ASTM D4533 TESTING METHOD.
8. THE GEOTEXTILE SHALL HAVE AN APPARENT OPENING SIZE OF 30 US STD. SIEVE (0.60 MM) PER ASTM D4751 TESTING METHOD.
9. THE GEOTEXTILE SHALL HAVE A PERMITTIVITY RATING OF 0.2 SEC-1 PER ASTM D4491 TESTING METHOD.
10. THE GEOTEXTILE SHALL HAVE A WATER FLOW RATING OF 22 GPM/FT2 (900 LPM/M2) PER ASTM D4491 TESTING METHOD.
11. THE GEOTEXTILE SHALL HAVE A UV RESISTANCE OF 70% @ 500 HRS. PER ASTM D4355 TESTING METHOD.

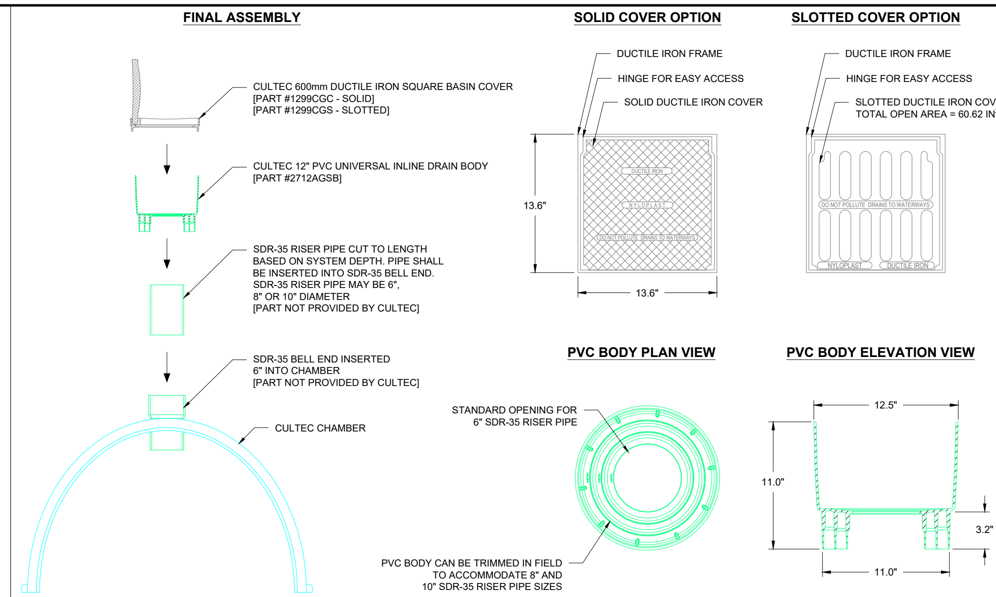
GENERAL NOTES

PIPE	A	B
6"	38.00"	1.00"
8"	36.00"	1.00"
10"	33.80"	1.25"
12"	29.25"	1.75"
15"	25.75"	2.00"
18"	21.75"	2.50"
21"	18.75"	2.50"
24"	15.75"	2.50"
30"	7.75"	3.50"
36"	N/A	3.50"

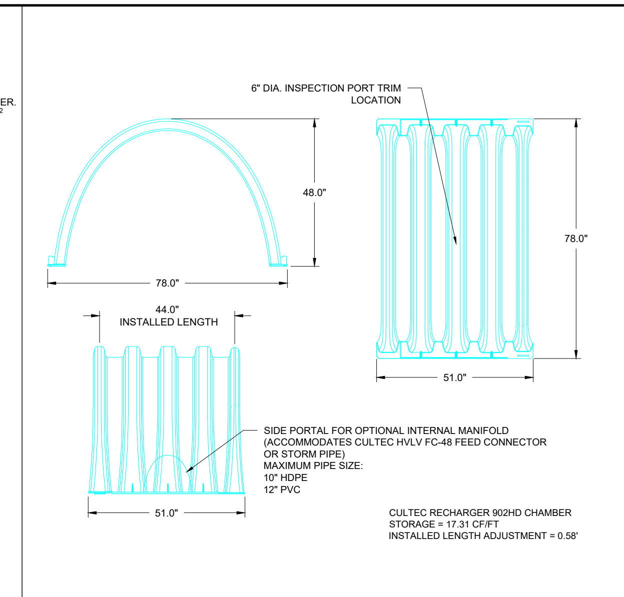
*THE TYPICAL INVERT TABLE ABOVE IS BASED ON THE INSIDE DIAMETER OF STANDARD CORRUGATED PLASTIC PIPE. THE HEAVY DUTY END CAP HAS PRE-MARKED TRIM LINES FOR PIPE DIAMETERS 12", 15", 18" AND 24".
PIPES OF ANY SIZE AND MATERIAL UP TO 24" MAY BE PLACED AT CUSTOM LOCATIONS AND CUSTOM INVERTS. THE CROWN OF THE PIPE MUST REMAIN A MINIMUM OF 4" FROM THE EDGE OF THE HEAVY DUTY END CAP.



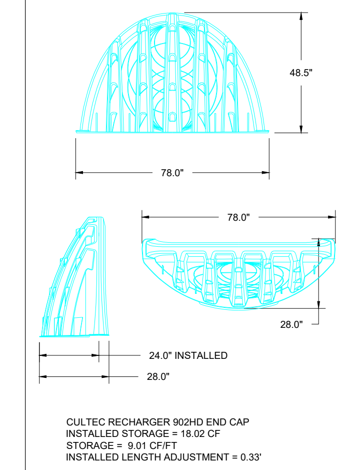
CULTEC RECHARGER 902HD TYPICAL PIPE INVERTS



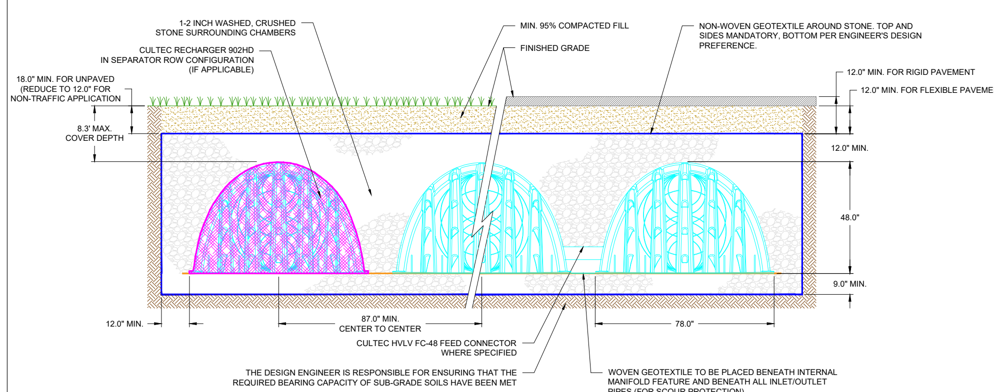
CULTEC UNIVERSAL INSPECTION PORT KIT DETAIL



CULTEC RECHARGER 902HD HEAVY DUTY THREE VIEW

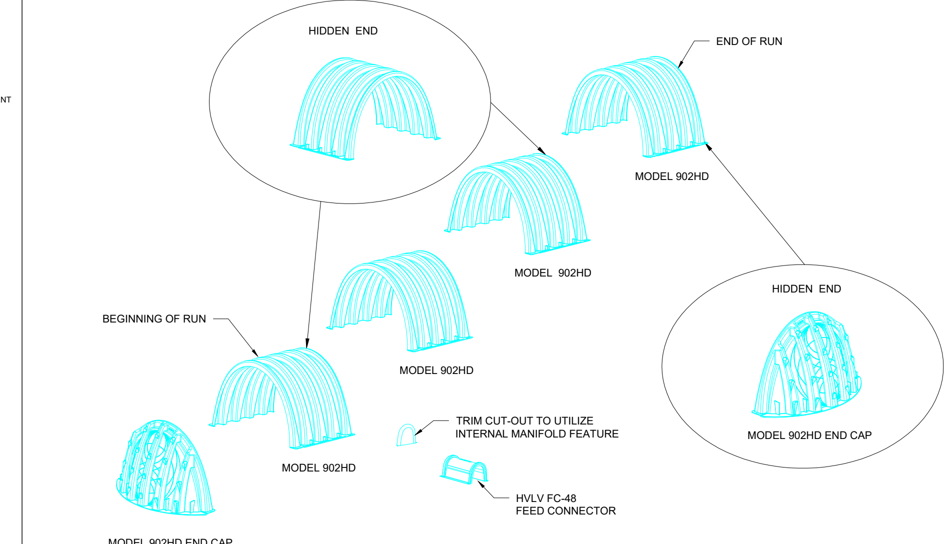


CULTEC RECHARGER 902HD HEAVY DUTY END CAP THREE VIEW

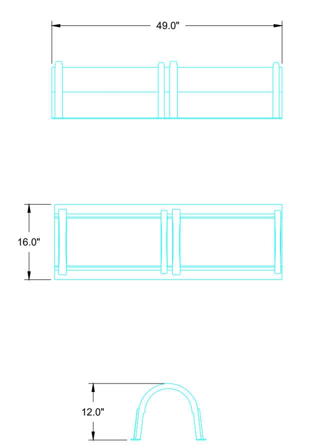


NOTES:
1. THE CHAMBERS SHALL BE DESIGNED AND TESTED IN ACCORDANCE WITH ASTM F2787 "STANDARD PRACTICE FOR STRUCTURAL DESIGN OF THERMOPLASTIC CORRUGATED WALL STORMWATER COLLECTION CHAMBERS." THE LOAD CONFIGURATION SHALL INCLUDE:
   1.a. INSTANTANEOUS AASHTO DESIGN TRUCK LIVE LOAD AT MINIMUM COVER
   1.b. MAXIMUM PERMANENT (50-YEAR) COVER LOAD
   1.c. 1-WEEK PARKED AASHTO DESIGN TRUCK LOAD
2. THE CHAMBERS SHALL MEET THE REQUIREMENTS OF ASTM F3430-20 "STANDARD SPECIFICATION FOR CELLULAR POLYPROPYLENE (PP) CORRUGATED WALL STORMWATER COLLECTION CHAMBERS"
3. THE INSTALLED CHAMBER SYSTEM SHALL PROVIDE RESISTANCE TO THE LOADS AND LOAD FACTORS AS DEFINED IN THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS SECTION 12.12, WHEN INSTALLED ACCORDING TO CULTEC'S RECOMMENDED INSTALLATION INSTRUCTIONS. THE STRUCTURAL DESIGN OF THE CHAMBERS SHALL INCLUDE THE FOLLOWING:
   3.a. THE CREEP MODULUS SHALL BE 50-YEAR AS SPECIFIED IN ASTM F3430
   3.b. THE MINIMUM SAFETY FACTOR FOR LIVE LOADS SHALL BE 1.75
   3.c. THE MINIMUM SAFETY FACTOR FOR DEAD LOADS SHALL BE 1.95

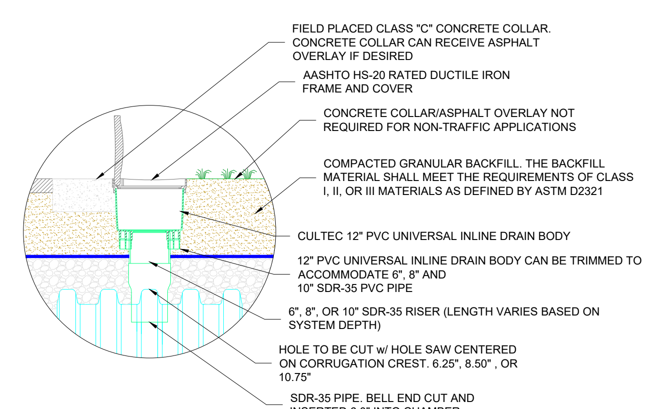
CULTEC RECHARGER 902HD CROSS SECTION



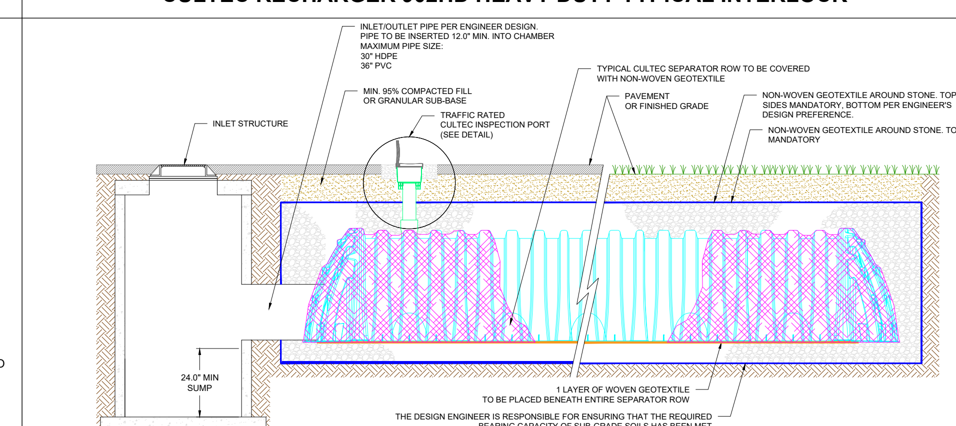
CULTEC RECHARGER 902HD HEAVY DUTY TYPICAL INTERLOCK



CULTEC HVLV FC-48 FEED CONNECTOR THREE VIEW

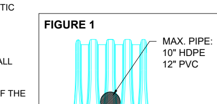


OPTIONAL CULTEC INSPECTION PORT - ZOOM DETAIL



NOTES:
1. THE CHAMBERS SHALL BE DESIGNED AND TESTED IN ACCORDANCE WITH ASTM F2787 "STANDARD PRACTICE FOR STRUCTURAL DESIGN OF THERMOPLASTIC CORRUGATED WALL STORMWATER COLLECTION CHAMBERS." THE LOAD CONFIGURATION SHALL INCLUDE:
   1.a. INSTANTANEOUS AASHTO DESIGN TRUCK LIVE LOAD AT MINIMUM COVER
   1.b. MAXIMUM PERMANENT (50-YEAR) COVER LOAD
   1.c. 1-WEEK PARKED AASHTO DESIGN TRUCK LOAD
2. THE CHAMBERS SHALL MEET THE REQUIREMENTS OF ASTM F3430-20 "STANDARD SPECIFICATION FOR CELLULAR POLYPROPYLENE (PP) CORRUGATED WALL STORMWATER COLLECTION CHAMBERS"
3. THE INSTALLED CHAMBER SYSTEM SHALL PROVIDE RESISTANCE TO THE LOADS AND LOAD FACTORS AS DEFINED IN THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS SECTION 12.12, WHEN INSTALLED ACCORDING TO CULTEC'S RECOMMENDED INSTALLATION INSTRUCTIONS. THE STRUCTURAL DESIGN OF THE CHAMBERS SHALL INCLUDE THE FOLLOWING:
   3.a. THE CREEP MODULUS SHALL BE 50-YEAR AS SPECIFIED IN ASTM F3430
   3.b. THE MINIMUM SAFETY FACTOR FOR LIVE LOADS SHALL BE 1.75
   3.c. THE MINIMUM SAFETY FACTOR FOR DEAD LOADS SHALL BE 1.95

FIGURE 1



ZOOM OF SIDE PORTAL SHOWING MAX. PIPE O.D.

CULTEC SEPARATOR ROW - CULTEC INSPECTION PORT DETAIL (IF APPLICABLE)

CULTEC STORMWATER CHAMBER

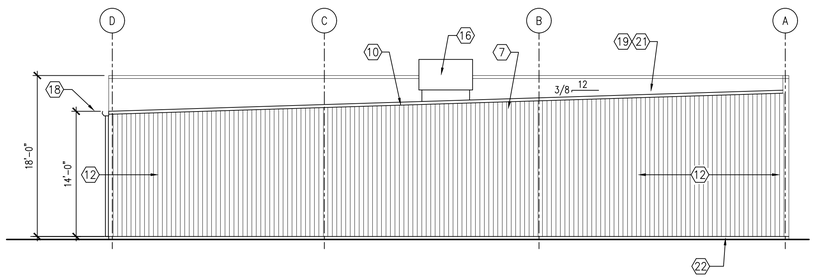
PROJECT NO:	23-1534.00	**DATE:**	12/8/2023
DESIGNED BY:	JIG	**CHECKED BY:**	JMG
SCALE:	N.T.S.	**SHEET NO:**	5 OF 5

DOLLAR GENERAL
US ROUTE 60
HUNTINGTON, WV
902HD DETAIL SHEET

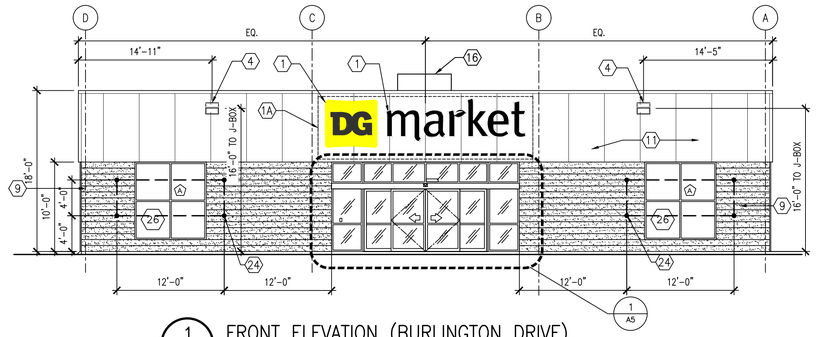
CULTEC
Subsurface Stormwater Management Systems
P.O. Box 280
878 Federal Road
Brookfield, CT 06804
www.cultec.com
PH: 1(203) 775-4416
PH: 1(800) 4-CULTEC
CT-tech@cultec.com

1.02

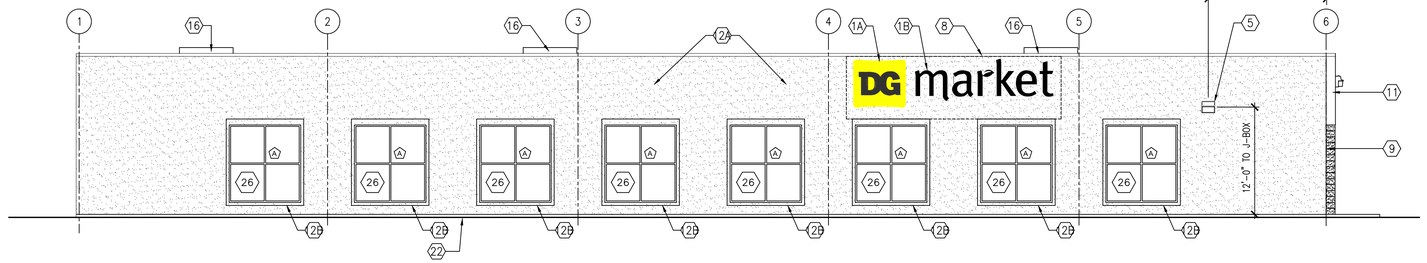
PEMB VENDOR	VP BUILDINGS					STAR BUILDING SYSTEMS					NUCOR BUILDING SYSTEMS					BIG BEE STEEL BUILDINGS					CHIEF BUILDINGS					COLORS WHERE EIFS IS USED INSTEAD OF METAL PANELS	
	ATTN: RANDY SPEARS 205-907-8176					ATTN: JEFF HORN 866-664-8899					ATTN: BOB BARRY 315-622-4440					ATTN: KEVIN BUSLER 800-633-3378					ATTN: ERIN SULLIVAN 800-845-1767						
FINISHES	COOL EGYPTIAN WHITE	COOL DARK BRONZE	BRONZE	COOL COTTON WHITE	GALVALUME	LIGHT STONE	MEDIUM BRONZE KYNAR 500	BRONZE	POLAR WHITE	GALVALUME	LIGHT STONE	MEDIUM BRONZE KYNAR 500	BRONZE	POLAR WHITE	GALVALUME	SANDSTONE	BURNISHED SLATE	BRONZE	POLAR WHITE	GALVALUME	PARCHMENT	ANTIQUE BRONZE	BRONZE	POLAR WHITE	GALVALUME	SHERWIN WILLIAMS #7103 "BALMACED BEIGE"	SHERWIN WILLIAMS #7674 "EXTRA MAPLE"
EXTERIOR FINISHES ARE TO MATCH OR BE EQUAL TO VP METAL BUILDING SYSTEM'S FINISH SELECTION UNLESSS AUTHORITY HAVING JURISDICTION DOES NOT ALLOW.																										
GUTTERS	•				•	•				•	•				•	•				•						
DOWN SPOUTS	•				•	•				•	•				•	•				•						
SIDE AND REAR WALLS (EIFS OR METAL WALL PANELS & TRIM) RECEIVING & EMERGENCY EXIT DOORS (EXTERIOR OF DOORS TO BE PAINTED, REFER TO DOOR SCHEDULE)	•					•					•					•					•					•	
ARCHITECTURAL BLOCK AT BUILDING FACADE TO BE PRE-FINISHED OR PAINTED (2 COATS - LOXON XP MASONRY COATING A24W400 SERIES) TO MATCH THE METAL WALL PANEL.	•					•					•					•					•						
FLAT METAL SOFFIT AT STOREFRONT VESTIBULE.				•					•					•					•					•			
BUILDING FASCIA WALL AND PARAPET OVER ENTRANCE																										•	
STOREFRONT SYSTEM AND SPANDREL WINDOWS			•					•					•					•					•				
STANDING SEAM METAL ROOF PANELS					•				•						•				•						•		
INTERIOR SALES AND RECEIVING FLOOR LINER PANELS				•					•					•					•					•			



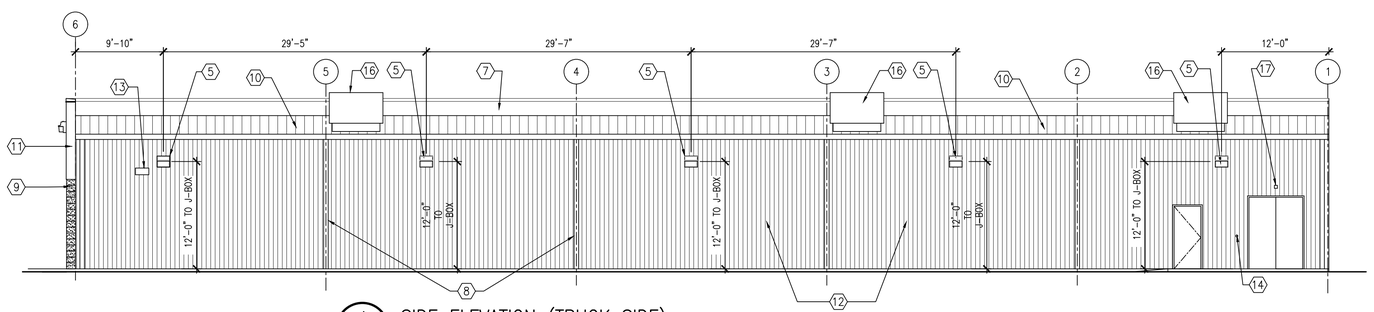
2 A2 REAR ELEVATION
1/8" = 1'-0"



1 A2 FRONT ELEVATION (BURLINGTON DRIVE)
1/8" = 1'-0"



3 A2 SIDE ELEVATION (WAVERLY ROAD)
1/8" = 1'-0"



4 A2 SIDE ELEVATION (TRUCK SIDE)
1/8" = 1'-0"

ELEVATION KEYED NOTES

1. SIGN FURNISHED AND INSTALLED BY DOLLAR GENERAL CORP. WITH CIRCUIT AS NOTED ON ELECTRICAL PLAN. SIGN TO BE CENTERED ON FRONT OF BUILDING. CONTRACTOR IS TO PROVIDE ADEQUATE BLOCKING AS REQUIRED BY SIGN MANUFACTURER TO SUPPORT SIGN WEIGHT OF UP TO 1,400 LBS. EXTERIOR CANOPY SIGN SHALL BE SUPPORTED BY THE FACE OF CANOPY. CONTRACTOR IS TO PROVIDE ADEQUATE STRUCTURE TO SUPPORT SIGN. COORDINATE THE PROPER SIGNAGE TO BE USED WITH DOLLAR GENERAL.

1A. PLYWOOD BLOCKING BEHIND METAL PANELS FOR SIGNAGE. 7'-0"X24'-0" FOR DGMM SIGNAGE.

1B. SECONDARY SIGN FURNISHED AND INSTALLED BY DOLLAR GENERAL CORP. WITH CIRCUIT AS NOTED ON ELECTRICAL PLAN. SIGN TO BE CENTERED OVER WINDOWS AS SHOWN. CONTRACTOR IS TO PROVIDE ADEQUATE BLOCKING AS REQUIRED BY SIGN MANUFACTURER TO SUPPORT SIGN WEIGHT OF UP TO 1,400 LBS. EXTERIOR CANOPY SIGN SHALL BE SUPPORTED BY THE FACE OF CANOPY. CONTRACTOR IS TO PROVIDE ADEQUATE STRUCTURE TO SUPPORT SIGN. COORDINATE THE PROPER SIGNAGE TO BE USED WITH DOLLAR GENERAL.

2. NOT USED.

3. NOT USED.

4. WALL PACK. REFER TO ELEC. DRAWINGS FOR ADDITIONAL INFO. 16'-0" HEIGHT O.C. TO J-BOX.

5. WALL PACK. REFER TO ELEC. DRAWINGS FOR ADDITIONAL INFO. 12'-0" HEIGHT O.C. TO J-BOX.

6. NOT USED.

7. TRIM - SEE FINISH SCHEDULE FOR COLOR.

8. GUTTER AND DOWNSPOUT - SEE FINISH SCHEDULE FOR COLOR.

9. 8" SMOOTH FACE CONCRETE MASONRY UNIT.

10. STANDING SEAM METAL ROOF.

11. PRE-FINISHED METAL WALL PANELS FOR FASCIA AND PARAPET OVER ENTRANCE, REVERSE RIB PROFILE.

12. PRE-FINISHED METAL WALL PANELS FOR SIDE AND REAR. PROVIDE TAMPER-RESISTANT FASTENERS FOR BOTTOM 8'-0".

2A. EIFS OVER METAL WALL PANELS FOR SIDE VISIBLE FROM MAIN STREET. PROVIDE TAMPER-RESISTANT FASTENERS FOR BOTTOM 8'-0".

2B. EIFS FRAMES AROUND SPANDREL WINDOWS AT SIDE.

13. VENT FOR BATHROOM EXHAUST. REFER TO MECHANICAL DRAWING M1 FOR ADDITIONAL INFORMATION.

14. DOOR BUZZER. REFER TO ELECTRICAL DRAWING E1 FOR ADDITIONAL INFORMATION.

15. NOT USED.

16. HVAC UNITS MOUNTED ON ROOF. REFER TO MECHANICAL SHEET M1 FOR MORE INFORMATION.

17. OUTSIDE AIR TEMP. SENSOR MOUNTED OVER RECEIVING DOOR AT 9'-0" A.F.F.

18. MINIMUM EAVE HEIGHT IS 14'-0" A.F.F.

19. PARAPET BEYOND.

20. IN NORTHERN CLIMATES, PROVIDE SNOW GUARDS ON ROOF PER LOCAL CODE.

21. EXTEND PARAPET WALL UP AS NEEDED TO SCREEN ROOF MOUNTED EQUIPMENT IF REQUIRED BY LOCAL ORDINANCE.

22. FINISH GRADE TO BE A MINIMUM OF 6" BELOW FINISHED FLOOR LEVEL AT ALL NONPAVED AREAS.

23. NOT USED.

24. 1/2" DIAMETER x 6" LONG STEEL EYE BOLTS (CLOSED) WITH 1" DIAMETER OPENINGS. DRILL AND EPOXY INTO BLOCK WALL. 4 BOLTS TO BE LOCATED AS SHOWN EACH SIDE OF ENTRY. TOTAL OF 8 BOLTS.

25. GUTTER LEAF GUARDS IF WITHIN 25'-0" OF A TREE.

26. SPANDREL WINDOW GLAZING - 8'-0" WIDE X 8'-8" TALL, CENTERED BETWEEN EYEBOLTS AND EXTENDING TO 10'-0" AFF

STEPHEN P. MAHER, ARCHITECT

ARCHITECTURE
PLANNING
GRAPHICS

2948 SIDCO DRIVE
NASHVILLE, TN 37204
(p) 615.244.8170
(f) 615.244.8141
www.mjmarch.com

CONSULTANT SEAL

OWNER

DG BTS HUNTINGTON, LLC

2525 Broad Street
Chattanooga TN 37408
mikeberry@berryconstruction
423-488-4053

LOCATION

DOLLAR GENERAL
STORE #30003
WAVERLY ROAD
HUNTINGTON, WV 25704

HUNTINGTON, WV

DATE

DOLLAR GENERAL PERMIT SET
1.9.24
MJM #23481

SHEET

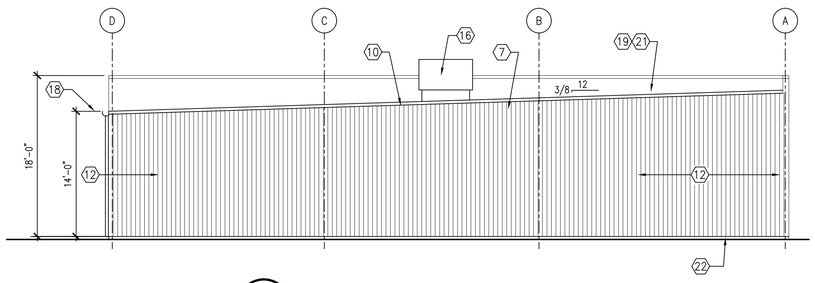
EXTERIOR ELEVATIONS

1/8" = 1'-0"

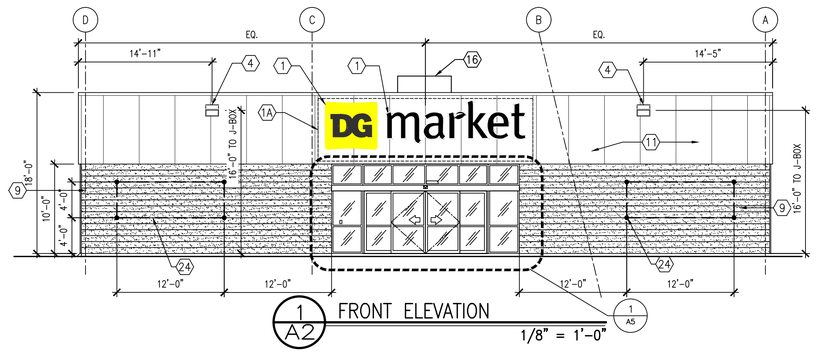
A2

FINISHES

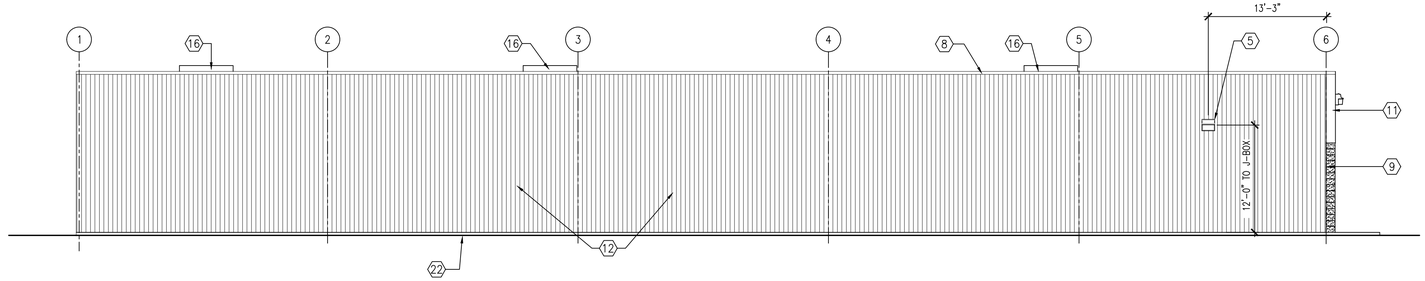
EXTERIOR FINISHES ARE TO MATCH OR BE EQUAL TO VP METAL BUILDING SYSTEM'S FINISH SELECTION UNLESSS AUTHORITY HAVING JURISDICTION DOES NOT ALLOW.	PEMB VENDOR	VP BUILDINGS ATTN: RANDY SPEARS 205-907-8176					STAR BUILDING SYSTEMS ATTN: JEFF HORN 866-664-8899					NUCOR BUILDING SYSTEMS ATTN: BOB BARRY 315-622-4440					BIG BEE STEEL BUILDINGS ATTN: KEVIN BUSLER 800-633-3378					CHIEF BUILDINGS ATTN: ERIN SULLIVAN 800-845-1767				
	KOOL KOTE EGYPTIAN	KOOL DARK BRONZE	BRONZE	KOOL COTTON	GALVALUME	LIGHT STONE	MEDIUM BRONZE KYNAR 500	BRONZE	POLAR WHITE	GALVALUME	LIGHT STONE	MEDIUM BRONZE KYNAR 500	BRONZE	POLAR WHITE	GALVALUME	SANDSTONE	BURNISHED SLATE	BRONZE	POLAR WHITE	GALVALUME	PARCHMENT	ANTIQUE BRONZE	BRONZE	POLAR WHITE	GALVALUME
GUTTERS	•					•					•					•					•				
DOWN SPOUTS	•					•					•					•					•				
SIDE AND REAR METAL WALL PANELS & TRIM, RECEIVING & EMERGENCY EXIT DOORS (EXTERIOR OF DOORS TO BE PAINTED, REFER TO DOOR SCHEDULE)	•					•					•					•					•				
ARCHITECTURAL BLOCK AT BUILDING FACADE TO BE PRE-FINISHED OR PAINTED (2 COATS – LOXON XP MASONRY COATING A24W400 SERIES) TO MATCH THE METAL WALL PANEL.	•					•					•					•					•				
FLAT METAL SOFFIT AT STOREFRONT VESTIBULE.				•					•					•					•					•	
BUILDING FASCIA WALL, PARAPET OVER ENTRANCE, AND CANOPY		•					•					•					•					•			
STOREFRONT SYSTEM			•					•					•					•					•			
STANDING SEAM METAL ROOF PANELS					•					•					•					•					•
INTERIOR SALES AND RECEIVING FLOOR LINER PANELS				•					•					•					•					•	



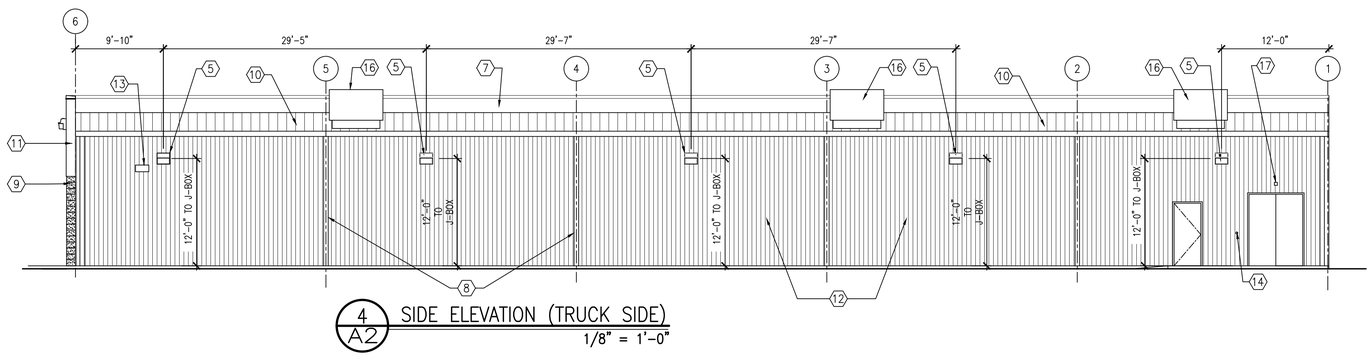
2 / A2 REAR ELEVATION
1/8" = 1'-0"



1 / A2 FRONT ELEVATION
1/8" = 1'-0"



3 / A2 SIDE ELEVATION (NO ACCESS)
1/8" = 1'-0"



4 / A2 SIDE ELEVATION (TRUCK SIDE)
1/8" = 1'-0"

ELEVATION KEYED NOTES

1. SIGN FURNISHED AND INSTALLED BY DOLLAR GENERAL CORP. WITH CIRCUIT AS NOTED ON ELECTRICAL PLAN. SIGN TO BE CENTERED ON FRONT OF BUILDING. CONTRACTOR IS TO PROVIDE ADEQUATE BLOCKING AS REQUIRED BY SIGN MANUFACTURER TO SUPPORT SIGN WEIGHT OF UP TO 1,400 LBS. EXTERIOR CANOPY SIGN SHALL BE SUPPORTED BY THE FACE OF CANOPY. CONTRACTOR IS TO PROVIDE ADEQUATE STRUCTURE TO SUPPORT SIGN. COORDINATE THE PROPER SIGNAGE TO BE USED WITH DOLLAR GENERAL.
- 1A. PLYWOOD BLOCKING BEHIND METAL PANELS FOR SIGNAGE. 4'-0"X28'-0" FOR DGP A/B/E/F LAYOUTS. 6'-0"X36'-0" FOR DGP A/B/E/F LAYOUTS. 7'-0"X24'-0" FOR ALL DGMM LAYOUTS.
2. NOT USED.
3. NOT USED.
4. WALL PACK. REFER TO ELEC. DRAWINGS FOR ADDITIONAL INFO. 16'-0" HEIGHT O.C. TO J-BOX.
5. WALL PACK. REFER TO ELEC. DRAWINGS FOR ADDITIONAL INFO. 12'-0" HEIGHT O.C. TO J-BOX.
6. NOT USED.
7. TRIM – SEE FINISH SCHEDULE FOR COLOR.
8. GUTTER AND DOWNSPOUT – SEE FINISH SCHEDULE FOR COLOR.
9. 8" SMOOTH FACE CONCRETE MASONRY UNIT.
10. STANDING SEAM METAL ROOF.
11. PRE-FINISHED METAL WALL PANELS FOR FASCIA AND PARAPET OVER ENTRANCE, REVERSE RIB PROFILE.
12. PRE-FINISHED METAL WALL PANELS FOR SIDE AND REAR. PROVIDE TAMPER-RESISTANT FASTENERS FOR BOTTOM 8'-0".
13. VENT FOR BATHROOM EXHAUST. REFER TO MECHANICAL DRAWING M1 FOR ADDITIONAL INFORMATION.
14. DOOR BUZZER. REFER TO ELECTRICAL DRAWING E1 FOR ADDITIONAL INFORMATION.
15. NOT USED.
16. HVAC UNITS MOUNTED ON ROOF. REFER TO MECHANICAL SHEET M1 FOR MORE INFORMATION.
17. OUTSIDE AIR TEMP. SENSOR MOUNTED OVER RECEIVING DOOR AT 9'-0" A.F.F.
18. MINIMUM EAVE HEIGHT IS 14'-0" A.F.F.
19. PARAPET BEYOND.
20. IN NORTHERN CLIMATES, PROVIDE SNOW GUARDS ON ROOF PER LOCAL CODE.
21. EXTEND PARAPET WALL UP AS NEEDED TO SCREEN ROOF MOUNTED EQUIPMENT IF REQUIRED BY LOCAL ORDINANCE.
22. FINISH GRADE TO BE A MINIMUM OF 6" BELOW FINISHED FLOOR LEVEL AT ALL NONPAVED AREAS.
23. NOT USED.
24. 1/2" DIAMETER x 6" LONG STEEL EYE BOLTS (CLOSED) WITH 1" DIAMETER OPENINGS. DRILL AND EPOXY INTO BLOCK WALL. 4 BOLTS TO BE LOCATED AS SHOWN EACH SIDE OF ENTRY. TOTAL OF 8 BOLTS.
25. GUTTER LEAF GUARDS IF WITHIN 25'-0" OF A TREE.

STEPHEN P. MAHER, ARCHITECT

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OWNER

DOLLAR GENERAL

STORE #30003
WAVERLY ROAD
HUNTINGTON, WV 25704

LOCATION

HUNTINGTON, WV

DOLLAR GENERAL PERMIT SET
1.9.24
MJM #23481

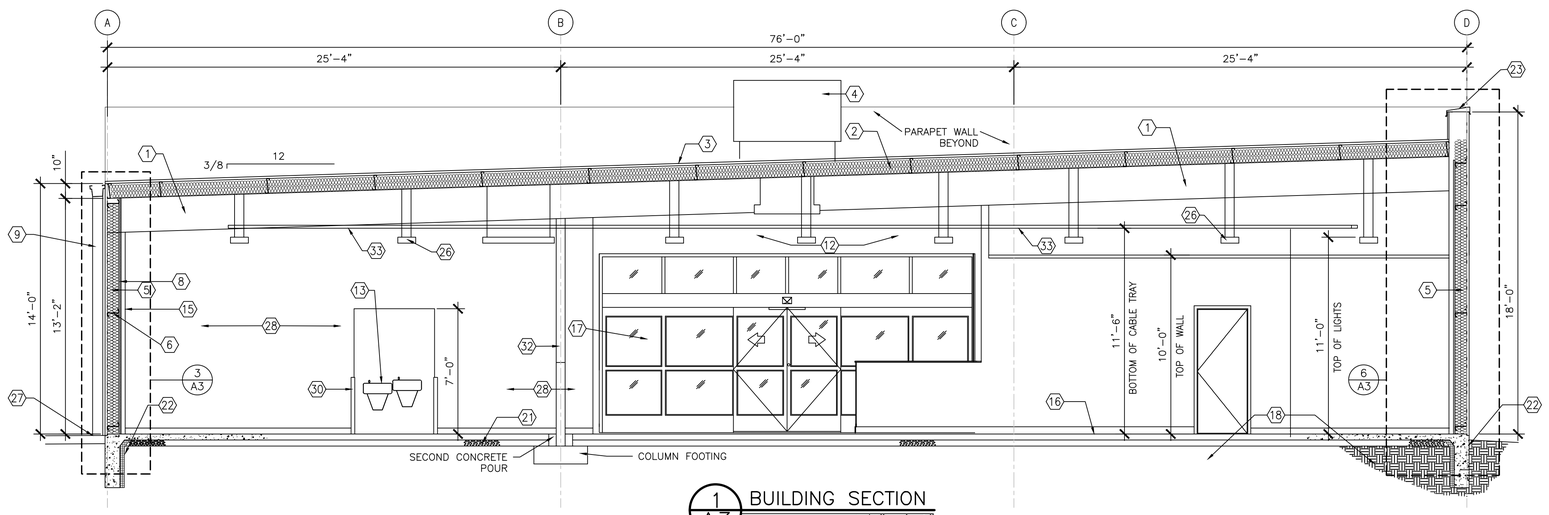
DATE

SHEET

EXTERIOR ELEVATIONS

1/8" = 1'-0"

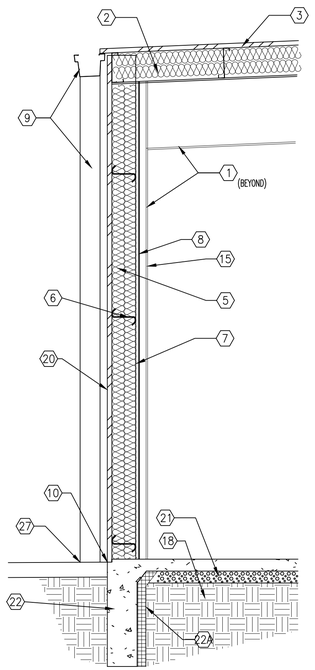
A2



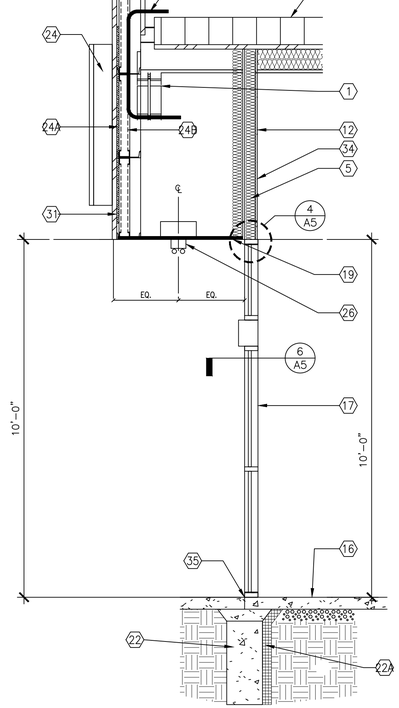
1 A3 BUILDING SECTION
1/4"=1'-0"

SECTION KEYED NOTES

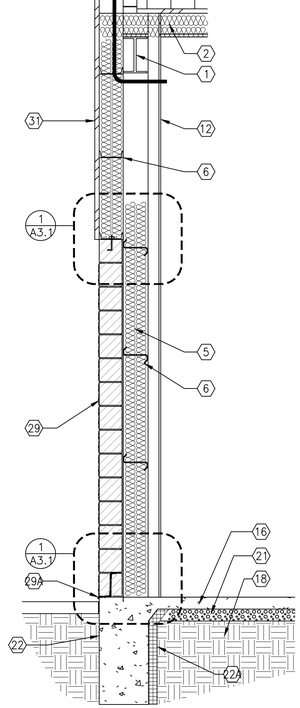
1. PRE-ENGINEERED METAL BUILDING SYSTEM BY BUILDING MANUF.
2. INSULATION SYSTEM (BY PEMB) TO LEAVE BOTTOM OF PURLINS EXPOSED FOR ATTACHMENTS – "SAG AND BAG" PREFERED, "LONG TAB/BANDED FILLED CAVITY" ACCEPTABLE. INSULATION TO BE MINIMUM R-25 BETWEEN PURLINS WITH AN R-15 LINER SYSTEM. INSULATION TO HAVE FLAME SPREAD OF NOT MORE THAN 25 AND SMOKE DEVELOPED OF NOT MORE THAN 450.
3. STANDING SEAM METAL ROOF – GALVALUME FINISH.
4. ROOF MOUNTED HVAC UNIT. SEE MECH.
5. INSULATION SYSTEM (BY PEMB). INSULATION TO BE MINIMUM R-30 CAVITY INFILL OR EQUIVALENT. INSULATION TO HAVE FLAME SPREAD OF NOT MORE THAN 25 AND SMOKE DEVELOPED OF NOT MORE THAN 450.
6. 8" METAL BUILDING GIRT. BEHIND STONE, SPACE LESS THAN 2'-0" O.C. APART TO SUPPORT ADDITIONAL WEIGHT
7. VERTICAL METAL LINER PANELS.
8. 1/2" GYPSUM BOARD(PAINTED) OVER LINER PANELS TO DECK ABOVE. SEE SHEET A4.
9. PRE-FINISHED DOWNSPOUT & GUTTER.
10. PROVIDE METAL CLOSURE, SEAL, AND CAULK TO RODENT PROOF BUILDING.
11. LOCATION OF 2" CONDUIT WITH PULLSTRING (ABOVE OFFICE) FOR SATELLITE LINE. PROVIDE WEATHERTIGHT SEAL AT HORIZONTAL PENETRATION. CONTRACTOR RESPONSIBLE FOR INSTALLATION OF CONDUIT. EXTEND CONDUIT TO OFFICE.
12. 1/2" GYPSUM BOARD TO DECK ON 3-5/8" METAL STUDS.
- 12A. 1/2" GYPSUM BOARD ON CEILING ON 3-5/8" METAL STUDS @ 16" O.C.
13. ADA COMPLIANT DRINKING FOUNTAIN, SEE PLUMBING DRAWINGS.
14. NOT USED.
15. METAL BUILDING COLUMNS SHALL BE STRAIGHT.
16. REIN. CONC. SLAB ON VINYL VAPOR BARRIER.
17. 21'X10' ALUMINUM BI-PART DOOR AND STOREFRONT GLAZING SYSTEM, COLOR BRONZE.
18. UNDISTURBED EARTH AND COMPACTED SUB GRADE.
19. SOFFIT BEYOND PROVIDED BY PRE-ENGINEERED BUILDING MANUF.
20. PRE-FINISHED METAL WALL PANELS.
21. GRANULAR FILL.
22. REINFORCED CONCRETE FOOTING. (REFER TO NOTES ON S2)
- 22A. 2'-0" MIN. R-10 PERIMETER INSULATION VERTICALLY AT FOUNDATIONS
23. PREFINISHED METAL COPING.
24. STOREFRONT SIGN (BY DOLLAR GENERAL). REFER TO SHEETS A02, A08 AND STRUCTURAL NOTES FOR REQUIRED BLOCKING
- 24A. ¾" THICK CDX PLYWOOD TO BE FLUSH WITH GIRTS
25. FINISH GRADE TO BE MINIMUM OF 6" BELOW FINISHED FLOOR AT ALL NONPAVED AREAS.
26. LIGHTING – SEE ELECTRICAL DRAWINGS.
27. TAKE DOWNSPOUTS UNDER SIDEWALK TO PAVING PER CIVIL DRAWINGS
28. INTERIOR WALL – PAINTED – SEE ROOM SCHEDULE FOR MORE INFORMATION.
29. 8" SPLIT-FACED CMU PAINTED SW 7037 "BALANCED BEIGE". ALIGN FACE OF BLOCK WITH STEEL GIRT. PROVIDE PROPER ANCHORAGE TO STRUCTURE.
- 29A. CONTINUOUS THRU-WALL FLASHING W/STAINLESS STEEL DRIP EDGE, PROVIDE WEEP HOLES @ 24" O.C.
30. MC CUE TRIM KIT.
31. PRE-FINISHED METAL FASCIA PANEL
32. METAL COLUMN, WRAP WITH CARPET TO 48" HIGH AT BASE.
33. CABLE TRAY – SEE A9
34. 20"X30" MINIMUM ACCESS PANEL FOR SIGNAGE JUNCTION BOX; COORDINATE LOCATION WITH ELECTRICAL DRAWINGS AND SIGNAGE VENDOR.
35. 1'x4' LIGHT FIXTURES HUNG WITH AIRCRAFT CABLE, ADD UNISTRUT TO PURLINS AS REQUIRED FOR HANGING.
36. BRACING FOR STOREFRONT – 362S137-43 33 KSI BRACE AT 6'-8" O.C. MAX
- 36A. BRACING FOR STOREFRONT – 362S137-43 33 KSI CONTINUOUS STUD ATTACHED WITH SCREWS TO ROOF PURLINS AND METAL STUDS OVER STOREFRONT
- 36B. BRACING FOR STOREFRONT – 362S137-43 33 KSI METAL STUDS AT 16" O.C. MAX



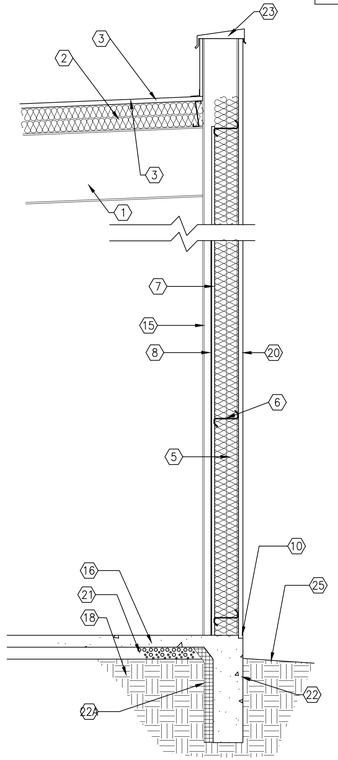
3 A3 WALL SECTION
1/2"=1'-0"



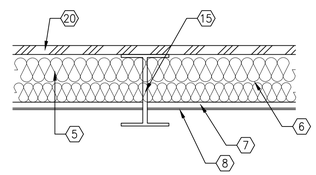
4 A3 WALL SECTION
1/2"=1'-0"



5 A3 WALL SECTION
1"=1'-0"



6 A3 WALL SECTION
1"=1'-0"



2 A3 DETAIL AT COLUMN
1"=1'-0"

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OWNER

DOLLAR GENERAL

STORE #30003
WAVERLY ROAD
HUNTINGTON, WV 25704

LOCATION

HUNTINGTON, WV

DATE

DOLLAR GENERAL PERMIT SET
1.9.24
MJM #23481

SHEET

SECTIONS/DETAILS

AS NOTED

A3

REPORT OF GEOTECHNICAL EXPLORATION

DOLLAR GENERAL - WAVERLY ROAD
HUNTINGTON, WEST VIRGINIA

TRIAD PROJECT NO. 04-23-0289

PREPARED FOR:

DG BTS HUNTINGTON, LLC
ATTN: MR. FRANCIS STANLEY
2525 BROAD STREET
CHATTANOOGA, TN 37408

PREPARED BY:

TRIAD ENGINEERING, INC.

10541 TEAYS VALLEY ROAD
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OCTOBER 31, 2023

October 31, 2023

DG BTS Huntington, LLC
Attn: Mr. Francis Stanley
2525 Broad Street
Chattanooga, TN 37408

RE: Report of Geotechnical Exploration
Dollar General - Waverly Road
Huntington, West Virginia
Triad Project No. 04-23-0289

Dear Mr. Stanley:

In accordance with your request, we have completed a geotechnical exploration for the proposed Dollar General in Huntington, Wayne County, West Virginia. Authorization to proceed with this project was provided by receipt of Professional Services Agreement dated September 1, 2023. The subsurface exploration was performed to evaluate the subsurface conditions encountered at the proposed Dollar General for the limited purposes of preparing design and construction recommendations for geotechnical aspects of the project. It is emphasized that subsurface conditions may vary dramatically between borings, and Triad makes no representations as to subsurface conditions other than those encountered at the specific boring locations.

This report has been prepared for the exclusive use of DG BTS Huntington, LLC for specific application to the design of the proposed Dollar General in Huntington, Wayne County, West Virginia. Triad's responsibilities and liabilities are limited to our Client and apply only to their use of our report for the purposes described above. To observe compliance with design concepts and specifications, and to facilitate design changes in the event that subsurface conditions differ from those anticipated prior to construction, it is recommended that Triad be retained to provide continuous engineering and testing services during the earthwork and foundation construction phases of the work.

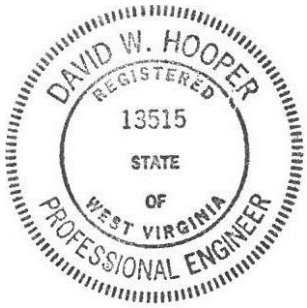
We appreciate the opportunity to assist you on this project and trust this report satisfies your needs at this time. Please feel free to contact us if you have questions concerning this report, or if we can provide further assistance.

Sincerely,

TRIAD ENGINEERING, INC.

Maria af Rolén, P.G.
Senior Geologist

David W. Hooper, P.E
Principal Engineer



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Report of Geotechnical Exploration
Dollar General – Waverly Road
Huntington, West Virginia

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APPENDICES

Appendix A – Figures
Appendix B – Field Exploration
Appendix C – Laboratory Testing
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Report of Geotechnical Exploration
Dollar General – Waverly Road
Huntington, West Virginia

SITE AND PROJECT DESCRIPTION

The project site is located northwest of the intersection of Waverly Road (US Route 60) and Burlington Road in Huntington, Wayne County, West Virginia. It is comprised of Wayne County Tax Parcel 6-1-223. The site currently includes two existing structures and associated parking that are anticipated to be demolished as part of the new development. The approximate site location is illustrated on Figure A-1 in Appendix A.

We understand that the proposed development will include construction of a single story structure encompassing a gross footprint of approximately 10,640 square feet. We have assumed that the structure will include structural steel framing, composite walls, and concrete slabs supported at grade. Structural loading information has not been provided to us. However, we have assumed that maximum structural loads will be relatively light. Associated access points, a truck loading area, and parking to accommodate 35 parking spaces are also planned as part of the proposed development.

We have assumed that the finished floor elevation (FFE) will be near current grade. Considering the existing topography across the site, it is anticipated that minimal cuts and fills will be required to achieve desired site grades.

GEOLOGY

Surficial Geology

According to the Map of Wayne County Showing General and Economic Geology, published in 1913 by the West Virginia Geological and Economic Survey (WVGES), the subject site is mapped within the boundaries of a Quaternary alluvium deposit. Alluvium is generally described as detrital deposits made by streams on riverbeds and floodplains. These deposits typically consist of unconsolidated and stratified clay, silt, sand, gravel, cobbles, and boulders.

Bedrock Geology

Based upon our review of the *Geologic Map of West Virginia*, published by the WVGES in 1968, the alluvium at the project site is underlain by the Conemaugh Group of the Pennsylvanian Subperiod. The Conemaugh Group is primarily non-marine and consists of cyclic sequences of red and gray shale, siltstone, and sandstone, with thin limestones and coals. It extends from the base of the Pittsburgh coal to the top of the Upper

Freeport coal—both of which are economically important and heavily mined coal beds. The unit includes the Elk Lick, Harlem, Bakerstown, and Mahoning coals, as well as the Ames Limestone. The contact between the Harlem coal and the Ames limestone and shale sequence forms a widespread stratigraphic marker extending across the Appalachian Basin to the Ohio River and beyond.

Coal Resources

We researched available mine maps provided by the WVGES to ascertain what minable coal beds are present below the site and to determine if past surface or underground mining operations have been conducted. In performing this evaluation, we could not identify any documented surface or underground mining at or beneath the project site.

It should be noted that the WVGES mine mapping database may be incomplete due to the limited number of years requiring permitting and mapping. As such, the lack of identified mines at the subject site does not constitute a guarantee of a mine free area.

SUBSURFACE EXPLORATION

As requested, ten (10) test borings were drilled at the project site on October 5 and 6, 2023. The boring locations were determined by Triad on site by measuring distances from existing site features. Surface elevations of the borings were estimated from Google Earth digital elevation model (DEM) data. Figure A-2 in Appendix A depicts the approximate locations of the test borings drilled for the project.

A representative of Triad was present full time during the drilling to direct the drilling crew, log all recovered soil samples, and observe groundwater and rock conditions. The recovered soil samples were transported to our laboratory for further testing. Detailed descriptions of materials encountered in the test borings are contained on the boring logs in Appendix B. Figure B-1 in Appendix B contains a description of the classification system and terminology utilized.

SUBSURFACE CONDITIONS

The materials encountered in the borings are generally described below. Stratification lines indicated on the logs represent the approximate boundaries between material types, and the actual transitions between boring locations may be gradual.

Asphalt and Subbase: Borings B-1 through B-9 encountered approximately 0.2 to 0.5 feet of asphalt at the surface, underlain by subbase material comprised of gravel and sand. These materials extended to a combined depth ranging from 0.5 to 1.5 feet. In boring B-10, approximately 0.3 feet of asphalt was encountered at the surface and was underlain by approximately 0.7 feet of concrete.

Fill: Fill was encountered beneath the subbase in borings B-1, B-2, B-3, and B-8. The fill consisted primarily of clay with varying amounts of gravel and sand. Pocket penetrometer values obtained within this fill material indicated a very stiff consistency. The fill material extended to depths ranging from 1 to 2.5 feet below the ground surface.

Alluvium: Underlying the fill, alluvial soils were encountered. The alluvial soils consisted of clay, silt, sand, and gravel. Pocket penetrometer values obtained within the finer grained alluvium indicated consistencies ranging from very soft to very stiff. Standard Penetration Test (SPT) N-values obtained within the coarser grained alluvium indicated a very loose to loose relative density. The borings, which were advanced to depths ranging from 11.5 to 21.5 feet, were all terminated in alluvial soils.

Groundwater: Groundwater levels were measured both during and after drilling operations and are documented on the boring logs in Appendix B. Each boring was dry upon completion of drilling operations, and groundwater was not encountered during drilling operations.

It is emphasized that fluctuations in true groundwater levels can occur due to variations in seasonal, climatic and environmental conditions which may not have been evident at the time of the field exploration. Consequently, groundwater levels can vary significantly from those recorded at the time measurements were taken.

LABORATORY TESTING

Laboratory tests were performed on selected soil and rock samples to aid in classification and provide a basis for estimating their engineering properties. The laboratory tests were performed in accordance with ASTM standard test methods. Detailed results are contained in Appendix C, and the results are summarized in the following table:

TEST TYPE	TEST RESULTS
Moisture Content	16.4 to 22.1%
Atterberg Limits	Liquid Limit: 19 to 23 Plasticity Index: 3 to 7
Percent Passing No. 200 Sieve	44 to 55%
USCS Classification	CL-ML and SM

DISCUSSION

Based on the results of the subsurface exploration, the project site is underlain by approximately 1 to 2.5 feet of old fill material. Alluvial soils consisting of clay, silt, sand,

and gravel of varying consistency and density extended to the termination depths in each boring. Although the alluvial soils encountered in the borings are generally acceptable for support of the proposed structure, the follow design issues on the project remain:

- The borings encountered 1 to 2.5 feet of old fill material. We are unaware of any available records relating to the placement of the fill (i.e. compaction testing). We believe that over-excavation of the existing fill is warranted to densify the bearing materials and reduce the potential for total and differential settlement.
- Concrete remnants were encountered below the asphalt in boring B-10. Any deleterious materials from the existing site structures should be removed and replaced with controlled, compacted fill. Any concrete obstructions should also be removed. When the existing building are razed, foundations should be removed in their entirety. Any below grade slabs should be broken up in place if more than 5 feet lower than grade or should be removed in their entirety.
- The side walls of excavation, particularly deeper excavations, may be impacted by loose site soils. As there are no permanent cut slopes on the project, these conditions will be temporary during construction. Recommendations regarding excavation are provided in this report.

It is recommended that all foundations be undercut to a minimum depth of 3 feet below the bottom of foundation to remove existing fill material. Where the building slab subgrade is located in cut, it should be undercut a minimum of 1 foot below slab subgrade. If after over excavating a soft subgrade or other unacceptable condition exist, additional over-excavation will likely be required. If additional over-excavation cannot be performed, Triad should be contacted to evaluate options to undercutting which are likely to include the use of geogrids or other materials to further distribute the loads.

Although other options to address the soft and loose near surface soils were considered, it is not anticipated that they will be economically feasible for support of this structure. If desired, Triad is available to discuss these options.

The finer grained soils encountered at some of the borings can be sensitive to moisture fluctuations. These materials can become unstable under construction traffic traversing the site, especially by rubber-tired equipment. Therefore, site grading should be performed during drier months, if possible.

The following sections of this report include recommendations for design and construction of the geotechnical elements of the project. Provided that these recommendations are followed, it is our opinion that the site is generally suitable for the proposed construction.

DESIGN RECOMMENDATIONS

The geotechnical engineering evaluation of the site and subsurface conditions at the property, as well as the recommendations for site preparation and foundation support, are based on our site observations, the field data obtained and our understanding of the project information as presented in this report.

Spread Foundations

Based on the results of the test borings, newly placed backfill undercut and the alluvial soils should be suitable for support of the proposed structure on a shallow spread foundation system. We recommend that conventional spread foundations bearing on natural soil be designed using a maximum allowable bearing pressure of 1,500 pounds per square foot (psf). The bearing capacity should be verified at the time of construction by our geotechnical engineer. If zones within the foundation subgrade cannot provide an allowable bearing pressure of 1,500 psf, undercuts may be necessary to achieve the desired bearing capacity.

Exterior foundations should be constructed to bear at a minimum depth of 36 inches below the final exterior grade to achieve the recommended allowable bearing pressure and provide adequate frost protection. Interior foundations in permanently heated areas can bear at a nominal depth below the floor slab. Foundations should be designed for minimum widths of 24 and 36 inches for continuous wall and individual column footings, respectively. Although these dimensions may not fully utilize the recommended bearing pressure, they should be maintained to reduce the potential for a local shear or “punching” type bearing failure.

Prior to foundation construction, we recommend the minimum undercuts described in the DISCUSSION section of this report be performed. Alternatively the building footprint may be over-excavated to a depth of 3 feet below the design foundation bearing level. The over-excavation should extend a minimum of 5 feet beyond the building perimeter on all sides. The bottom of the excavation should be heavily proof-rolled and re-compacted with appropriate compaction equipment. New controlled fill should then be placed up to design foundation levels in accordance with the recommendations outlined in the CONSTRUCTION RECOMMENDATIONS section of this report. The over-excavated fill may be re-used for controlled fill, as long as organics or other deleterious materials are completely removed and properly discarded prior to placement.

The final grading around the proposed building should be sloped away from these structures at a minimum 2% grade for a distance of at least 10 feet. Utility trenches that enter the footprint of the proposed building also should be graded at a minimum 2% grade away from the building.

The soil bearing materials are susceptible to softening if left exposed to air and/or standing water for an extended period of time. Therefore, footing concrete should be placed as soon as possible after the excavations are completed. We recommend that all

foundation excavations be examined by our geotechnical engineer prior to placing footing concrete to verify that the bearing materials are suitable for the design bearing pressure. Testing should be accomplished using a dynamic cone penetrometer (DCP), proving ring cone penetrometer or similar equipment.

Settlement Considerations

Settlements due to structural loading were estimated based on the results of the test borings, the recommended allowable bearing pressure of 1,500 psf, laboratory test results and our past experience with similar conditions. Based on this information, we estimate that foundation settlement for the proposed structure could be on the order of 1 inch. Differential settlement which could occur along continuous walls or between individual similarly loaded column foundations is estimated to be on the order of approximately ½ inch. If structural loads require foundations larger than the minimum widths for continuous wall and individual column footings recommended in this report, we should be contacted to re-evaluate our settlement estimates using the actual structural loads and proposed foundation dimensions.

Floor Slab Recommendations

Based on the information provided and the recommendations in this report, concrete slabs for the structure may be designed as slabs-on-grade. For slabs supported at grade on undercut backfill and/or new controlled fill, it is our opinion that a modulus of subgrade reaction, “k,” of 100 pci can be utilized for design. A minimum 4-inch thick layer of ASTM No. 57 coarse aggregate should be placed under all concrete slabs to serve as a capillary water barrier and a leveling surface. A six-mil thick polyethylene vapor barrier should be placed between the stone and concrete as specified by the structural designer. Joints in the floor slabs should be provided in accordance with the guidelines specified by the Portland Cement Association (PCA) or American Concrete Institute (ACI).

Seismic Site Classification

The site soils were evaluated and classified according to the 2015 International Building Code Section 1613 - Earthquake Loads - Site Ground Motion. This building code establishes the criteria for project site evaluation. Section 1613.3.2 and 2010 ASCE-7 Standard-Table 20.3-1 defines the parameters for determining the seismic site class based on N-values. The seismic site class may be determined by calculating an average N-value of subsurface materials to a depth of 100 feet. For the determination, the N-values recorded in test borings are used for overburden soil, and then, typically, materials below the depth that auger refusal or hard rock is encountered (to a depth of 100 feet) are assigned an N-value of 100. Based on the results of the test borings, the site has an average N-value of 5. Using this information along with knowledge of the site geologic setting, the seismic site class and additional seismic information is as follows:

PARAMETER DESCRIPTION	SEISMIC RESULT
Seismic Site Class	**E**
Soil Profile Name	**Soft Soil**
Site Amplification Factor at 0.2 second, F_a	**2.5**
Site Amplification Factor at 1.0 second, F_v	**3.5**
$MCER_R$ Ground Motion (0.2 second period), S_s	**0.151**
$MCER_R$ Ground Motion (1.0 second period), S_1	**0.073**

Based on results from the test borings, published regional geologic information and the probable maximum strength of earthquake, it is our opinion that liquefaction potential for the on-site soils during seismic activity is minimal. Seismic coefficients and other seismic information to be considered for structural design of the project are provided in Appendix D of this report.

Pavement Design

For flexible pavement design, a California Bearing Ratio (CBR) of 3 was assigned to existing site soils or newly placed, properly compacted structural fill. For rigid pavement design, we assumed a modulus of subgrade reaction, “k”, equal to 90 pci. These values were based on our experience with similar site soils. The soil subgrade should be crowned or properly sloped to provide drainage of base course aggregate.

A design life of 10 years was selected for the pavement section but overlay repairs may be required before the design life of 10 years due to weathering of the flexible pavement. The listed materials shall adhere to the West Virginia Department of Highways Design Specifications, Roads and Bridges, 2010 Publication. Our recommended pavement sections for the project are tabulated below.

RECOMMENDED FLEXIBLE PAVEMENT SECTIONS		
MATERIAL	**Standard Duty (in.)**	**Heavy Duty (in.)**
Wearing I	1.5	1.5
Base II	2.0	2.5
Class 8 Aggregate	6.0	8.0

RECOMMENDED RIGID (CONCRETE) PAVEMENT SECTION	
Material	**k = 100 pci**
Heavy Duty Rigid 10-yr.	5.0” Concrete (4,000 psi) #4’s @ 12” c-c, E.W. 18” x ⅝” dowels @ 12” c-c 4.0” WVDOH Class 8 Aggregate

All exterior rigid pavements, including truck aprons, sidewalks and curb/gutter, should be constructed with Type IA Portland cement. Drainage ditches and/or inlets should be designed for the pavement areas to maintain drainage and divert runoff away from the pavement subgrade. The final subgrade should be properly sloped or crowned. If any heavy-duty pavement sections are generally located adjacent to light duty pavement sections, we recommend the use of parking bollards and signage to prevent damage to the adjacent light duty pavement sections.

Although static groundwater at shallow depths is not typically a concern on this site, adequate sub-drainage of the pavement section should be incorporated into the design to maintain long-term performance. “Finger” drains, or shallow subsurface interceptor drains, should be provided beneath large areas of pavement to capture and remove water which may accumulate in the pavement base course. These drains should be routed to discharge into appropriate stormwater basins or drop inlets. The spacing of the drains can be determined after grading plans have been developed further.

CONSTRUCTION RECOMMENDATIONS

Site Preparation

Initial preparation of the site for construction should include installation of sediment and erosion control measures and any upslope diversion ditching or berms that are required. Existing utilities that conflict with proposed foundations and/or new utility alignments should be relocated as necessary. The existing asphalt should be either completely removed or sufficiently perforated such that water is not allowed to pond on the asphalt surface.

Site Excavations

It is anticipated that most of the on-site soils can be effectively removed with conventional earth-moving equipment such as backhoes and dozers. It is assumed that excavations required for the project will likely not extend to depths sufficient to encounter weather or hard bedrock. The means necessary to perform the excavation are a function of the consistency/hardness of the material, the type/size of excavation equipment utilized and the effort the contractor is willing to apply. For bidding purposes, potential contractors should be instructed to perform their own investigations as to measures necessary to excavate materials encountered.

Removal of the existing foundation and building remnants needs to be considered carefully by the contractor. Some specialty equipment may be necessary to complete that task.

Excavated materials should not be stockpiled and construction equipment should not be positioned beside open excavations, since the added load may cause a sudden

collapse of the excavation side walls. The design and construction of all excavations should comply with applicable local, state, and federal safety regulations, including the current requirements of the Occupational Safety and Health Administration (OSHA). In no case should slope height, slope inclination, or excavation depth exceed those specified by OSHA or any other regulatory agencies or local authorities having jurisdiction at the construction site.

Controlled Fill

Suitable Fill Material

Fill required to attain design grades should be placed as controlled, compacted fill. Satisfactory fill includes approved on-site excavated materials, off-site borrow material, residual soils, soil/rock mixtures, and soft weathered rock, or a well-graded commercial stone such as crusher run aggregate. The fill should be free of trash, wood, coal, topsoil, organics, pyritic material with greater than 0.1 percent by weight of pyritic sulfur, frozen material, and pieces of rock greater than 4 inches in any dimension for lift thicknesses of 9 inches or 1½ inches in any dimension for lift thicknesses of 4 inches. Materials classified as MH, CH, OH, OL and Pt based on the Unified Soil Classification System (USCS) are not considered suitable for use as new fill. All fill should be tested and approved prior to placement and compaction.

Fill Placement and Compaction

Before initiating fill placement, the asphalt surface and other surficial material should be removed. The subgrade surface should be proof-rolled with appropriate rubber-tired construction equipment and/or visually evaluated to locate any soft spots or areas of excessive "pumping." Any such areas should be over-excavated to a firm subgrade and replaced with new, controlled fill material. The engineer should be contacted if excessive over-excavation is required.

During placement, moisten or aerate each layer of fill, as necessary, to obtain the required compaction. Fill should not be placed on surfaces that are muddy, frozen or have not been approved by prior testing and/or proof-rolling. Free water should be prevented from appearing on the surface during or after compaction operations. Fill placed on sloping areas should be properly benched or “notched” into the slope face such that a smooth transition between the new fill and existing slope face is not present.

Soil material which is removed because it is too wet to permit proper compaction may be spread and allowed to dry. Drying can be facilitated by discing, harrowing, or by pulverizing until the moisture content is reduced to an acceptable level. When the soil is too dry, water may be uniformly applied to the subgrade surface or to the layer to be compacted.

Fill material compacted by heavy compaction equipment should be placed in loose layers not exceeding 9 inches in thickness. Fill compacted with lightweight equipment, such as hand-operated tampers or walk-behind rollers, should be placed in loose layers

not exceeding 4 inches in thickness. The compaction equipment utilized should be suitable for the type of material being compacted. Vibratory rollers are best suited to coarse-grained soils, while pad foot (often called sheepsfoot) rollers are appropriate for fine-grained materials. Fill placed adjacent to foundation walls should be compacted using lightweight equipment.

New fill placed within the structure footprint and extending at least five (5) feet beyond its perimeter, or to that extent possible, should be compacted to at least 98 percent of the laboratory maximum dry density as determined by the Standard Proctor method (ASTM D 698). Fill placed outside of these areas should be compacted to at least 95 percent of the maximum dry density as determined by the same standard. The placement moisture content of fill material should be within ±3 percentage points of the optimum moisture content as determined by ASTM D 698, except the structural areas where the moisture content should be within ±2 percent of the optimum moisture content. Granular materials, such as clean sand or aggregate, should be compacted to at least 85% of its relative density, as determined by ASTM D 4253 and D 4254 test methods.

Subgrade of the floor slab area should be compacted and tested to at least 98 percent of the laboratory maximum dry density as determined by the Standard Proctor method (ASTM D 698), prior to placement of the four-inch layer of crushed stone such as ASTM No. 57 coarse aggregate. The moisture content of the subgrade should be within ±2 percent of the optimum moisture content.

Foundation Construction

Foundation excavations should be cleaned of all loose or otherwise disturbed materials present in the base of the excavations. The excavations should be observed and tested by a qualified geotechnical engineer, or his/her representative, prior to concrete placement to verify that materials capable of providing the recommended bearing capacity are present. Materials exposed in the foundation excavations will be susceptible to softening and/or degradation if exposed to precipitation or surface water runoff. In addition, some foundation excavations could be relatively deep. Consequently, foundation concrete should be placed in the excavations as soon as possible once the excavations have been observed and approved, and only that amount of foundation excavation which can be backfilled with concrete should be opened up on any given day. Once foundation walls have been constructed up to final exterior grades, we recommend that the foundation excavations be backfilled with compacted soil fill to prevent ponding of water adjacent to foundations.

Pavement Construction

All subgrade areas approved during rough grading should be re-evaluated prior to placement of the base stone. Any wet and/or unstable soils present at the subgrade level during fine grading operations should be either scarified, aerated and re-compacted or should be removed and replaced with suitable fill material. Any unsuitable

subgrade soils should be corrected immediately prior to placement of base stone and pavement material.

It will be very important that the final soil subgrade be properly sloped or crowned to promote drainage of surface water from precipitation. Also, it will be very important that adequate ditches be constructed along cut sections to effectively remove surface runoff. It is very important that both the base stone and pavement section be placed immediately after acceptable subgrade conditions have been achieved due to the potential for subgrade softening from adverse weather conditions. In addition, heavy construction traffic should be limited from traveling across approved final subgrade areas that have been exposed to precipitation to help maintain a stable subgrade prior to pavement construction. If base stone and pavement sections cannot be placed immediately after acceptable subgrade conditions have been achieved, we recommend stabilizing the soil subgrade with either lime or Portland cement to reduce the potential for subgrade softening from adverse weather conditions. All base stone and asphaltic concrete placement and testing should be performed in accordance with WVDOH criteria.

Groundwater and Surface Runoff Control

The contractor should be prepared to implement temporary and/or permanent dewatering measures since groundwater conditions can change. We anticipate that sources of subsurface water which may develop during construction can probably be managed and removed by a gravity drainage system, sump pits and pumps or other minor dewatering procedures.

Surface water runoff should be prevented from flowing through the construction area. If necessary, diversion ditches or berms should be installed upslope of the construction area. Ditches should be protected from excessive erosion through the use of riprap, erosion control matting, or vegetation.

Quality Assurance and Control

We recommend that the geotechnical engineer-of-record, Triad, be retained to monitor the construction activities to verify that the field conditions are consistent with the findings of our exploration. If significant variations are encountered, or if the design is altered, we should be notified.

The geotechnical engineer should provide personnel full-time and/or intermittently to:

- Observe and document installation of the drainage features and verify initial subgrade conditions prior to fill placement.
- Observe undercut subgrade conditions to determine if additional overexcavation is necessary.
- Observe and test material compaction during fill construction. Field density tests should be performed in accordance with ASTM D 6938 (nuclear method). At

least three (3) field density tests should be performed for each lift or at a frequency determined by the geotechnical engineer to be sufficient for the size of the fill area to verify the required soil compaction.
- Examine all subgrade bearing levels to confirm compliance with our recommendations and verify that adequate support is available.
- Test fresh structural concrete placed for the project.

LIMITATIONS

This report has been prepared for the exclusive use of DG BTS Huntington, LLC for specific application to the design of the proposed Dollar General in Huntington, Wayne County, West Virginia. The work has been performed in accordance with generally accepted geotechnical engineering practices. No other warranty, expressed or implied, is made.

This report should not be used for estimation of construction quantities and/or costs, and contractors should conduct their own investigation of site conditions for these purposes. Please note that Triad is not responsible for any claims, damages or liability associated with any other party's interpretation of the data or reuse of these data or engineering analyses without the express written authorization of Triad. Additionally, this report must be read in its entirety. Individual sections of this report may cause the reader to draw incorrect conclusions if considered in isolation from each other.

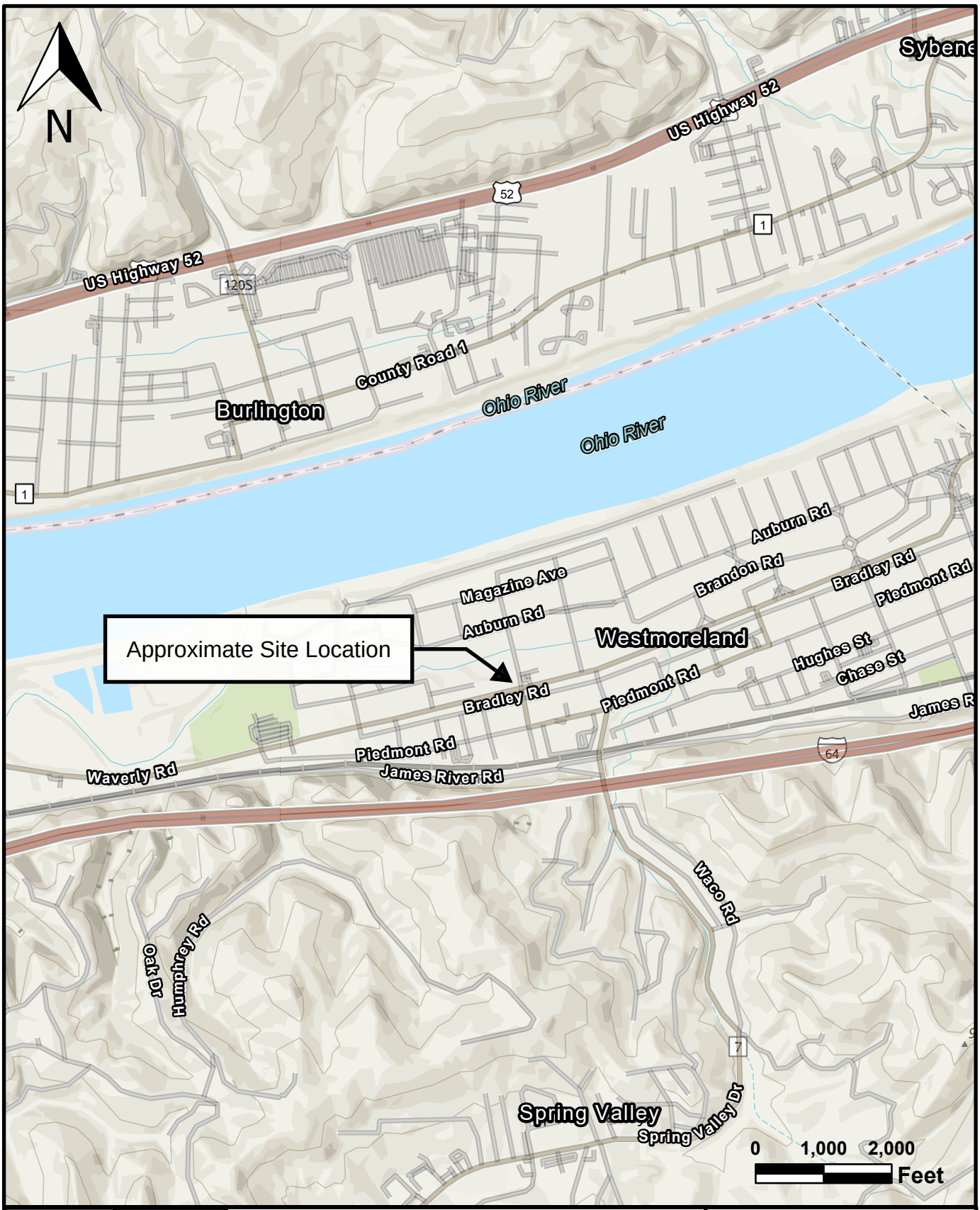
The conclusions and recommendations contained in this report are based, in part, upon our field observations and data obtained from the borings at the site. The nature and extent of variations may not become evident until construction. If variations then appear evident, it may be necessary to re-evaluate the recommendations presented herein. Similarly, in the event that any changes in the nature, design, or location of the facilities are planned, the conclusions and recommendations contained herein shall not be considered valid unless the changes are reviewed and the conclusions are modified or verified in writing by Triad.

It is recommended that we be provided the opportunity to review the final grading plan, overall foundation design, and specifications so that earthwork and foundation recommendations may be properly interpreted and implemented. If we are not afforded the privilege of making this review, we will not assume responsibility for misinterpretation of our recommendations, as our recommendations are strictly limited to conditions represented to Triad at the time this report was issued.

TRIAD ENGINEERING, INC.

APPENDIX A

Figures



PREPARED BY:	CHECKED BY:
MAR	DWH
PROJECT NUMBER: 01-23-0289	
FIGURE A-1	

GENERAL SITE VICINITY

Dollar General - Waverly Road
Huntington, West Virginia
World Topographic Map (ESRI)

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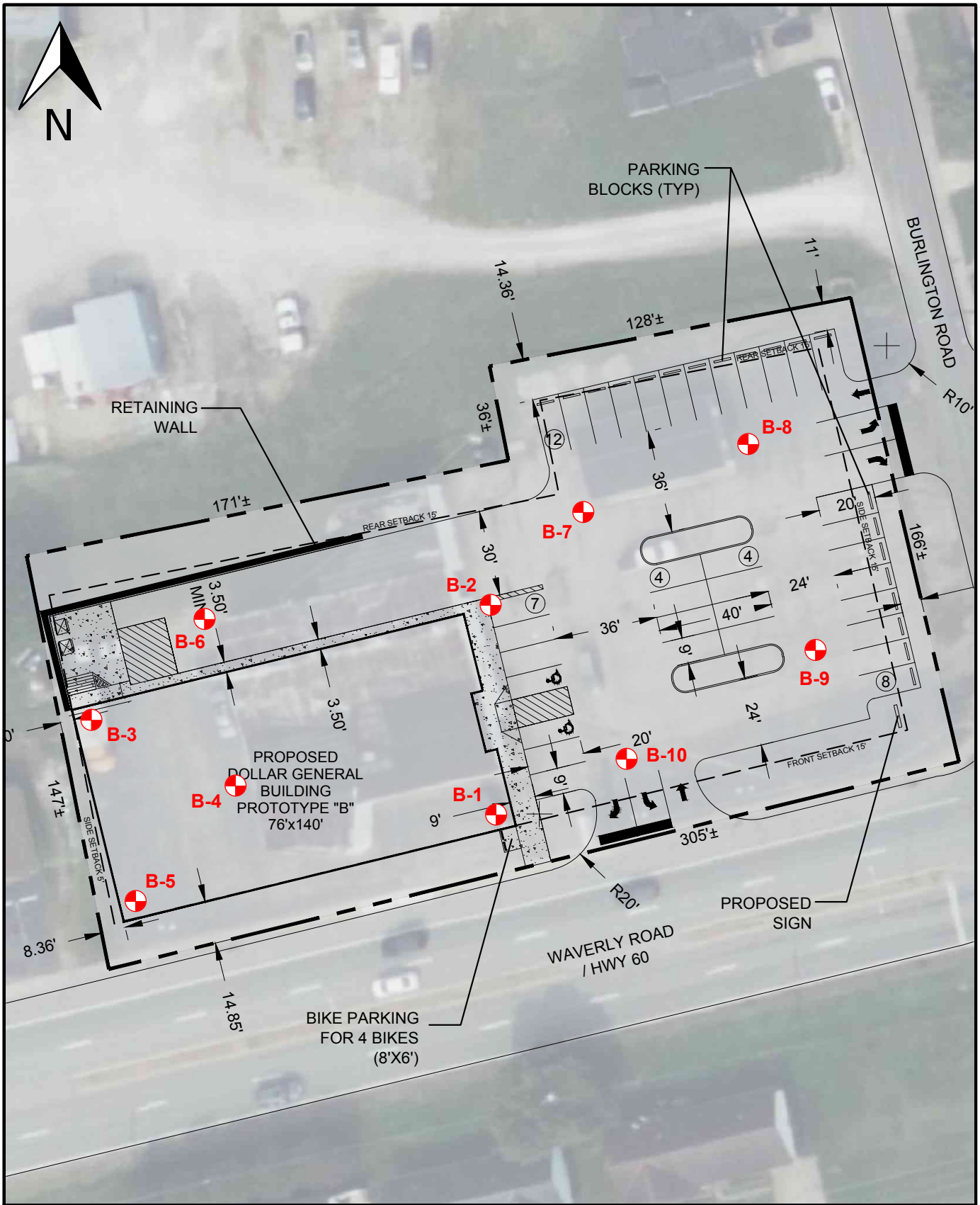

PREPARED BY:	CHECKED BY:
MAR	DWH
PROJECT NUMBER: 04-23-0289	
FIGURE A-2	

BORING LOCATION PLAN
Dollar General - Waverly Road
Huntington, West Virginia
Proposed Site Plan

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APPENDIX B

Field Exploration

Triad Engineering, Inc.

Field Exploration

A representative of Triad was present to direct the drill crew, log recovered samples and observe groundwater conditions. The borings were drilled utilizing a CME-55 rotary auger drill rig. Samples of in-situ soil and weathered bedrock were obtained using a split-barrel sampler while performing Standard Penetration Tests (ASTM D 1586). The results of these tests (N-values) are commonly interpreted to provide an index to strength, consistency or relative density of the sampled materials and their ability to support foundations.

Groundwater levels were checked both during and after drilling operations and are recorded on the individual logs if applicable. It is emphasized that groundwater levels typically vary and are dependent upon climatic conditions and other environmental factors.

It is also emphasized that the lines shown on the logs are estimates of the changes in material. Actual changes may be gradual and may vary from those indicated on the logs, and the subsurface conditions between the borings may differ from those depicted on the logs. The boreholes were backfilled upon completion of the drilling with auger cuttings. Samples were transported to our office for temporary storage and additional analysis. The samples will be discarded after a period of 60 days unless other arrangements are made.

KEY TO IDENTIFICATION OF SOIL AND WEATHERED BEDROCK SAMPLES

Descriptor Sequence	
1	Color
2	Primary Component
3	Fractions
4	Moisture
5	Descriptors
6	Plasticity
7	Consistency/ Relative Density
8	Deposition Type

1. Color	
Gray	Tan
Brown	Black
Orange	Red
Green	Yellow
Purple	Blue
Modifiers	
Light	Lighter side of color range
Dark	Darker side of color range
Mottled	Irregularly marked with spots of different colors
Banded	Alternating shades or colors

2. Primary Component	
Component	Grain Size
Boulders	≥ 12 inches
Cobbles	3 to 12 inches
Coarse Gravel	1 to 3 inches
Medium Gravel	$^{3}/_{8}$ to 1 inch
Fine Gravel	$^{5}/_{64}$ to $^{3}/_{8}$ inch
Coarse Sand	#40 to #10
Fine Sand	#200 to #40
Silt/Clay	≤ #200

3. Fractions	
And	≥ 35%
Some	20 to 35%
Little	10 to 20%
Trace	< 10%

4. Moisture	
Dry	Dry to touch
Damp	Slightly moist
Moist	No visible free water
Wet	Visible free water

5. Descriptors	
Fissile	Splits easily along closely spaced parallel planes (breaks into plates)
Hackly	Jagged or irregular fracture planes
Slickenside	Polished and striated surface that results from friction along a fault plane
Laminated	Alternating thin layers of varying material or colors less than ¼" thick
Lensed	Inclusion of small pockets of different soils
Saprolitic	Completely weathered rock that retains the appearance of the original rock structure but has only a trace of the original bond strength
Micaceous	Containing mica minerals
Varved	Laminated sediment consisting of alternating layers of fine sand and silt or clay deposited in still water

6. Plasticity of Fine-Grained Soils

Fine-Grained Component	Plasticity	Estimated Plasticity Index (PI)	Smallest Thread Diameter	Thread Characteristics	Dilatancy
Predominately Silt: Silt ↑ More Silt ↑	Non-Plastic	0 - 2%	Ball cracks	Dries rapidly; a 1/8-inch thread cannot be rolled at any water content	Moist ball sheds water when shaken giving a glossy appearance
	Low Plasticity	3 - 10%	$^{1}/_{8}$ to $^{1}/_{4}$ inch	Feels powdery when drying out during rolling; thread can barely be rolled	Moist ball retains water or sheds water slowly when shaken
Predominately Clay: More Clay ↓ Clay ↓	Medium Plasticity	> 10 - 20%	$^{1}/_{16}$ inch	Thread cannot be rerolled after reaching plastic limit	
	Highly Plastic	> 20%	$^{1}/_{32}$ inch	Thread can be rerolled after reaching plastic limit	Moist ball retains water when shaken

7a. Relative Density of Granular Coarse-Grained Soils	
Descriptor	N-Value
Very Loose	≤ 4
Loose	5 - 10
Medium Dense	11 - 30
Dense	31 - 50
Very Dense	> 50

7b. Consistency of Fine-Grained Soils		
Descriptor	Pocket Penetrometer (tons/ft^2)	N-Value
Very Soft	≤ 0.25	≤ 2
Soft	≥ 0.25 - 0.5	3 - 4
Medium Stiff	> 0.5 - 1.0	5 - 8
Stiff	> 1.0 - 2.0	9 - 15
Very Stiff	> 2.0 - 4.0	16 - 30
Hard	> 4	≥ 31

8. Type of Deposit	
Alluvium	Sediment deposited by moving water
Colluvium	Sediment deposited by gravity
Fill	Manmade deposit
Fluviomarine	Stratified materials formed by the combined action of river and sea processes
Glacial Outwash	Sediment deposited by glacial meltwater; commonly sand and gravel
Glacial Till	Unsorted sediment deposited by glacier
Glacial Lake Deposit	Sediment deposited in glacial lake; commonly silt and clay
Residuum	Insoluble material remaining from weathered rock
Weathered Bedrock	Bedrock that has been weathered

FIGURE B-1

TEST BORING LOG

Project Number: **04-23-0289** Project Name: **Dollar General - Waverly Road** Boring No.: **B-1**

Logger: RKH Boring Location: See Boring Location Plan

Date Started: 10/5/23 Drill/Method: CME-55

Date Completed: 10/5/23 Driller: HL (TRIAD) Ground Elev.: 550

Shelby Tube | Standard Split Spoon | Core Sample | Auger Probe

Depth (feet)	Sample No.	Sample Type	Blow Counts	Recovery (%)	RQD (RUN)	Strata Depth (ft)	MATERIAL DESCRIPTION	RQD (Strata)	Water Level	Graphic Log	Strata Elevation
						0.3	**ASPHALT**				549.7
						0.5	Gray **GRAVEL**, some sand, dry, **subbase**				549.5
	S-1		2-3-4 PP: 3.5	100%		2.5	Brown **CLAY**, little sand, trace gravel, damp, medium plasticity, very stiff, **fill**				547.5
	S-2		2-3-3 PP: 1.5	100%			Brown **CLAY** and **SAND**, moist, medium plasticity, stiff, **alluvium**				
5.0	S-3		1-3-2 PP: 1.5	100%		7.5					542.5
	S-4		1-2-3	100%			Brown **SAND** and **SILTY CLAY**, moist, loose, **alluvium**				
10.0	S-5		1-3-2	100%			- From 10.0 to 16.5 feet: W=16.4%, LL=19, PL=16, PI=3, Gravel=0%, Sand=56%, Fines=44%, SM				
15.0	S-6		1-2-3	100%							
20.0	S-7		3-4-3	100%		21.5					528.5
25.0							Boring Terminated at 21.5 feet.				

TRIAD_C 04-23-0289 BORING LOGS.GPJ TRIAD 3.GDT 10/22/23

TRIAD ENGINEERING, INC.

10541 Teays Valley Road
Scott Depot, WV 25560
Phone: 304.755.0721
Fax: 304.755.1880

Remarks: Boring dry upon completion. Elevation estimated from Google Earth DEM data.

TEST BORING LOG

Project Number: **04-23-0289** Project Name: **Dollar General - Waverly Road** Boring No.: **B-2**

Logger: RKH Boring Location: See Boring Location Plan

Date Started: 10/6/23 Drill/Method: CME-55

Date Completed: 10/6/23 Driller: HL (TRIAD) Ground Elev.: 549

Shelby Tube | Standard Split Spoon | Core Sample | Auger Probe

Depth (feet)	Sample No.	Sample Type	Blow Counts	Recovery (%)	RQD (RUN)	Strata Depth (ft)	MATERIAL DESCRIPTION	RQD (Strata)	Water Level	Graphic Log	Strata Elevation
						0.4	**ASPHALT**				548.6
						0.8	Gray **GRAVEL**, some sand, dry, **subbase**				548.2
	S-1		1-2-5 PP: 2.0	100%		1.8	Gray and brown **CLAY**, some gravel, little sand, damp, medium plasticity, stiff, **fill**				547.2
	S-2		3-4-5 PP: 2.5	100%			Brown **CLAY**, some sand, moist, medium plasticity, stiff to very stiff, **alluvium** - From 2.5 to 4.0 feet: W=16.5%				
5.0						5.0					544.0
	S-3		3-4-4 PP: 2.5	100%			Brown **CLAY** and **SAND**, moist, medium plasticity, very stiff, **alluvium**				
						7.5					541.5
	S-4		2-3-4	100%			Brown **SAND**, little to some clay, moist, loose, **alluvium**				
10.0	S-5		2-2-3	100%							
15.0	S-6		1-2-3	100%							
20.0	S-7		1-3-3	100%							
						21.5					527.5
25.0							Boring Terminated at 21.5 feet.				

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Remarks: Boring dry upon completion. Elevation estimated from Google Earth DEM data.

TEST BORING LOG

Sheet 1 of 1

Project Number: **04-23-0289** Project Name: **Dollar General - Waverly Road** Boring No.: **B-3**

Logger: RKH Boring Location: See Boring Location Plan

Date Started: 10/6/23 Drill/Method: CME-55

Date Completed: 10/6/23 Driller: HL (TRIAD) Ground Elev.: 548

Shelby Tube | Standard Split Spoon | Core Sample | Auger Probe

Depth (feet)	Sample No.	Sample Type	Blow Counts	Recovery (%)	RQD (RUN)	Strata Depth (ft)	MATERIAL DESCRIPTION	RQD (Strata)	Water Level	Graphic Log	Strata Elevation
						0.2	**ASPHALT**				547.8
						0.6					547.4
	S-1		3-4-3 PP: 3.5	100%		1.0	Gray **GRAVEL**, some sand, dry, **subbase**				547.0
							Brown and gray **CLAY**, some sand, little gravel, damp, medium plasticity, very stiff, **fill**				
	S-2		2-2-3 PP: 2.5	100%			Brown **CLAY**, some sand, damp, medium plasticity, very stiff, **alluvium**				
5.0						5.0					543.0
	S-3		1-2-2	100%			Brown **SAND**, some clay, moist, very loose, **alluvium**				
	S-4		2-1-2	100%			- From 7.5 to 9.0 feet: W=20.0%				
10.0						10.0					538.0
	S-5		1-2-2	100%			Brown **SAND**, trace to little clay, moist, very loose to loose, **alluvium**				
15.0											
	S-6		1-2-3	100%							
20.0											
	S-7		2-2-2	100%		21.5					526.5
							Boring Terminated at 21.5 feet.				
25.0											

TRIAD_C 04-23-0289 BORING LOGS.GPJ TRIAD 3.GDT 10/22/23

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Fax: 304.755.1880

Remarks: Boring dry upon completion. Elevation estimated from Google Earth DEM data.

TEST BORING LOG

Sheet 1 of 1

Project Number: **04-23-0289** Project Name: **Dollar General - Waverly Road** Boring No.: **B-4**

Logger: RKH Boring Location: See Boring Location Plan

Date Started: 10/6/23 Drill/Method: CME-55

Date Completed: 10/6/23 Driller: HL (TRIAD) Ground Elev.: 549

Shelby Tube | Standard Split Spoon | Core Sample | Auger Probe

Depth (feet)	Sample No.	Sample Type	Blow Counts	Recovery (%)	RQD (RUN)	Strata Depth (ft)	MATERIAL DESCRIPTION	RQD (Strata)	Water Level	Graphic Log	Strata Elevation
						0.3	**ASPHALT**				548.7
						0.5	Gray **GRAVEL**, some sand, dry, **subbase**				548.5
	S-1		2-2-4 PP: 3.0	100%			Brown **CLAY**, some sand, moist, medium plasticity, stiff to very stiff, **alluvium**				
	S-2		2-3-3 PP: 3.5	100%			- From 2.5 to 4.0 feet: W=18.6%				
5.0	S-3		1-2-4 PP: 1.5	100%							
						7.5					541.5
	S-4		1-1-2	100%			Brown **SAND**, little to some clay, moist, very loose to loose, **alluvium**				
10.0	S-5		1-1-3	100%							
15.0	S-6		1-2-3	100%							
20.0						20.0					529.0
	S-7		1-2-1	100%			Brown **SAND**, trace clay, moist, very loose, **alluvium**				
						21.5					527.5
25.0							Boring Terminated at 21.5 feet.				

TRIAD_C 04-23-0289 BORING LOGS.GPJ TRIAD 3.GDT 10/22/23

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Phone: 304.755.0721
Fax: 304.755.1880

Remarks: Boring dry upon completion. Elevation estimated from Google Earth DEM data.

TEST BORING LOG

Project Number: **04-23-0289** Project Name: **Dollar General - Waverly Road** Boring No.: **B-5**

Logger: RKH Boring Location: See Boring Location Plan

Date Started: 10/6/23 Drill/Method: CME-55

Date Completed: 10/6/23 Driller: HL (TRIAD) Ground Elev.: 550

Shelby Tube | Standard Split Spoon | Core Sample | Auger Probe

Depth (feet)	Sample No.	Sample Type	Blow Counts	Recovery (%)	RQD (RUN)	Strata Depth (ft)	MATERIAL DESCRIPTION	RQD (Strata)	Water Level	Graphic Log	Strata Elevation
						0.4	**ASPHALT**				549.6
						0.7	Gray **GRAVEL**, some sand, damp, **alluvium**				549.3
	S-1	X	3-3-3 PP: 3.0	100%			Brown **CLAY**, some sand, damp, medium plasticity, very stiff, **alluvium**				
	S-2	X	4-4-4 PP: 3.0	100%							
5.0	S-3	X	1-2-3 PP: 2.0	100%		7.5					542.5
	S-4	X	1-1-3 PP: 2.0	100%			Brown **SAND**, some clay, moist, very loose to loose, **alluvium**				
10.0	S-5	X	1-1-2	100%			- From 10.0 to 11.5 feet: W=21.1%				
15.0	S-6	X	4-4-4	100%		15.0	Brown **SAND**, moist, loose, **alluvium**				535.0
20.0	S-7	X	2-2-2	100%		21.5					528.5
25.0							Boring Terminated at 21.5 feet.				

TRIAD_C 04-23-0289 BORING LOGS.GPJ TRIAD 3.GDT 10/22/23

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Phone: 304.755.0721
Fax: 304.755.1880

Remarks: Boring dry upon completion. Elevation estimated from Google Earth DEM data.

TEST BORING LOG

Project Number: **04-23-0289** Project Name: **Dollar General - Waverly Road** Boring No.: **B-6**

Logger: RKH Boring Location: See Boring Location Plan

Date Started: 10/6/23 Drill/Method: CME-55

Date Completed: 10/6/23 Driller: HL (TRIAD) Ground Elev.: 548

Shelby Tube | Standard Split Spoon | Core Sample | Auger Probe

Depth (feet)	Sample No.	Sample Type	Blow Counts	Recovery (%)	RQD (RUN)	Strata Depth (ft)	MATERIAL DESCRIPTION	RQD (Strata)	Water Level	Graphic Log	Strata Elevation
						0.3	**ASPHALT**				547.7
	S-1	X	10-1-1	53%		1.0	Gray **SAND** and **GRAVEL**, little clay, damp, **subbase**				547.0
							Brown **SILTY CLAY** and **SAND**, moist, low plasticity, very soft, **alluvium**				
	S-2	X	1-1-1	0%							
5.0											
	S-3	X	0-0-0 PP: 0.5	1.5%			- From 5.0 to 6.5 feet: W=21.7%, LL=23, PL=16, PI=7, Gravel=0%, Sand=45%, Fines=55%, CL-ML				
						7.5					540.5
	S-4	X	1-1-2	1.5%			Brown **SAND**, little to some clay, moist, very loose, **alluvium**				
10.0											
	S-5	X	1-2-3	1.5%		11.5					536.5
							Boring Terminated at 11.5 feet.				
15.0											
20.0											
25.0											

TRIAD_C 04-23-0289 BORING LOGS.GPJ TRIAD 3.GDT 10/22/23

TRIAD ENGINEERING, INC.

10541 Teays Valley Road
Scott Depot, WV 25560
Phone: 304.755.0721
Fax: 304.755.1880

Remarks: Boring dry upon completion. Elevation estimated from Google Earth DEM data.

TEST BORING LOG

Sheet 1 of 1

Project Number: **04-23-0289** Project Name: **Dollar General - Waverly Road** Boring No.: **B-7**

Logger: RKH Boring Location: See Boring Location Plan

Date Started: 10/6/23 Drill/Method: CME-55

Date Completed: 10/6/23 Driller: HL (TRIAD) Ground Elev.: 547

Shelby Tube | Standard Split Spoon | Core Sample | Auger Probe

Depth (feet)	Sample No.	Sample Type	Blow Counts	Recovery (%)	RQD (RUN)	Strata Depth (ft)	MATERIAL DESCRIPTION	RQD (Strata)	Water Level	Graphic Log	Strata Elevation
						0.5	**ASPHALT**				546.5
	S-1		1-2-3 PP: 2.0	100%		1.5	Gray **CLAY**, some gravel, little sand, damp, medium plasticity, stiff, **subbase**				545.5
	S-2		4-4-6 PP: 2.5-4.0	100%			Brown **CLAY**, some sand, moist, medium plasticity, very stiff, **alluvium**				
5.0	S-3		2-3-4 PP: 1.5	100%			- From 5.0 to 6.5 feet: W=18.9%				
						7.5					539.5
	S-4		1-2-3 PP: 1.5	100%			Brown **CLAY** and **SAND**, moist, medium plasticity, stiff, **alluvium**				
10.0	S-5		2-2-3 PP: 1.5	100%		11.5					535.5
							Boring Terminated at 11.5 feet.				
15.0											
20.0											
25.0											

TRIAD_C 04-23-0289 BORING LOGS.GPJ TRIAD 3.GDT 10/22/23

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Scott Depot, WV 25560
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Remarks: Boring dry upon completion. Elevation estimated from Google Earth DEM data.

TEST BORING LOG

Sheet 1 of 1

Project Number: **04-23-0289** Project Name: **Dollar General - Waverly Road** Boring No.: **B-8**

Logger: RKH Boring Location: See Boring Location Plan

Date Started: 10/5/23 Drill/Method: CME-55

Date Completed: 10/5/23 Driller: HL (TRIAD) Ground Elev.: 545

Shelby Tube | Standard Split Spoon | Core Sample | Auger Probe

Depth (feet)	Sample No.	Sample Type	Blow Counts	Recovery (%)	RQD (RUN)	Strata Depth (ft)	MATERIAL DESCRIPTION	RQD (Strata)	Water Level	Graphic Log	Strata Elevation
						0.5	**ASPHALT**				544.5
						0.9	Gray **GRAVEL**, some sand, dry, **subbase**				544.1
	S-1		2-3-3 PP: 3.0	100%		2.0	Brown **CLAY**, some sand, trace gravel, damp, medium plasticity, very stiff, **fill**				543.0
	S-2		2-2-4 PP: 2.5	100%			Brown **CLAY**, some sand, moist, medium plasticity, very stiff, **alluvium**				
5.0	S-3		2-2-3 PP: 2.5	100%							
	S-4		1-3-3 PP: 2.5	100%			- From 7.5 to 9.0 feet: W=21.3%				
10.0	S-5		1-1-2 PP: 2.0	100%		10.0					535.0
						11.5	Brown **CLAY** and **SAND**, moist, medium plasticity, stiff to very stiff, **alluvium**				533.5
							Boring Terminated at 11.5 feet.				
15.0											
20.0											
25.0											

TRIAD_C 04-23-0289 BORING LOGS.GPJ TRIAD 3.GDT 10/22/23

TRIAD ENGINEERING, INC.

10541 Teays Valley Road
Scott Depot, WV 25560
Phone: 304.755.0721
Fax: 304.755.1880

Remarks: Boring dry upon completion. Elevation estimated from Google Earth DEM data.

TEST BORING LOG

Project Number: **04-23-0289** Project Name: **Dollar General - Waverly Road** Boring No.: **B-9**

Logger: RKH Boring Location: See Boring Location Plan

Date Started: 10/5/23 Drill/Method: CME-55

Date Completed: 10/5/23 Driller: HL (TRIAD) Ground Elev.: 547

Shelby Tube | Standard Split Spoon | Core Sample | Auger Probe

Depth (feet)	Sample No.	Sample Type	Blow Counts	Recovery (%)	RQD (RUN)	Strata Depth (ft)	MATERIAL DESCRIPTION	RQD (Strata)	Water Level	Graphic Log	Strata Elevation
						0.5	**ASPHALT**				546.5
						0.9	Gray **GRAVEL**, some sand, dry, **subbase**				546.1
	S-1		2-3-4 PP: 3.0	100%		2.0	Brown **CLAY**, some sand, trace gravel, damp, medium plasticity, very stiff, **fill**				545.0
	S-2		2-2-3 PP: 1.5	100%			Brown **CLAY**, little to some sand, moist, medium plasticity, stiff to very stiff, **alluvium**				
5.0	S-3		1-3-4 PP: 2.5	100%		7.5					539.5
	S-4		1-1-3 PP: 1.5	100%			Brown **CLAY** and **SAND**, moist, medium plasticity, stiff, **alluvium**				
10.0	S-5		1-1-3 PP: 1.5	100%			- From 10.0 to 11.5 feet: W=22.1%				
						11.5					535.5
							Boring Terminated at 11.5 feet.				
15.0											
20.0											
25.0											

TRIAD_C 04-23-0289 BORING LOGS.GPJ TRIAD 3.GDT 10/22/23

TRIAD ENGINEERING, INC.

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Fax: 304.755.1880

Remarks: Boring dry upon completion. Elevation estimated from Google Earth DEM data.

TEST BORING LOG

Sheet 1 of 1

Project Number: **04-23-0289** Project Name: **Dollar General - Waverly Road** Boring No.: **B-10**

Logger: RKH Boring Location: See Boring Location Plan

Date Started: 10/6/23 Drill/Method: CME-55

Date Completed: 10/6/23 Driller: HL (TRIAD) Ground Elev.: 549

Shelby Tube | Standard Split Spoon | Core Sample | Auger Probe

Depth (feet)	Sample No.	Sample Type	Blow Counts	Recovery (%)	RQD (RUN)	Strata Depth (ft)	MATERIAL DESCRIPTION	RQD (Strata)	Water Level	Graphic Log	Strata Elevation
						0.3	**ASHPALT**				548.7
						1.0	**CONCRETE**				548.0
	S-1		2-1-3 PP: 2.0	100%			Brown **CLAY**, some sand, moist, medium plasticity, very stiff, **alluvium**				
	S-2		3-6-6 PP: 2.5	100%			- From 2.5 to 4.0 feet: W=18.2%				
5.0	S-3		3-4-5 PP: 2.5	100%							
						7.5					541.5
	S-4		0-2-5 PP: 0.5	100%			Brown **CLAY** and **SAND**, moist, medium plasticity, soft, **alluvium**				
10.0	S-5		0-1-1 PP: 0.5	100%							
						11.5					537.5
							Boring Terminated at 11.5 feet.				
15.0											
20.0											
25.0											

TRIAD_C 04-23-0289 BORING LOGS.GPJ TRIAD 3.GDT 10/22/23

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Phone: 304.755.0721
Fax: 304.755.1880

Remarks: Boring dry upon completion. Elevation estimated from Google Earth DEM data.

TRIAD ENGINEERING, INC.

APPENDIX C

Laboratory Testing

Triad Engineering, Inc.

Laboratory Testing

The samples obtained from the test borings were visually classified in the field by geotechnical engineering personnel from Triad. The recovered soils were further evaluated by laboratory testing. Laboratory soils tests were conducted in accordance with applicable ASTM Standards as listed below:

1. Moisture content tests were performed in accordance with ASTM D 2216.

2. Atterberg Limits tests, consisting of the liquid limit, plastic limit, and plasticity index, were performed in accordance with ASTM D 4318.

3. Sieve analyses with washed No. 200 sieve tests were performed in accordance with ASTM D 1140.

A summary and details of the laboratory test results are included on the following pages of this appendix.

TRIAD ENGINEERING, INC.

LABORATORY DATA SUMMARY

BORING NO.	SAMPLE DEPTH (ft)	SAMPLE TYPE	NATURAL MOISTURE (%)	ATTERBERG LIMITS			GRADATION			USCS SOIL CLASS.	ADDITIONAL TESTS CONDUCTED
				LL	PL	PI	% GRAVEL	% SAND	% FINES		
B-1	10.0 - 16.5	SS	16.4	19	16	3	0	56	44	SM	
B-2	2.5 - 4.0	SS	16.5								
B-3	7.5 - 9.0	SS	20.0								
B-4	2.5 - 4.0	SS	18.6								
B-5	10.0 - 11.5	SS	21.1								
B-6	5.0 - 6.5	SS	21.7	23	16	7	0	45	55	CL-ML	
B-7	5.0 - 6.5	SS	18.9								
B-8	7.5 - 9.0	SS	21.3								
B-9	10.0 - 11.5	SS	22.1								
B-10	2.5 - 4.0	SS	18.2								

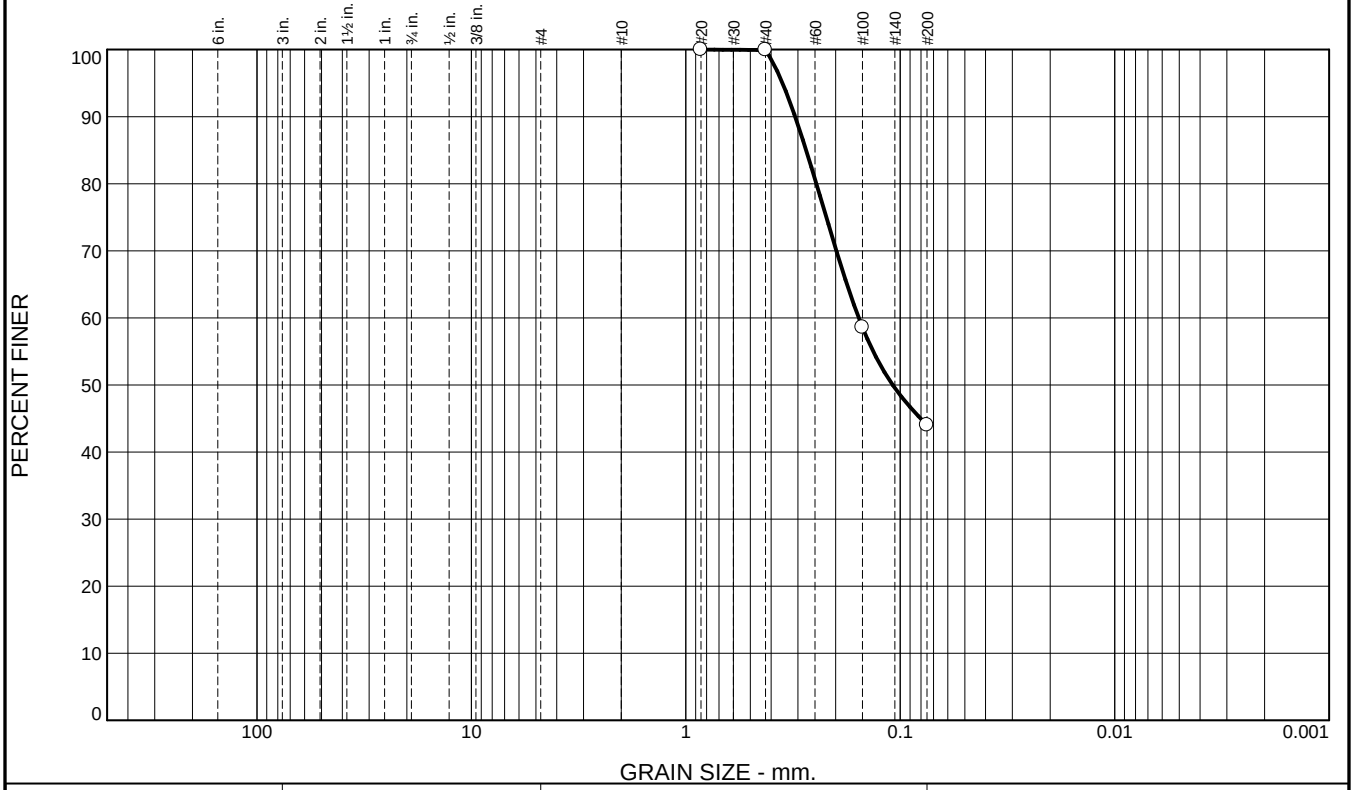
TRIAD ENGINEERING, INC.

Notes:
1) Soil tests performed in accordance with recognized ASTM testing standards.
2) SS = Split Spoon; UD = Undisturbed
RC = Rock Core
3) NV = Non Viscous; NP = Non Plastic

PROJECT NUMBER: 04-23-0289
PROJECT NAME: Dollar General - Waverly Road
LOCATION: Huntington, West Virginia

FIGURE C-1

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	0.0	0.1	55.9	44.0	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
#20	100.0		
#40	99.9		
#100	58.6		
#200	44.0		

* (no specification provided)

Material Description

medium brown silty sand

Atterberg Limits

PL= 16 LL= 19 PI= 3

Coefficients

D_{90}= 0.3093 D_{85}= 0.2750 D_{60}= 0.1561
D_{50}= 0.1085 D_{30}= D_{15}=
D_{10}= C_u= C_c=

Classification

USCS= SM AASHTO= A-4(0)

Remarks

Source of Sample: B-1 **Depth:** 10.0'-16.5'
Sample Number: S-5/S-6 **Date:** 10/12/2023

Triad Engineering, Inc.

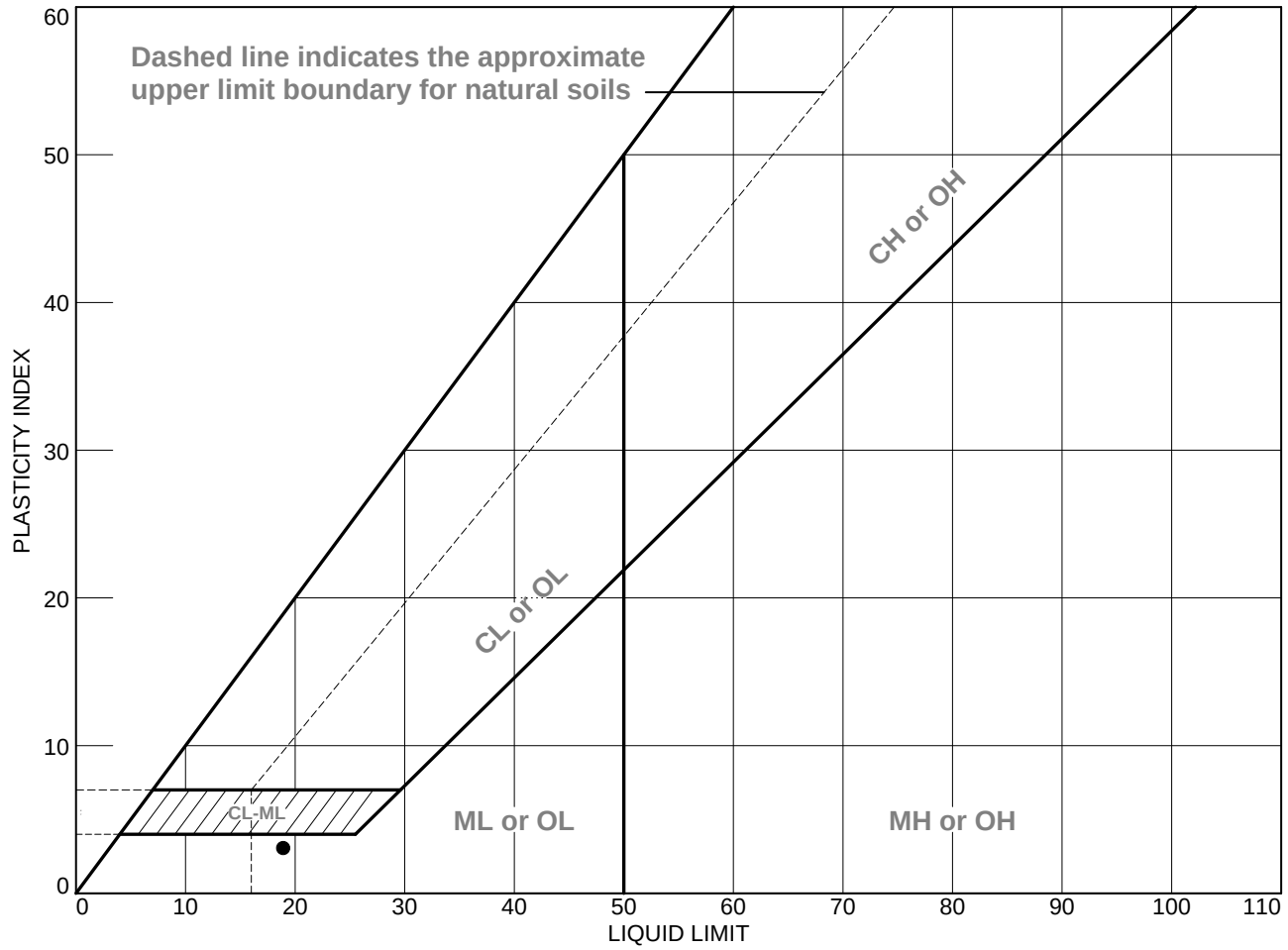
Morgantown, WV

Client: Berry Construction
Project: Dollar General-Waverly Road

Project No: 04-23-0289 **Figure C-2**

Tested By: DTB **Checked By:** JKM

LIQUID AND PLASTIC LIMITS TEST REPORT



SOIL DATA								
SYMBOL	SOURCE	SAMPLE NO.	DEPTH	NATURAL WATER CONTENT (%)	PLASTIC LIMIT (%)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	USCS
●	B-1	S-5/S-6	10.0'-16.5'	16.4	16	19	3	SM

Triad Engineering, Inc.

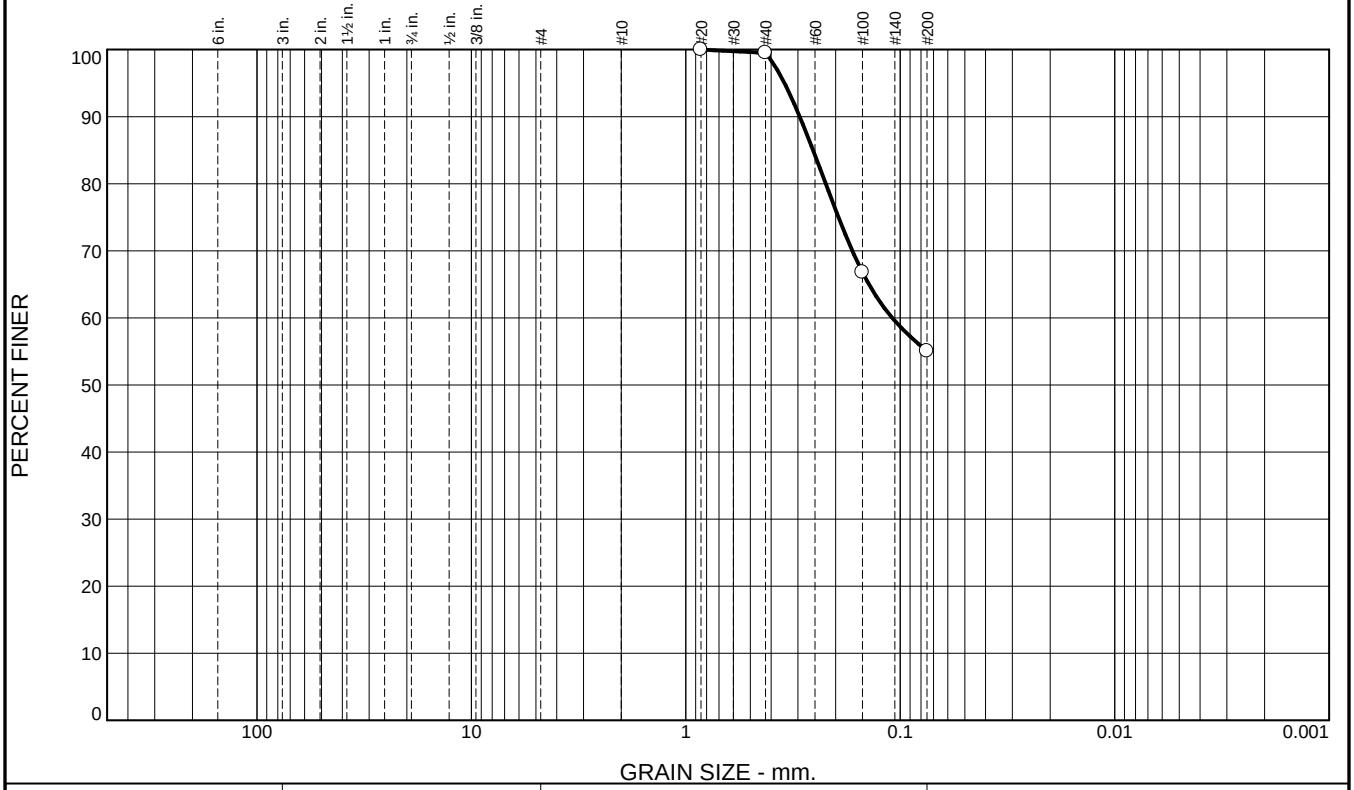
Morgantown, WV

Client: Berry Construction
Project: Dollar General-Waverly Road
Project No.: 04-23-0289

Figure C-3

Tested By: LMC **Checked By:** JKM

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	0.0	0.5	44.5	55.0	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
#20	100.0		
#40	99.5		
#100	66.8		
#200	55.0		

* (no specification provided)

Material Description

medium brown sandy silty clay

Atterberg Limits

PL= 16 LL= 23 PI= 7

Coefficients

D_{90}= 0.2945 D_{85}= 0.2552 D_{60}= 0.1090
D_{50}= D_{30}= D_{15}=
D_{10}= C_u= C_c=

Classification

USCS= CL-ML AASHTO= A-4(1)

Remarks

Source of Sample: B-6 **Depth:** 5.0'-6.5'
Sample Number: S-3

Date: 10/12/2023

Triad Engineering, Inc.

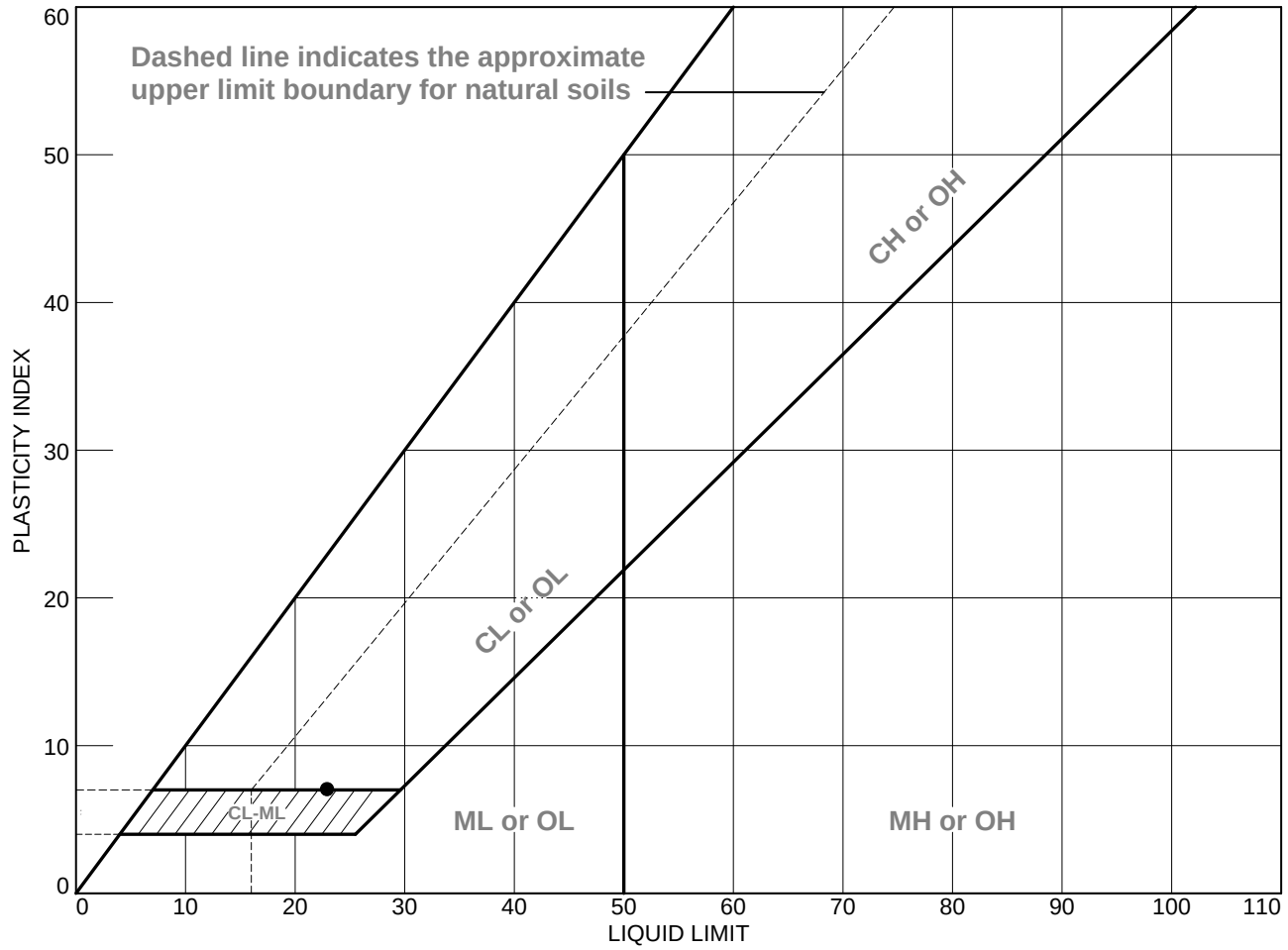
Morgantown, WV

Client: Berry Construction
Project: Dollar General-Waverly Road

Project No: 04-23-0289 **Figure C-4**

Tested By: DTB **Checked By:** JKM

LIQUID AND PLASTIC LIMITS TEST REPORT



SOIL DATA								
SYMBOL	SOURCE	SAMPLE NO.	DEPTH	NATURAL WATER CONTENT (%)	PLASTIC LIMIT (%)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	USCS
●	B-6	S-3	5.0'-6.5'	21.7	16	23	7	CL-ML

Triad Engineering, Inc.

Morgantown, WV

Client: Berry Construction
Project: Dollar General-Waverly Road

Project No.: 04-23-0289 **Figure C-5**

Tested By: LMC **Checked By:** JKM

TRIAD ENGINEERING, INC.

APPENDIX D

Seismic Information

Dollar General - Waverly Road

Latitude, Longitude: 38.398820, -82.520198

Date	10/11/2023, 2:51:47 PM
Design Code Reference Document	IBC-2015
Risk Category	II
Site Class	E - Soft Clay Soil

Type	Value	Description
S_S	0.151	MCE_R ground motion. (for 0.2 second period)
S_1	0.073	MCE_R ground motion. (for 1.0s period)
S_{MS}	0.379	Site-modified spectral acceleration value
S_{M1}	0.256	Site-modified spectral acceleration value
S_{DS}	0.252	Numeric seismic design value at 0.2 second SA
S_{D1}	0.171	Numeric seismic design value at 1.0 second SA

Type	Value	Description
SDC	C	Seismic design category
F_a	2.5	Site amplification factor at 0.2 second
F_v	3.5	Site amplification factor at 1.0 second
PGA	0.071	MCE_G peak ground acceleration
F_{PGA}	2.5	Site amplification factor at PGA
PGA_M	0.178	Site modified peak ground acceleration
T_L	12	Long-period transition period in seconds
SsRT	0.151	Probabilistic risk-targeted ground motion. (0.2 second)
SsUH	0.164	Factored uniform-hazard (2% probability of exceedance in 50 years) spectral acceleration
SsD	1.5	Factored deterministic acceleration value. (0.2 second)
S1RT	0.073	Probabilistic risk-targeted ground motion. (1.0 second)
S1UH	0.081	Factored uniform-hazard (2% probability of exceedance in 50 years) spectral acceleration.
S1D	0.6	Factored deterministic acceleration value. (1.0 second)
PGAd	0.6	Factored deterministic acceleration value. (Peak Ground Acceleration)
PGA_{UH}	0.071	Uniform-hazard (2% probability of exceedance in 50 years) Peak Ground Acceleration
C_{RS}	0.923	Mapped value of the risk coefficient at short periods
C_{R1}	0.905	Mapped value of the risk coefficient at a period of 1 s
C_V		Vertical coefficient

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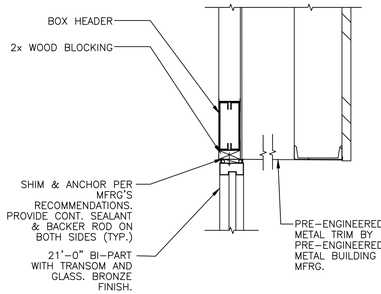
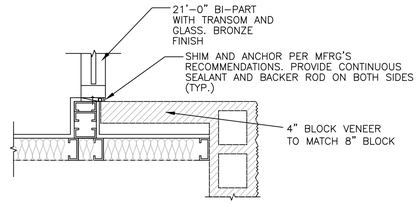
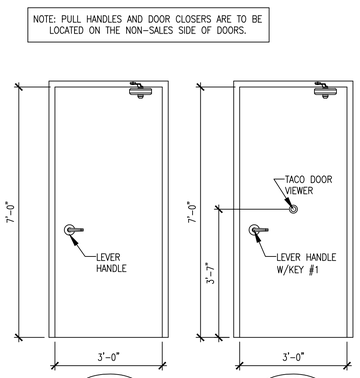


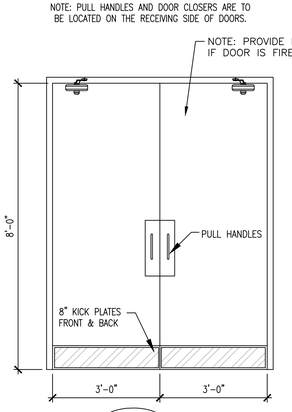
4 / A5 HEAD DETAIL
1"=1'-0"

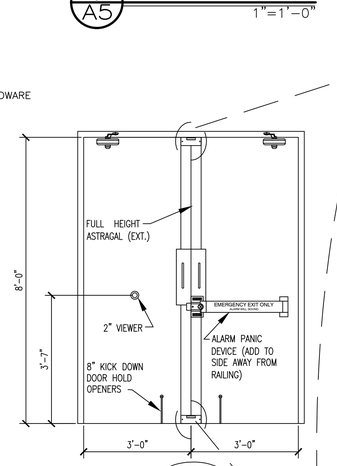
5 / A5 NOT USED
1"=1'-0"

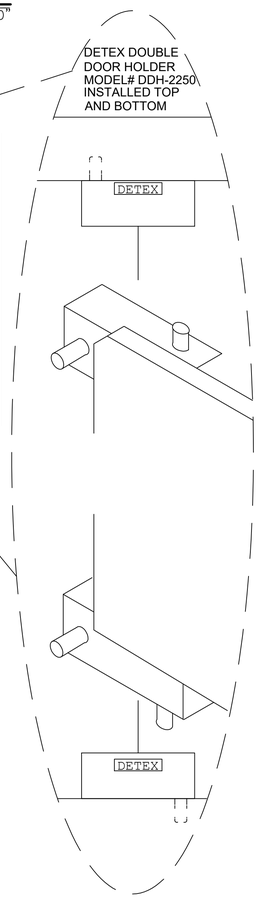


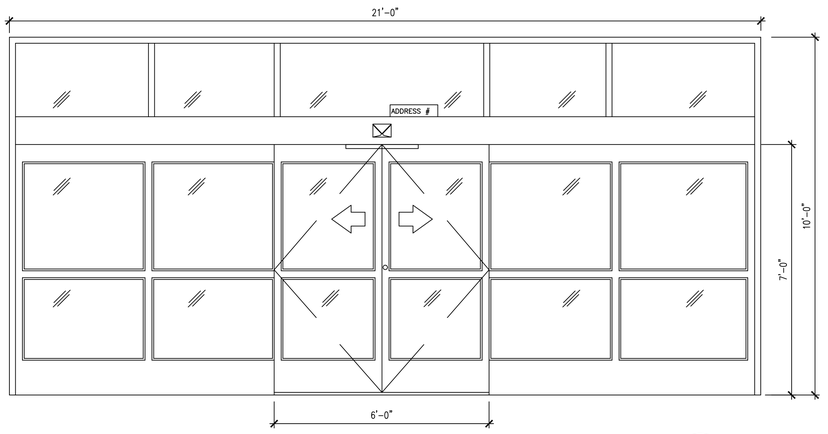
6 / A5 CORNER DETAILS
1"=1'-0"

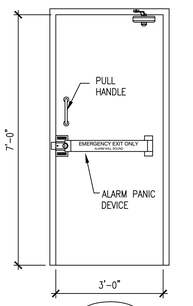












1 / A5 DOOR ELEVATIONS
1/2"=1'-0"

Z:\Projectmaster\23481-Wilson Pike Dev-Huntington,WV-DG\04 CDs\13-A5_DG_HuntingtonWV-B-DGWM.dwg Dec 20, 2023 - 5:53pm

STEPHEN P. MAHER, ARCHITECT

ARCHITECTURE
PLANNING
GRAPHICS

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SEAL

STATE OF WEST VIRGINIA
STEPHEN P. MAHER
12/21/23
14740
REGISTERED
ARCHITECT

CONSULTANT

OWNER

DG BTS HUNTINGTON, LLC

2525 Broad Street
Chattanooga TN 37408
mikeberry@berryconstruction
423-488-4053

LOCATION

DOLLAR GENERAL

STORE #30003
WAVERLY ROAD
HUNTINGTON, WV 25704

HUNTINGTON, WV

DATE

DOLLAR GENERAL PERMIT SET
1.9.24
MJM #23481

SHEET

DOOR DETAILS

AS NOTED

A5